Appendix 9.5Preliminary Stormwater PollutionPrevention Plan (SWPPP)

Appendix 9.5.1 Proposed Action SWPPP

# Engineering Report SILO RIDGE RESORT COMMUNITY

SEQRA - Master Stormwater Pollution Prevention Plan

Town of Amenia Dutchess County, New York

April 10, 2006



Prepared for:

Higher Ground Country Club Management Co., L.L.C. P.O. Box 86, Route 22 Amenia, New York 12501

# Stormwater Pollution Prevention Plan SILO RIDGE RESORT COMMUNITY

SEQRA - Master Stormwater Pollution Prevention Plan

Town of Amenia Dutchess County, New York

April 10, 2006



Prepared by:

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#### **PREPARER OF THE SWPPP**

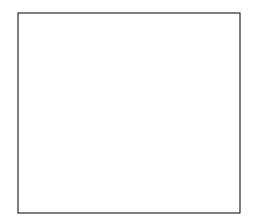
"I certify under penalty of law that this document and all attachments were prepared under my direction or supervision in accordance with a system designed to assure that qualified personnel properly gathered and evaluated the information submitted. Based on my inquiry of the person(s) who manage the system, or those persons directly responsible for gathering the information, the information submitted is, to the best of my knowledge and belief, true, accurate, and complete. I am aware that false statements made herein are punishable as a class A misdemeanor pursuant to Section 29.45 if the Penal Law."

Name:<sup>1</sup> Richard Chazen, P.E.

Title: <u>Principal – The Chazen Companies</u>

License No.: 57818

Date: <u>April 10, 2006</u>



 $<sup>^{\</sup>rm 1}$  This is a signature of an officer of the corporation authorized in policy or decision making functions of the corporation.

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- 1 year 24 hour Storm Event Model Computations
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#### **1.0 EXECUTIVE SUMMARY**

The proposed Silo Ridge Resort Community ("the project") is a master planned community complex incorporating recreational amenities, community facilities, up-scale hotel and spa and a range of residential housing types, which will be designed, developed and operated with sensitivity to environmental resources and in keeping with the character of the area and local community.

This Master Stormwater Pollution Prevention Plan (SWPPP) has been prepared to support state environmental quality review (SEQR) of the proposed project. As such, design concepts are provided for stormwater collection and conveyance systems, and water quality and quantity control facilities. This report is not intended to be a final engineering design as certain detailed aspects of the project are liable to change during the review process. Portions of the design were advanced to substantiate regulatory compliance determinations and to provide input pertinent to the environmental assessment of impacts of the proposed project. Final stormwater design will be advanced in support of and during site plan permitting.

The intent of this Master SWPPP is to provide sufficient documentation for an overall SEQR determination, and to serve as the baseline for the final SWPPP that will be prepared for the proposed development, as approved. The stormwater analysis follows the *NYS Guidelines for Urban Erosion and Sediment Control* and USDA Technical Release No. 20.

The analysis is an integral part of the project's natural resource environmental analysis which takes into consideration existing parameters of site topography, soils, erosion potential, surface waters, their connectivity and water quality, vegetative characteristics, visual resources, and the overall health of the watersheds. Flow projections, modeling, and project design sensitive to stormwater concerns combine to manage stormwater in compliance with current regulations, incorporating engineering design measures to minimize impacts on the sites natural resources through proposed stormwater management facilities discussed in the report. The methodology used to develop this Master SWPPP shall be adhered to for the preparation of the project's final SWPPP. Stormwater quality and quantity controls designed for this Master SWPPP are preliminary in nature and are intended to demonstrate their location, approximate size, and design concept. Detailed analysis of these practices must be performed, and the design of each practice must be refined as part of the final SWPPP preparation. Construction phase pollutant sources anticipated at the site include sediment, vehicle fuels and lubricants, chemicals associated with building construction, and building materials. Without adequate control there is the potential for each type of pollutant to be transported by stormwater. Mitigation measures involve preventing soil erosion and sedimentation resulting from stormwater run-off both during and after construction. During construction, this is accomplished by sequencing site disturbance activities to establish erosion controls, minimize disturbed areas, maintain existing vegetation as much as possible, and stabilize newly disturbed areas as soon as possible. Stormwater pollutant controls utilized during construction will include temporary sediment barriers and sediment basins. Stormwater pollutant controls utilized after construction will include stormwater quality control facilities designed in accordance with the "New York State Stormwater Management Design Manual".

Land development can also have an effect on site hydrology. Impervious areas such as rooftops, roads, driveways, and parking lots can cause rainfall to rapidly convert into stormwater runoff. Increases in runoff can cause stream bank erosion and floodplain expansion. To mitigate these impacts, stormwater quantity controls will be implemented to capture and release runoff less than predevelopment discharge rates. A hydrologic and hydraulic analysis was performed using computer modeling and an evaluation of the proposed improvements across the subject site. A conventional stormwater management system was developed, consisting of centralized stormwater quantity controls designed to meet the requirements of the *"New York State Stormwater Management Design Manual"*, dated August 2003.

The plan, for the most part, allows for the maintenance of existing drainage patterns while continuing the conveyance of upland watershed areas. The stormwater management system has been designed to attenuate runoff generated during the 1-, 2-, 10-, 25-, 50- and 100- year storm events such that the peak rates realized at the designated design points will not exceed the rates that existed prior to development of the project.

There are two relatively large unnamed NYSDEC Class "C" ponds, DEC #1121 and #1122, in the central portion of the project site. The existing Silo Ridge Golf Club draws water from pond #1121 to irrigate the golf course during the summer months. During irrigation operations the water demands can outpace the natural recharge of this pond causing a noticeable drop (approximately 8-feet) in water level. To offset this drop in water level, a system of valves allows the golf course operators to transfer water from pond #1122 as necessary.

Because the golf course will not be expanded (i.e. additional holes), it is anticipated that the water demand for irrigation purposes will not increase. However, to minimize the strain on pond #1121, treated wastewater will be pumped into pond #1122. As the site becomes developed and occupied, pond #1122 will become the primary source of golf course irrigation water, with pond #1121 providing only supplemental "make-up" water during the most severe droughts. The recycling of highly treated "clean" wastewater effluent is a sustainable design feature of the project which will assist in the preservation of ground water resources.

Several areas of proposed roadway are located such that the topography or adjacent constraints make it impractical to locate stormwater quality facilities. Waivers will be requested from NYSDEC for treatment of stormwater runoff from these areas as the site plan review and approval process progresses.

The "New York State Standards and Specifications for Erosion and Sediment *Control*" identifies that no more than 5-acres may be disturbed at any given time. Construction of this project will involve a golf course, cluster developments, and residential subdivisions with roadways in excess of a mile long. This will require construction to proceed with disturbance of greater than 5-acres at one time. The NYSDEC allows disturbance of greater than 5-acres upon receipt of written authorization. Therefore, waivers will be requested from NYSDEC for these areas as the site plan review and approval process progresses. Typically NYSDEC will grant such a waiver provided that every attempt is made to minimize erosion and establish vegetation as quickly as possible.

# 2.0 NYSDEC SPDES GENERAL PERMIT GP-02-01

A summary of responsibilities and obligations of all parties involved with compliance with the NYSDEC SPDES General Permit, GP-02-01 conditions are outlined in the subsequent sections. For a complete listing of the responsibilities and obligations refer to the SPDES General Permit GP-02-01 presented in Appendix A.

# 2.1 Definitions

1. "General Permit" shall mean the general stormwater permit for construction activities issued by the United States Environmental Protection Agency, New York State Department of Environmental Conservation or a comparable general permit issued by local or other appropriate governmental agency.

- 2. "Operator" shall be any party (or parties) that has (or have) either (a) operational control over construction plans and specifications, including the ability to make modification to those plans and specifications or (b) day-to-day operational control of those activities at a project which are necessary to ensure compliance with the SWPPP for the site or other permit conditions. There may be occasions during the course of a project in which there are multiple Operators, all of which will need to file and maintain the appropriate SWPPP documents and plans, including without limitation, the Notice of Intent (NOI) and Notice of Termination (NOT).
- 3. "Operator's Engineer" shall be that person or entity retained by an Operator to design and oversee the implementation of the SWPPP.
- 4. "Contractor" shall be that person or entity identified as such in the construction contract with the Operator. The term "Contractor" shall also include the Contractor's authorized representative, as well as any and all subcontractors retained by the Contractor.
- 5. "Qualified Professional" shall be a person knowledgeable in the practices of erosion and sediment controls, such as a NYS professional engineer or Certified Professional in Erosion and Sediment Control (CPESC).

# 2.2 Operator's Responsibilities

- 1. Have an authorized corporate officer sign the NOI and SWPPP Operator's Certification forms.
- 2. Submit the signed form along with any required fees and attachments to the following:

NYS DEC "Notice of Intent" Bureau of Permit 625 Broadway Albany, New York 12233-3505

Town of Amenia Planning Board P.O. Box 126 36B Mechanic Street Amenia, New York 12501

3. Retain the services of a "Qualified Professional" as defined under Section 2.1 "Definitions" to provide the services outlined in Section 2.3 "Operator's Engineer's Responsibilities".

- 4. Schedule a pre-construction meeting which shall include the Town representative, Operator's Engineer, Contractor, and their sub-contractors to discuss responsibilities as they relate to the implementation of this SWPPP.
- 5. Require the Contractor to fully implement the SWPPP prepared for the site by the Operator's Engineer.
- 6. Forward a copy of the original permit certificate received from the regulatory agency to the Operator's Engineer for project records, and to the Contractor for display at the job site.
- 7. Keep a copy of the SWPPP, all NOI's, permit certificates, permit language, Spill Prevention, Countermeasures, and Cleanup ("SPCC") Plan, inspection records, and other required records on the job site so that they may be made available to the regulatory agencies.
- 8. Post at the site, in a publicly-accessible location, a summary of the site inspection activities on a monthly basis.
- 9. Prepare a written summary of projects status with respect to compliance with the general permit at a minimum frequency of every three months during which coverage under the permit exists. The summary should address the status of achieving the overall goal of the SWPPP. The summary shall be handled in the same manner as prescribed for SWPPP's under Part III, subsection B of the NYSDEC SPDES General Permit GP-02-01.
- 10. Submit a Notice of Termination (NOT) form (see Appendix G) within 48 hours of receipt the Operator's Engineer's certification of final site stabilization to the following:

NYS DEC "Notice of Termination" Bureau of Permit 625 Broadway Albany, New York 12233-3505

Town of Amenia Planning Board P.O. Box 126 36B Mechanic Street Amenia, New York 12501

11. Request and receive all SWPPP records from the Operator's Engineer and archive those records for a minimum of three years after the NOT is filed.

12. Require the implementation of the Post-Construction Inspections and Maintenance procedures outlined in Appendix H.

# 2.3 Operator's Engineers Responsibilities

- 1. Prepare the SWPPP using good engineering practices, best management practices, and in compliance with all federal, state, and local regulatory requirements.
- 2. Prepare the Notice of Intent Form (NOI) form (see Appendix B) and forward to Operator for signature.
- 3. Prepare and forward the SWPPP Operator Certification form for Operator's signature (see Appendix C).
- 4. Include a signed NOI and Operator Certification forms in the SWPPP prepared for the job site.
- 5. Provide copies of the SWPPP to the Town of Amenia once all signatures and attachments are complete.
- 6. Prepare a construction site log book to be used in maintaining a record of all inspection reports generated throughout the duration of construction.
- 7. Participate at pre-construction meeting with the Town representative, Operator, Contractor, and their sub-contractors to discuss responsibilities as they relate to the implementation of this SWPPP.
- 8. Enter Contractor's information in Section 2.5 "SWPPP Participants" once a Contractor is selected by the Operator.
- 9. Conduct an initial site assessment of the site prior to the commencement of construction and certify in an inspection report that the appropriate erosion and sediment control measures described within this SWPPP and required by Part III.D of the NYSDEC General Permit, GP-02-01, have been adequately installed and implemented to ensure overall preparedness of the site.
- 10. Provide on-site inspections at least every seven (7) calendar days and within 24 hours of the end of a storm event of ½-inch or greater to determine compliance with the SWPPP. The written inspection reports shall be provided to the Operator within 24 hours of the field inspection with any deficiencies identified. A description of Construction Phase Inspections and Maintenance requirements are in presented Appendix D. A sample inspection form is provided in Appendix E.

- 11. Review the Contractor's SWPPP records on a periodic basis to ensure compliance with the requirements for daily reports and inspections and maintenance logs.
- 12. Maintain the construction site log book throughout the duration of construction.
- 13. Update the SWPPP each time there is a significant modification to the pollution prevention measures or a change of the principal Contractor working on the project who may disturb site soil.
- 14. Provide the Operator certification that an inspection has been completed verifying that the site has undergone final stabilization using appropriate measures and that all temporary erosion and sediment controls have been removed.
- 15. Transfer the SWPPP documents, along with all NOI's, permit certificates, NOT's, construction site log book, and written records required by the General Permit to the Operator for archiving.

# 2.4 Contractor's Responsibilities

- 1. Send all notifications required by SPDES General Permit Number GP-02-01 via certified mail with return receipt. Copies of mailing receipts shall be kept on record at the project site with the SWPPP and shall be considered part of the contract documents.
- 2. Sign the SWPPP Contractor's Certification form contained within Appendix C and forward to the Operator's Engineer for inclusion into the SWPPP.
- 3. Provide the names and addresses of all subcontractors working on the project site. Require all subcontractors who will be involved with the major construction activities that will result in soil disturbance sign a copy of the Contractor's Certification Form and forward to the Operator's Engineer for inclusion into the SWPPP. This information must be retained as part of the SWPPP.
- 4. Participate in pre-construction meeting which shall include the Town representative, Operator, Operator's Engineer, and all sub-contractors to discuss responsibilities as they relate to the implementation of this SWPPP.
- 5. Implement site stabilization, erosion and sediment control measures, and other requirements of the SWPPP.

- 6. Conduct daily inspections, prepare, and retain written documentation of inspections as well as all repairs/maintenance activities performed on erosion and sediment control measures.
- 7. Maintain a record of the dates when major grading activities occur, when construction activities temporarily or permanently cease on a portion of the site, and when stabilization measures are initiated, until such time as the NOT is filed. A log for keeping such records is provided in Appendix F.
- 8. Provide monthly training sessions for all entities and subcontractors involved with installing, applying, performing, maintaining and inspection of the SWPPP.

# **2.5 SWPPP Participants**

1.	Operator's Engineer:	The Chazen Companies 21 Fox Street Poughkeepsie, NY 12601 Phone: (845) 454-3980 Fax: (845) 454-4026
2.	Operator:	Mr. Robert F. Caeners Silo Ridge Country Club P.O. Box 86 Route 22 Amenia, NY 12501 Phone: (845) 373-7000 Fax: (845) 373-8847
3.	Contractor <sup>2</sup> :	Name and Title: Company Name: Mailing Address: Phone: Fax:

<sup>&</sup>lt;sup>2</sup> Contractor's information to be entered once the Contractor has been selected.

# **3.0 INTRODUCTION**

Higher Ground Country Club Management Co. L.L.C. is proposing the improvement of the existing 668 (+/-) acre Silo Ridge Country Club into a resort community which will include 328 town home and condominium units, 41 single-family residences, two resort hotels, a banquet space, two restaurants, a conference space, a spa and wellness center as well as improved golf course facilities. In general stormwater management ponds will be used to treat and attenuate additional stormwater runoff produced from the proposed improvements.

The proposed project is a master planned community complex incorporating recreational amenities, community facilities, up-scale hotel and spa and a range of residential housing types, which will be designed, developed and operated with sensitivity to environmental resources and in keeping with the character of the area and local community. The existing Silo Ridge Golf Course will be modified to incorporate post construction stormwater management practices developed in support of the proposed project. It is anticipated that these practices will become course amenities and/or hazard areas. A variety of housing configurations will be oriented to various lifestyles using architectural themes compatible with the natural setting of the site and consistent with the character of the local community. The project will include all necessary infrastructure, including an on-site central water and sanitary sewer system, drainage facilities and extensive natural resource conservation, protection and enhancement areas.

This Master SWPPP has been prepared for major activities associated with construction of Silo Ridge Country Club Golf Resort Community in the Town of Amenia, Dutchess County, New York. This Master SWPPP includes the elements necessary to comply with the national baseline general permit for construction activities enacted by the U.S. Environmental Protection Agency (EPA) under the National Pollutant Discharge Elimination System (NPDES) program and all local governing agency requirements. A final detailed SWPPP conforming to the intent of this Master SWPPP will be prepared in support of and during site plan permitting. This SWPPP must be implemented at the start of construction.

#### **3.1 Project Description**

The Applicant, Higher Ground Country Club, LLC, is proposing the development of a resort community on a 668±-acre site to be known as Silo Ridge. The project area is located west of New York State (NYS) Route 22 in the Town of Amenia, Dutchess County, New York, identified as Parcel Numbers 7066-00-732810, 7066-00-860725, 7066-00-742300, 7066-00-670717, and 7067-00-709177 by the Town of Amenia Tax Map. The site is currently developed with a 170-acre 18-hole championship golf course. Irrelevant

The development will consist of 328 town home and condominium units, 41 singlefamily residences, two resort hotels, a banquet space, two restaurants, a conference space, a spa and wellness center as well as improved golf course facilities. The Project construction will consist primarily of site grading, roadway grading and paving, storm drainage, water supply and sanitary sewage collection and treatment.

# **3.2 Generalized Construction Phasing**

It is the intent of the project sponsor to commence with construction in the following order:

- 1. The first component of development planned will be the modification of the existing golf course. There currently is an 18-hole golf course with practice green and driving range on the project site. In support of the proposed development, multiple greens and fairways will require partial modification. Construction will include the relocation of tees, regrading of greens and fairways and the incorporation of new stormwater management facilities.
- 2. The next component of development planned is the first of two hotels, existing golf club house renovation, the Resort District (Area "A") and Golf District (Area "H"). This component will also involve the construction of the water supply, treatment and storage system, and wastewater treatment plant with associated roadway and infrastructure.
- 3. Subsequent components of development will include residential communities consisting of town homes and single-family residences, and second hotel with associated roadway and infrastructure. The order and timeframe for each of these phases will depend on market demand.

# 3.3 Purpose

Construction phase pollutant sources anticipated at the site are disturbed (exposed) soil, vehicle fuels and lubricants, chemicals associated with building construction, and building materials. Without adequate control there is the potential for each type of pollutant to be transported by stormwater.

This report considers the impacts associated with the intended development with the purpose of:

1. Maintaining existing drainage patterns as much as possible while continuing the conveyance of upland watershed runoff;

- 2. Controlling increases in the rate of stormwater runoff resulting from the proposed development in order to not adversely impact downstream conditions;
- 3. Mitigating potential stormwater quality impacts and preventing soil erosion and sedimentation resulting from stormwater runoff generated both during and after construction.

To demonstrate this, pre- and post-development stormwater runoff conditions have been estimated and proposed stormwater management facilities have been described and evaluated.

The hydrologic and hydraulic analyses were completed in accordance with the following standards and guidelines:

- New York State Stormwater Management Design Manual (August 2003).
- New York State Standards and Specifications for Erosion and Sediment Control (August 2005).
- Town of Amenia Regulations.

The analysis and design completed and documented in this report is intended to be part of the application made for a resort community development project completed on behalf of the Higher Ground Country Club Management Co. L.L.C.

This Master SWPPP and the accompanying drawings entitled "Silo Ridge Golf Resort Community" have been submitted as a set. These drawings are considered an integral part of this Master SWPPP, therefore this Master SWPPP is not considered complete without them. References made herein to "the plans" or to a specific "sheet" refer to these drawings.

A location map of the site has been provided in Appendix I, as Figure 1.

# 4.0 TYPICAL SITE DEVELOPMENT CONSTRUCTION SEQUENCE

Described below are the major construction activities that are the subject of this SWPPP. They are presented in the order (or sequence) they are expected to begin, but each activity will not necessarily be completed before the next begins. Also, these activities could occur in a different order if necessary to maintain adequate erosion and sediment control.

The Contractor will be responsible for implementing the following erosion and sediment control measures. The Contractor may designate these tasks to certain subcontractors as he sees fit, but the ultimate responsibility for implementing these controls and ensuring their proper function remains with the Contractor. The order of activities will be as follows:

- 1. Selectively clear only the areas required for the installation of the stabilized construction entrances/exits and temporary sediment and erosion measures.
- 2. Install stabilized construction entrances/exits for all construction entrances/exits. This will be the first construction work on the project.
- 3. Install sediment control barriers down slope from construction activities that disturb site soil.
- 4. Install temporary sediment basins.
- 5. Begin clearing and grubbing operations. Clearing and grubbing shall be done only in areas where earthwork will be performed and only in areas where construction is planned to commence within 14 days after clearing and grubbing.
- 6. Frequent watering of the excavation and fill areas shall be done to minimize wind erosion.
- 7. Commence site grading.
- 8. Disturbed areas of the site, where construction activity has ceased for more than 14 days, shall be temporarily or permanently seeded, mulched, and watered.
- 9. Install protective measures at the locations of all grate inlets, curb inlets, and at the ends of all exposed storm sewer pipes.
- 10. Construct all utilities, curb or gutter, gutter inlets, area inlets, and storm sewer manholes, as shown on the plans. Inlet protection may be removed temporarily for this construction. Place required rip-rap at locations shown on the plans.
- 11. Finalize pavement sub-grade preparation.
- 12. Install sub-base material as required for pavement.

- 13. Remove protective measures around inlets and manholes no more than 24 hours prior to placing stabilized base course.
- 14. Carry out final grading, seeding, mulching, and landscaping.
- 15. Install asphalt pavement.
- 16. Remove silt fencing only after all paving is complete and exposed tributary surfaces are stabilized.
- 17. Remove stabilized construction entrances only prior to pavement construction in these areas (These areas are to be paved last).
- 18. Complete on-site stabilization.
- 19. Remove temporary sediment controls only after all paving is complete and exposed surfaces are completely stabilized, and cleanout all stormwater collection conveyance, and treatment facilities.

Refer to the accompanying plans for clarifications and specifications regarding the construction sequencing schedule.

### **5.0 SITE DESCRIPTION**

#### 5.1 Land Use

The entire project site is located west of New York State Route 22. The majority of the project site lies south of New York State Route 44, with a small portion of the project site lying north of NYS Route 44. At the north end of the project site (north of NYS Route 44) the project site currently consists of open meadows as well as wooded areas. On the south side of NYS Route 44, the project site consists of an 18hole golf course, a golf course club house, parking areas, as well as other miscellaneous golf course amenities. The Amenia Stream (otherwise known as Cascade Brook) traverses along the eastern edge of the project site and exits the project site near the existing golf course entrance on NYS Route 22. Along the western half of the project site the existing land is comprised of open meadows and large wooded areas which contain an existing dirt trail system. Lastly, the southwestern corner of the project site is largely wooded. It should be noted that there are numerous wetlands scattered throughout the project site, and a large NYSDEC wetland exists near the southeast corner of the project site. Please refer to the accompanying plan set for more information.

The project site is primarily lies within the Town of Amenia's RA, Agricultural Density, with a small portion along NYS Route 22 in the Town's M, Industrial, districts.

Generally, stormwater on the project site flows in an easterly direction and either drains offsite, or infiltrates into the ground. Specifically, at the north end of the project site, (north of NYS Route 44) stormwater flows southeasterly across the north end of the project site, flows under NYS Route 44 via culverts, and spills back onto the project site south of NYS Route 44. The stormwater runoff, then flows across the project site via overland flow, stormwater underdrains, culverts, or ponds and channels, and eventually reaches the Amenia Stream (which flows from north to south). The Amenia Stream then flows offsite via a large box culvert located near the existing golf course entrance on NYS Route 22.

With the exception of a small area consisting of approximately 1,400 linear feet of the existing site entrance roadway as well as the area immediately adjacent to the site roadway, the remainder of the project site drains west to east to a large wetland (Wetland L, see accompanying plans) located near the southeast corner of the project site. This wetland drains offsite via a 30-inch culvert located under NYS Route 22.

Generally stormwater that drains to this large wetland goes through a series of ponds, culverts and/or streams located throughout the central portion of the site prior to reaching the wetland. Lastly, the aforementioned 1400 linear feet of existing site entrance roadway as well as the area adjacent to the roadway drain to two small infiltration ponds located at the front entrance to the site (see accompanying plans).

# 5.2 Soils

The United States Department of Agriculture (USDA) Soil Conservation Service (SCS) Soil Survey for Dutchess County was reviewed and provided surficial soil conditions for the study area. The SCS identified the presence of Copake, Dutchess-Cardigan complex, Fluvaquents-Udifluvents complex, Galway-Farmington complex, Georgia, Hollis-Chatfield-Rock outcrop complex, Nassau-Cardigan complex, Nassau-Rock outcrop complex, Palms muck, Stockbridge, Stockbridge-Farmington complex, Sun, Udorthents, Udorthents, and Wayland series soil types. Soil data was provided by the SCS and is presented in Table 1.

	HYDRO-		S	OIL PROFILE		DEPTH TO	DEPTH
MAP SYMBOL/ DESCRIPTION	LOGIC SOIL GROUP	SLOPE (%)	DEPTH (IN)	USDA TEXTURE	K VALUE	WATER TABLE (FT)	TO BEDROCK (IN)
CuC / Copake gravelly silt loam, nearly level	В	5 to 16	0-6 6-36 36-80	Gravelly silt loam. Gravelly loam, gravelly silt loam, fine sandy loam. Stratified gravelly loamy fine sand to very gravelly coarse sand.	0.10-0.24	> 6.0	> 60
CuD / Copake gravelly silt loam,	В	15 to 30	0-6 6-36	Gravelly silt loam. Gravelly loam,	0.10-0.24	> 6.0	> 60

#### **Table 1: USDA Soil Data**

	HYDRO-	GLODE	SOIL PROFILE		К	DEPTH TO	DEPTH
MAP SYMBOL/ DESCRIPTION	LOGIC SOIL GROUP	SLOPE (%)	DEPTH (IN)	USDA TEXTURE	K VALUE	WATER TABLE (FT)	TO BEDROCK (IN)
rolling			36-80	gravelly silt loam, fine sandy loam. Stratified gravelly loamy fine sand to very gravelly coarse sand.			
CwB / Copake channery silt loam, 3 to 8 % slopes	В	3 to 8	0-6 6-36 36-80	Channery silt loam. Channery loam, channery silt loam, fine sandy loam. Stratified channery loamy fine sand to channery coarse sand.	0.10-0.24	3.0 to 6.0	> 60
DwB / Dutchess- Cardigan complex, undulating	В	1 to 6	0-8 8-28 28-86	Silt loam. Silt loam, channery silt loam, gravelly loam. Channery silt loam, very channery fine sandy loam, very gravelly sandy loam.	0.32-0.37	> 6.0	20 to 40 & > 60
DwD / Dutchess- Cardigan complex, hilly, rocky	В	15 to 30	0-8 8-28 28-86	Silt loam. Silt loam, channery silt loam, gravelly loam. Channery silt loam, very channery fine sandy loam, very gravelly sandy loam.	0.32-0.37	> 6.0	20 to 40, > 60 & rock outcropping
Ff / Fluvaquents- Udifluvents complex, frequently flooded	D	0 to 3	0-5 5-72	Silt loam. Very gravelly sand, gravelly silt loam, silty clay loam.	0.28-0.32	+0.5 to 1.5 & 2.0 to 6.0	> 60
GfD / Galway- Farmington complex, hilly	В	15 to 30	0-7* 7-15* 15*	Loam.* Silt loam, loam, gravelly fine sandy loam.* Unweathered bedrock*	0.24-0.32	1.5 to 3.0 & >60	10-20, 20-40, & rock outcropping
GsB / Georgia silt loam, 3 to 8 % slopes	С	3 to 8	0-8 8-27 27-80	Silt loam. Loam, silt loam, very gravelly fine sandy loam. Loam, very gravelly fine sandy loam, silt loam gravelly fine sandy loam.	0.32	1.5 to 3.0	> 60
GsC / Georgia silt loam, 8 to 15 % slopes	С	8 to 15	0-8 8-27 27-80	Silt loam. Loam, silt loam, very gravelly fine sandy loam. Loam, very gravelly fine sandy loam, silt loam gravelly fine sandy loam.	0.32	1.5 to 3.0	> 60
HoE / Hollis- Chatfield-Rock outcrop complex, steep	C/D	25 to 45	0-3** 3-15** 15**	Loam.** Gravelly fine sandy loam, sandy loam, loam.** Unweathered bedrock.**	0.24-0.32	> 6.0	10-20, 20-40, & rock outcropping

	HYDRO- SOIL PROFILE		OIL PROFILE		DEPTH TO	DEPTH	
MAP SYMBOL/ DESCRIPTION	LOGIC SOIL GROUP	SLOPE (%)	DEPTH (IN)	USDA TEXTURE	K VALUE	WATER TABLE (FT)	TO BEDROCK (IN)
NwC / Nassau- Cardigan complex, rolling, very rocky	В	5 to 16	0-5*** 5-16*** 16***	Channey silt loam.*** Very channery silt loam, very channery loam.*** Unweathered bedrock.***	0.20-0.37	> 6.0	10-20, 20-40, & rock outcropping
NwD / Nassau- Cardigan complex, hilly, very rocky	С	15 to 30	0-5*** 5-16*** 16***	Channey silt loam.*** Very channery silt loam, very channery loam.*** Unweathered bedrock.***	0.20-0.37	> 6.0	10-20, 20-40, & rock outcropping
NxE / Nassau-Rock outcrop complex, steep	С	25 to 45	0-5 5-16 16	Channery silt loam. Very channery silt loam, very channery loam. Unweathered bedrock.	0.20	>6.0	10-20 & rock outcropping
NxF / Nassau-Rock outcrop complex, very steep	С	45 to 70	0-5 5-16 16	Channery silt loam. Very channery silt loam, very channery loam. Unweathered bedrock.	0.20	>6.0	10-20 & rock outcropping
Pc / Palms muck	A/D	0 to 2	0-12 12-30 03-80	Muck. Muck. Clay loam, silty clay loam, gravelly fine sandy loam.	0.37	+1.0-1.0	> 60
SkC / Stockbridge silt loam, 8 to 15 % slopes	С	8 to 15	0-6 6-23 23-80	Silt loam. Loam, silt loam, gravelly loam. Gravelly loam, silt loam, very gravelly fine sandy loam.	0.24-0.37	> 6.0	> 60
SkD / Stockbridge silt loam, 15 to 25 % slopes	С	15 to 25	0-6 6-23 23-80	Silt loam. Loam, silt loam, gravelly loam. Gravelly loam, silt loam, very gravelly fine sandy loam.	0.24-0.37	> 6.0	> 60
SkE / Stockbridge silt loam, 25 to 45 % slopes	С	25 to 45	0-6 6-23 23-80	Silt loam. Loam, silt loam, gravelly loam. Gravelly loam, silt loam, very gravelly fine sandy loam.	0.24-0.37	> 6.0	> 60
SmD / Stockbridge- Farmington complex, hilly, rocky	С	15 to 30	0-7 7-15 15	Loam**** Silt loam, loam, very fine sandy loam, gravelly fine sandy loam.**** Unweathered bedrock.****	0.24-0.37	> 6.0	10-20, > 60 & rock outcropping
Su / Sun silt loam	D	0 to 3	0-4 4-22 22-80	Silt loam. Gravelly fine sandy loam, sandy loam, gravelly loam, silt loam, loam. Gravelly fine sandy loam, gravelly loam, very gravelly sandy loam.	0.20-0.28	+1.0-0.5	> 60

	HYDRO-	CL ODE	S	OIL PROFILE		DEPTH TO	DEPTH
MAP SYMBOL/ DESCRIPTION	LOGIC SOIL GROUP	SLOPE (%)	DEPTH (IN)	USDA TEXTURE	K VALUE	WATER TABLE (FT)	TO BEDROCK (IN)
Ud / Udorthents, smoothed	A/D	0 to 8	0-4 4-70	Loam. Very gravelly sandy loam, channery loam, silty clay loam.	0.32-0.37	> 3.0	> 60
Ue / Udorthents, wet substratum	A/D	0 to 8	0-4 4-72	Loam. Very gravelly loamy sand, channery loam, silty clay loam.	0.32-0.37	1.0-3.0	> 60
Wy / Wayland silt loam	C/D	0 to 3	0-9 9-80	Silt loam. Silt loam, silty clay loam.	0.43	+0.5-1.0	> 60

\*Soil Profile listed is for the Farmington soil portion of the Galway-Farmington complex.

\*\*Soil Profile listed is for the Hollis soil portion of the Hollis-Chatfield-Rock outcrop complex.

\*\*\*Soil Profile listed is for the Nassau soil portion of the Nassau-Cardigan complex.

\*\*\*\*Soil Profile listed is for the Farmington soil portion of the Stockbridge-Farmington complex.

The Soil Conservation Service defines the hydrologic soil groups as follows:

- <u>Type A Soils</u>: Soils having a high infiltration rate and low runoff potential when thoroughly wet. These soils consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a moderate rate of water transmission.
- <u>Type B Soils:</u> Soils having a moderate infiltration rate when thoroughly wet and consists mainly of moderately deep to deep, moderately well to well drained soils with moderately fine to moderately course textures. These soils have a moderate rate of water transmission.
- <u>Type C Soils</u>: Soils having a low infiltration rate when thoroughly wet and consists chiefly of soils with a layer that impedes downward movement of water and soils with moderately fine-to-fine texture. These soils have a low rate of water transmission.
- <u>Type D Soils</u>: Soils having a very low infiltration rate and high runoff potential when thoroughly wet. These soils consist chiefly of clays that have high shrink-swell potential, soils that have a permanent high water table, soils that have a clay pan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very low rate of water transmission.

The soils map for the study area is presented in Appendix I, as Figure 2.

#### 5.3 Groundwater

The project site is not located over a primary, principal, or sole source aquifer as per the NYSDEC Division of Water Technical and Operational Guidance, Series (2.1.3), Primary and Principle Aquifer Determinations, Table 1, 1990, and the Atlas of Eleven Selected Aquifers in New York, U.S. Geological Survey in cooperation with the NYS Department of Health, 1982.

Depth to groundwater varies across the site according to Table 17 "Soil and Water Features" of the USDA Soil Conservation Service Soil Survey for Dutchess County as shown in Table 1 "USDA Soil Data".

#### 5.4 Topography

The overall site has varying slopes, with slopes ranging from over 100 percent to nearly level. Site elevations range from over 1100 feet above mean sea level (MSL) to approximately 480 feet above MSL. As previously discussed, the northern end of the site generally slopes southeasterly toward NYS Route 44. Overall, the western portion of the project site is generally higher in elevation than the rest of the project site and slopes toward the central and eastern portions of the project site. Refer to the accompanying plans for more information.

#### 5.5 Wetlands

Wetlands depicted on the accompanying plan set were delineated by The Chazen Companies on May 3, 5, 6, and November 3, 2005. The wetland boundaries were surveyed by The Chazen Companies on June 7, November 3, and December 29, 2005 and are presented on a map entitled "Map of Wetland Survey Prepared for Higher Ground Country Club Management Co." dated January 13, 2006. The majority of these wetlands are either federally or NYSDEC regulated and they encompass approximately 47(+/-) acres of the 668(+/-) acre property. A Wetland Delineation Report, dated January 2006, prepared by The Chazen Companies, has been submitted to the regulatory authorities in pursuit of jurisdictional determinations.

#### **5.6 Surface Waters and Flood Plains**

According to the National Flood Insurance Program Flood Insurance Rate Map (FIRM), Town of Amenia, New York, Community Panel Number 361332 0006 D a small portion of the project site located adjacent to the Amenia Stream lies within Flood Zone AE, an area of "Special Flood Hazard Area Inundated by 100-Year Flood" where base flood elevations have been determined. All other areas of the site appear to be outside the 100-year flood plain.

Numerous wetlands exist on-site, as well as three NYSDEC Class "C" streams. The first Class "C" stream is an unnamed stream that is located in central part of the northern portion of the project site (north of NYS Route 44). This unnamed stream eventually drains to a Class "C<sub>t</sub>" stream known as Amenia Brook (also know as Cascade Brook). Amenia Brook enters the project site south of NYS Route 44, traverses along the eastern property boundary and exits the site near the existing golf course site entrance at NYS Route 22. Another Class "C" stream starts at the outlet of two wetlands (golf ponds) located in the central portion of the site. This unnamed stream flows through Wetland L (refer to accompanying plans) and eventually spills in the Amenia Brook off the project site.

Stormwater runoff from the project site flows to the one of three places; as previously mentioned the northern end of the project site eventually drains to the Amenia Brook. The existing entrance road off of NYS Route 22 and the immediate surrounding areas drain to existing infiltration ponds located at the existing site entrance. Finally the remainder of the project site (central portion, westernsouthwestern portion) drain to the large wetland (labeled as "Wetland L" on the accompanying plans) located in the southeastern portion of the project site.

The Amenia Brook is not a 303(d) listed segment, and it should be noted that the project is located within a Total Maximum Daily Load (TMDL) Watershed (Long Island Sound).

# 5.7 Rainfall Data

Rainfall data utilized in the modeling and analysis was taken from United States Department of Agriculture (USDA) Technical Release 55 (TP-55), Urban Hydrology for Small Watersheds. Rainfall data specific to the portion of Dutchess County under consideration, for various 24 hour storm events, are presented in Table 2:

STORM EVENT	24-HOUR RAINFALL
1-year	2.7 inches
2-year	3.4 inches
10-year	5.0 inches
25-year	5.9 inches
50-year	6.7 inches
100-year	7.0 inches

**Table 2: Rainfall Data** 

The 2-year through the 100-year values were used to evaluate the pre- and postdevelopment stormwater runoff conditions. The 1-year storm was used to ensure that all stormwater management basins meet the NYSDEC stream channel protection requirements.

#### 6.0 EROSION AND SEDIMENT CONTROLS

The SWPPP and accompanying plans identify the temporary and permanent erosion and sediment control measures that have been incorporated into the design of this project. These measures will be implemented during construction, to minimize soil erosion and control sediment transport off-site, and after construction, to control the quality and quantity of stormwater runoff from the developed site.

Erosion control measures, designed to minimize soil loss, and sediment control measures, intended to retain eroded soil and prevent it from reaching water bodies or adjoining properties, have been developed in accordance with the following documents:

- NYSDEC SPDES General Permit for Stormwater Discharges From Construction Activity, Permit No. GP-02-01 (effective January 2003).
- New York State Standards and Specifications for Erosion and Sediment Control, NYSDEC (August 2005).
- New York State Stormwater Management Design Manual, NYSDEC (August 2003).

The SWPPP and accompanying plans outline the construction scheduling for implementing the erosion and sediment control measures. The SWPPP and accompanying plans include limitations on the duration of soil exposure, criteria and specifications for placement and installation of the erosion and sediment control measures, a maintenance schedule, and specifications for the implementation of erosion and sediment control practices and procedures.

#### 6.1 Erosion and Sediment Control Measures

The proposed stormwater management system has been designed to convey stormwater flows off-site via a combination of closed storm sewers, open ditches, stormwater quality and quantity control measures with overflow spillways, thereby preventing erosion and uncontrolled conveyance to the down gradient facilities.

The use of micropool extended detention ponds and wet ponds provides treatment of stormwater runoff and removal of suspended particles.

Temporary and permanent erosion and sediment control measures that shall be applied during construction generally include:

1. Minimizing soil erosion and sedimentation by stabilization of disturbed areas and by removing sediment from construction-site discharges.

- 2. Preservation of existing vegetation as much as possible. Following the completion of construction activities in any portion of the site permanent vegetation shall be established on all exposed soils.
- 3. Site preparation activities shall be planned to minimize the area and duration of soil disruption.
- 4. Permanent traffic corridors shall be established and "routes of convenience" shall be avoided.

# 6.2 Temporary Erosion and Sediment Control Measures

Temporary erosion and sediment control measures are included as part of the construction documents and generally include the following:

1. Stabilized Construction Entrance

Prior to construction, stabilized construction entrances will be installed, as shown on the detail plan, to reduce the tracking of sediment onto public roadways.

Construction traffic must enter and exit the site at the stabilized construction entrance. The intent is to trap dust and mud that would otherwise be carried off-site by construction traffic.

The entrance will be maintained in a condition, which will control tracking of sediment onto public rights-of-way or streets. When necessary, the placement of additional aggregate atop the filter fabric will be done to assure the minimum thickness is maintained. All sediments and soils spilled, dropped, or washed onto the public rights-of-way must be removed immediately. Periodic inspection and needed maintenance will be provided after each substantial rainfall event.

2. Dust Control

Water trucks will be used as needed during construction to reduce dust generated on the site. Dust control must be provided by the general Contractor to a degree that is acceptable to the Owner, and in compliance with the applicable local and state dust control requirements.

#### 3. Temporary Soil Stockpile

Materials, such as topsoil, will be temporarily stockpiled (if necessary) on the site during the construction process. Stockpiles shall be located in an area away from storm drainage, water bodies and/or courses, and will be properly protected from erosion by a surrounding silt fence barrier.

4. Silt Fencing

Prior to the initiation of and during construction activities, a geotextile filter fabric (or silt fence) will be established along the perimeter of areas to be disturbed as a result of the construction which lie up gradient of water courses or adjacent properties. These barriers may extend into non-impact areas to ensure adequate protection of adjacent lands.

Clearing and grubbing will be performed only as necessary for the installation of the sediment control barrier. To ensure effectiveness of the silt fencing, daily inspections and inspections immediately after significant storm events will be performed by site personnel. Maintenance of the fence will be performed as needed.

5. Temporary Seeding

Within 14 days after construction activity ceases on any particular area of the site, all disturbed areas where there will not be construction for longer than 21 days shall be temporarily seeded and mulched to minimize erosion and sediment loss.

6. Stone Inlet Protection Barrier

Concrete blocks surrounded by wire mesh and crushed stone will be placed around both existing catch basins, and proposed catch basins once they have been installed, to keep sediment from entering the catch basins and storm sewer system. During construction, crushed stone shall be replaced as necessary to ensure proper function of the structure.

7. Erosion Control Blanket

Erosion control blankets shall be installed on all slopes exceeding 3:1. Erosion control blankets provide temporary erosion protection, rapid vegetative establishment, and long-term erosion resistance to shear stresses associated with high runoff flow velocities associated with steep slopes.

#### 8. Stone Check Dams

Stone check dams will be installed within drainage ditches to reduce the velocity of stormwater runoff, to promote settling of sediment, and to reduce sediment transport offsite.

The stone check dams will be inspected at least every seven (7) calendar days and within 24 hours of the end of a storm event of ½-inch or greater. Damage will be repaired upon discovery. If significant erosion has occurred between structures, a liner of stone or other suitable material will be installed in that portion of the channel.

Sediment accumulated behind the stone check dam will be removed as needed to allow the channel to drain through the stone check dam and prevent large flows from carrying sediment over or around the dam. Stones shall be replaced as needed to maintain the design cross section of the structures.

9. Temporary Sediment Basin

Temporary sediment basins will be constructed to intercept sediment laden runoff and reduce the amount of sediment leaving the disturbed areas and to protect drainage ways, properties, and rights-of-way.

Temporary sediment basins will be inspected at least every seven (7) calendar days and within 24 hours of the end of a storm event of ½-inch or greater. All damages caused by soil erosion and construction equipment will be repaired upon discovery. Accumulated sediment will be removed from the basin when it reaches 50 percent of the design capacity and shall not exceed 50 percent. Sediment will not be placed downstream from the embankment, adjacent to a stream, or floodplain.

#### **6.3 Permanent Erosion and Sediment Control Measures**

Permanent erosion and sediment control measures are included as part of the construction documents and include the following:

1. Establishment of Permanent Vegetation

Disturbed areas that will be vegetated must be seeded in accordance with the contract documents. The type of seed, mulch, and maintenance measures as described in the contract documents shall also be followed.

All areas at final grade must be seeded and mulched within 14 days after completion of the major construction activity. All seeded areas should be protected with mulch.

Final site stabilization is achieved when all soil-disturbing activities at the site have been completed and a uniform, perennial vegetative cover with a density of 80 percent has been established or equivalent stabilization measures (such as the use of mulches or geotextiles) have been employed on all unpaved areas and areas not covered by permanent structures.

2. Rock Outlet Protection

Rock outlet protection shall be installed at the locations as indicated and detailed on the accompanying plans. The installation of rock outlet protection will reduce the depth, velocity, and energy of water, such that the flow will not erode the receiving water course or water body.

# **6.4 Other Pollutant Controls**

Control of sediments has been described previously. Other aspects of this SWPPP are listed below:

1. Solid Waste Disposal

No solid materials, including building materials, are allowed to be discharged from the site with stormwater. All solid waste, including disposable materials incidental to the major construction activities, must be collected and placed in containers. The containers will be emptied periodically by a contract trash disposal service and hauled away from the site.

Substances that have the potential for polluting surface and/or groundwater must be controlled by whatever means necessary in order to ensure that they do not discharge from the site. As an example, special care must be exercised during equipment fueling and servicing operations. If a spill occurs, it must be contained and disposed so that it will not flow from the site or enter groundwater, even if this requires removal, treatment, and disposal of soil. In this regard, potentially polluting substances should be handled in a manner consistent with the impact they represent.

2. Sanitary Facilities

Temporary sanitary facilities will be provided by the Contractor throughout the construction phase. They must be utilized by all construction personnel and will be serviced by a commercial Contractor. These facilities must comply with state and local sanitary or septic system regulations. 3. Water Source

Non-stormwater components of site discharge must be clean water. Water used for construction, which discharges from the site, must originate from a public water supply or private well approved by the Health Department. Water used for construction that does not originate from an approved public supply must not discharge from the site. It can be retained in the ponds until it infiltrates and evaporates.

4. Long-Term Pollutant Controls

In addition to the permanent stormwater management facilities, identified on the accompanying plans, stormwater pollutant control measures installed during construction that will also provide benefits after construction include temporary sediment basins and rip-rapped outfalls. Temporary sediment basins that do not interfere with normal operations and appear to provide long-term benefits may be left in place after construction is completed, as directed by the Operator.

#### **6.5 Construction Housekeeping Practices**

During the construction phase, the general Contractor will implement the following measures:

- 1. Material resulting from the clearing and grubbing operation will be stockpiled up slope from adequate sedimentation controls.
- 2. The general Contractor will designate areas for equipment cleaning, maintenance, and repair. The general Contractor and subcontractors will utilize those areas. The areas will be protected by a temporary perimeter berm.
- 3. The use of detergents for large scale washing is prohibited (i.e., vehicles, buildings, pavement surfaces, etc.)
- 4. Spill Prevention and Response

A Spill Prevention and Response Plan shall be developed for the site by the Contractor. The plan shall detail the steps needed to be followed in the event of an accidental spill and shall identify contact names and phone numbers of people and agencies that must be notified.

The plan shall include Material Safety Data Sheets (MSDS) for all materials to be stored on-site. All workers on-site will be required to be trained on safe handling and spill prevention procedures for all materials used during construction. Regular tailgate safety meetings shall be held and all workers that are expected on the site during the week shall be required to attend.

5. Material Storage

Construction materials shall be stored in a dedicated staging area. The staging area shall be located in an area that minimizes the impacts of the construction materials effecting stormwater quality.

Chemicals, paints, solvents, fertilizers, and other toxic material must be stored in waterproof containers. Except during application, the contents must be kept in trucks or within storage facilities. Runoff containing such material must be collected, removed from the site, treated and disposed at an approved solid waste or chemical disposal facility.

#### **6.6 Inspection and Maintenance Requirements**

1. Pre-Construction Inspection and Certifications

Prior to the commencement of construction, the Operator's Engineer will conduct an assessment of the site and certify that the appropriate erosion and sediment control structures have been adequately installed and implemented. The Contractor shall contact the Operator's Engineer once the erosion and sediment control measures have been installed.

2. Construction Inspection and Maintenance

To ensure the stability and effectiveness of all protective measures and practices during construction, all erosion and sediment control measures employed will be inspected by the Operator's Engineer at least every seven (7) calendar days and within 24 hours of the end of a storm event of ½-inch or greater. Section 6.7 Subsection 1 "Inspection and Maintenance Reports" outlines what each inspection shall include.

In addition to the inspections performed by the Operator's Engineer, routine inspections shall be performed by the Contractor and include a visual check of all erosion and sediment control measures. All inspections and maintenance will be performed in accordance with the inspection and maintenance schedule provided on the accompanying plans. Sediment removed from erosion and sediment control measures will be exported from the site, stockpiled for later use, or used immediately for general nonstructural fill.

3. Post-Construction Inspection and Maintenance

Inspections shall be performed by the Operator in accordance with Appendix H, when all disturbed areas are stabilized and all stormwater management systems are in place and operable.

# 6.7 Reporting

1. Inspection and Maintenance Reports

Inspection/maintenance reports shall be prepared prior to and during construction in accordance with the schedule outlined herein and in the SPDES General Permit GP-02-01. The reports shall be prepared to identify and document the maintenance of the erosion and sediment control measures.

Specifically, each inspection shall record the following information:

- 1. On a site map, indicate the extent of all disturbed site areas and drainage pathways. Indicate site areas that are expected to undergo initial disturbance or significant site work within the next 14 day period.
- 2. Indicate on a site map all areas of the site that have undergone temporary or permanent stabilization.
- 3. Indicate all disturbed site areas that have not undergone active site work during the previous 14 day period.
- 4. Inspect all sediment control practices and record the approximate degree of sediment accumulation as a percentage of the sediment storage volume (e.g., 10 percent, 20 percent, 50 percent, etc.).

- 5. Inspect all erosion and sediment control practices and record all maintenance requirements such as verifying the integrity of barrier or diversion systems (earthen berms or silt fencing) and containment systems (sediment basins and sediment traps). Identify any evidence of rill or gully erosion occurring on slopes and any loss of stabilizing vegetation or seeding/mulching. Document any excessive deposition of sediment or ponding water along barrier or diversion systems. Record the depth of sediment within containment structures, any erosion near outlet and overflow structures, and verify the ability of rock filters around perforated riser pipes to pass water.
- 6. All deficiencies identified with the implementation of the SWPPP.
- 2. Site Log Book

The Operator shall retain a copy of the SWPPP required by NYSDEC SPDES General Permit GP-02-01 at the construction-site from the date of initiation of construction activities to the date of final stabilization.

During construction, the Operator or Operator's representative shall maintain a record of all erosion and sediment control inspection reports at the site in a log book. The site log book shall be maintained on-site and made available to the permitting authority.

3. Post Construction Records and Archiving

Following construction, the Operator shall retain copies of the SWPPP, the complete construction site log book, and records of all data used to complete the NOI to be covered by this permit, for a period of at least three years from the date that the site is finally stabilized. This period may be extended by the Department, in its sole discretion, at any time upon written notification.

The Operator should maintain a record of all post construction inspections and maintenance work performed in accordance with the requirements outlined in Appendix H.

## 7.0 STORMWATER MANAGEMENT PLAN

The goals of this Stormwater Management Plan are to analyze the peak rate of runoff under pre- and post-development conditions, not to exceed pre-development in order to minimize impacts to adjacent or downstream properties, and to minimize the development's impact on the quality of runoff leaving the site. These objectives will be met by applying Best Management Practices (BMPs) to limit peak runoff rates and treat the stormwater runoff. In addition, both temporary and permanent erosion and sediment control measures will be installed prior to and during construction.

# 7.1 Stormwater Management Systems

Stormwater runoff from the proposed development will be collected and conveyed to the quantity and quality control systems described herein through a network of closed and open conveyances.

The closed stormwater network, consisting of catch basins, drainage manholes, and high density polyethylene piping (HDPE), has been designed to convey the 100-year storm event.

The open stormwater conveyance system, consisting of roadside ditches and HDPE culverts with flared end sections and inlet/outlet protection, has been designed to convey the 50-year storm event.

The following stormwater quantity and quality control systems have been incorporated into the stormwater management plan for this project:

1. Micropool Extended Detention Pond (P-1)

The micropool extended detention pond is an effective means of removing pollutants and will provide a high pollutant removal rate for stormwater runoff. According to the NYSDEC publication *Reducing the Impacts of Stormwater Runoff from New Development*, high pollutant removal from extended detention ponds is primarily attributed to the permanent pool of water that provides gravity settling of sediment, chemical flocculation and biological uptake of pollutants.

Sediment forebays will capture sediment and other trash/debris prior to entering the pond. The pond is landscaped with a variety of plantings including emergents and woody shrubs, with each type of planting corresponding to the water depth. An extended aquatic bench will maximize the biological uptake of pollutants.

The Micropool extended detention pond(s) (P-1) were designed according to the criteria set forth in Section 6.1 "Stormwater Ponds" of the NYS Stormwater Management Design Manual.

# 2. Wet Pond (P-2)

Wet ponds typically consist of two general components - a forebay and a permanent wet pool. The forebay provides pretreatment by capturing coarse sediment particles in order to minimize the need to remove the sediments from the primary wet pool. The wet pool serves as the primary treatment mechanism and where much of the retention capacity exists.

When sized to store the water quality volume, a pond system will retain all of the water from storms that generate runoff less than or equal to the water quality volume and result in a significantly increased period of time available for treatment. For storms that generate runoff greater than the water quality volume, wet ponds still provide a reduced level of treatment through conventional settling and filtration for the additional runoff volume that is conveyed through the pond. When properly designed, the permanent pool reduces the velocity of incoming water to prevent resuspension of particles and promote settling of newly introduced suspended solids. The energy dissipating and treatment properties of the permanent pool are enhanced by aquatic vegetation, which is an essential part of the stormwater pond design.

The wet ponds (P-2) were designed according to the criteria set forth in Section 6.1 "Stormwater Ponds" of the *NYS Stormwater Management Design Manual*. Design Calculations have been provided in Appendix L.

# 7.2 Hydrologic and Hydraulic Analysis

This report presents the pre-development and post-development features and conditions associated with surface water runoff within the study area. For both cases, the drainage patterns, drainage structures, soil types, and ground cover types are considered in this study.

1. Methodology

The methodology used for the hydrologic and hydraulic analysis was obtained from the United States Department of Agriculture (USDA) Soil Conservation Service's (SCS) Technical Release No. 20, as utilized by the application program HydroCAD. HydroCAD, developed by Applied Microcomputer Systems of Chocorua, New Hampshire, is a Computer-Aided-Design (CAD) program for analyzing the hydrologic and hydraulic characteristics of a given watershed and associated stormwater management facilities. HydroCAD has the capability of computing hydrographs (which represent discharge rates characteristic of specified watershed conditions and precipitation), combining hydrographs and routing flows though pipes, streams and ponds. Documentation for HydroCAD can be found on their website: <u>http://www.hydrocad.net/</u>.

For this analysis, the watershed and drainage system was broken down into a network consisting of three types of components as described below:

- 1. Subcatchment: A relatively homogeneous area of land, which produces a volume and rate of runoff unique to that area.
- 2. Reach: Uniform streams, channels or pipes that convey stormwater from one point to another.
- 3. Pond: Natural or man-made impoundment, which temporarily stores stormwater runoff and empties in a manner determined by its geometry and the hydraulic structure located at its outlets.

Subcatchments, reaches, and ponds are represented by hexagons, squares, and triangles respectively, on the watershed routing diagrams provided with the computations included in Appendix J and Appendix K.

2. Analysis

The analysis of hydrologic and hydraulic conditions and proposed stormwater management facilities, servicing the study area, was performed by dividing the tributary watershed into relatively homogeneous sub-catchments. The separation of the watershed into sub-catchments was dictated by watershed conditions, methods of collection, conveyance, and points of discharge. Watershed characteristics for each subcatchment were then assessed from United States Geological Service (USGS) 7.5-minute topographic maps, aerial photographs, a topographical survey, soil surveys, site investigations, and land use maps.

Proposed stormwater management facilities were designed and evaluated in accordance with the *NYS Stormwater Management Design Manual* and local regulatory requirements. The hydrologic and hydraulic analysis considered the SCS, Type III 24-hour storm events identified in Table 3.

Facility	24 Hour Storm Event			
Storm Culverts	50 year			
Stormwater Collection and Conveyance System to Stormwater Management Facilities	100 year			
	2 year			
Detention Basin (pond)	10 year			
	25 year			
	50 year			
	100 year			
Flood Conditions	100 year			

#### **Table 3: Design Events**

3. Study Area and Design Points (DP)

The study area consists of an overall watershed that encompasses approximately 776 acres and contains the entire 668-acre project site. The overall watershed was broken down into smaller watersheds, or subcatchments, to allow for analysis of runoff conditions at several locations throughout the study area. Each of these locations was defined as a Design Point (DP) in order to evaluate the effects of the project on the watershed hydrology. Descriptions of each of the selected design points are provided below.

- <u>Design Point 1</u>: A low area located adjacent to a utility easement within western half of the north portion of the project site (north of New York State Route 44). This low area is drained by a 36-inch Corrugated Metal Pipe (CMP) which flows under NYS Route 44 and discharges back onto the project site south of NYS Route 44. A total of 27-acres drain to this design point in a southeasterly direction.
- <u>Design Point 2</u>: A low area located within the NYS R.O.W. adjacent to NYS Route 44 at the southeastern side of the northern portion of the project site (north of NYS Route 44). This low area receives the waters from a NYSDEC Class "C" stream and is drained by a 24-inch reinforced concrete pipe (RCP). This 24-inch RCP flows under NYS Route 44 and discharges back onto the project site south of NYS Route 44. A total of 98-acres drain to this design point in a southeasterly direction.
- <u>Design Point 3:</u> The entrance to a 12-foot by 12-foot box culvert located within NYS Route 22 which conveys the Amenia Creek (also known as Cascade Brook) off the project site. This culvert is located within the NYS R.O.W. adjacent to NYS Route 22 on the eastern project site property boundary. A total of 213-acres drain to this design point in an easterly direction.

- <u>Design Point 4</u>: The outlet of "Wetland L" located within the NYS R.O.W. adjacent to NYS Route 22 along the eastern project site property boundary. The outlet associated with this wetland is a 30-inch CMP which crosses NYS Route 22 and discharges easterly offsite. A total of 489-acres drain to this design point in an easterly direction.
- <u>Design Point 5:</u> A low area located within the NYS R.O.W. adjacent to NYS Route 22. This area is off the project site near the southeast corner of the project property. The low area is drained by a culvert pipe, which crosses NYS Route 22 and discharges easterly. A total of 28-acres drain to this design point in an easterly direction.

# 7.3 Pre-Development Watershed Conditions

The existing project site is covered predominantly by open meadows, grass, wetlands, woods, as well as an existing 18-hole golf course, golf course club house and associated amenities. Analysis of pre-development conditions considered existing drainage patterns, soil types, ground cover, and topography. The Pre-Development Watershed Delineation Map has been provided in Appendix I as Figure 3. The results of the computer modeling used to analyze the overall watershed under pre-development conditions are presented in Appendix J. A summary of the pre-development discharge rates is presented in Table 4.

# 7.4 Post-Development Watershed Conditions

The analysis of post-development conditions considered existing drainage patterns, soil types, ground cover to remain, planned site development, site grading, and stormwater management facilities proposed as part of site improvements. The Post-Development Watershed Delineation Map has been provided in Appendix I as Figure 4. The results of the computer modeling used to analyze the overall watershed under post-development conditions are presented in Appendix K. A summary of the post-development discharge rates is presented in Table 4.

# 7.5 Hydrologic and Hydraulic Calculations

Comparison of pre- and post-development watershed conditions demonstrates that the peak rate of runoff from the proposed site will not be increased at all design points therefore will not pose a significant adverse impact to the adjacent or downstream properties or receiving water courses. Table 4 "Summary of Pre- and Post-Development Peak Discharge Rates" summarizes the results of the analyses for such comparison.

Pre- vs. Post-Development Discharge Rate (cfs)											
Design Point (DP)	2 year 24 hr storm event		10 year 24 hr storm event		25 year 24 hr storm event		50 year 24 hr storm event		100 year 24 hr storm event		
	Pre	Post	Pre	Post	Pre	Post	Pre	Post	Pre	Post	
1	12.00	10.95	29.17	26.58	40.11	36.10	50.31	47.25	54.23	51.27	
2	25.54	24.04	69.51	65.20	98.45	92.09	125.74	117.46	136.27	127.33	
3	121.33	121.12	195.74	192.28	249.61	242.90	300.14	289.71	319.24	307.06	
4	14.57	14.96	26.91	25.67	33.57	32.31	37.28	36.39	38.56	37.64	
5	16.05	16.05	37.09	37.09	50.24	50.24	62.42	62.42	67.08	67.08	

Table 4: Summary of Pre- & Post-Development Peak Discharge Rates

The results of the computer modeling used to analyze the stormwater management system under pre- and post-development conditions are presented in Appendix J and Appendix K, respectively.

# 7.6 Proposed Water Quantity and Quality Controls

1. Water Quantity Controls

The proposed stormwater quantity controls will include open detention basins.

The proposed quantity controls have been designed and sized to provide channel protection ( $C_{p_v}$ ), overbank flood control ( $Q_{p_{10}}$ ) and extreme flood protection ( $Q_{f_{100}}$ ), where:

- Channel Protection Volume  $(C_{p_v})$  requirements are designed to protect stream channels from erosion. This is accomplished by providing 24 hours of extended detention of the 1-year 24-hour storm event, provided that this can be accomplished utilizing an outlet orifice sized to prevent clogging.
- Overbank Flood Control Volume  $(Q_{p_{10}})$  requirements are designed to prevent flow events that exceed the bankfull capacity of a channel, and therefore must spill over into the floodplain. This requires storage to assure that the post-development 10-year 24-hour peak discharge rates do not exceed pre-development rates.

Page 34

• Extreme Flood Protection Volume  $(Q_{f_{100}})$  requirements are designed to prevent the increased risk of flood damage from large storm events, maintain the boundaries of pre-development 100-year floodplain, as well as to protect the physical integrity of the stormwater management practices. This requires storage to assure that the post-development 100-year 24-hour peak discharge rates do not exceed pre-development rates.

There are numerous locations and methods for providing controls of off-site discharge of stormwater. Each proposed stormwater management basin has been designed to provide the above quantity controls by attenuating stormwater runoff from several storm events (up to and including the 100year event) to ensure that the discharge rate at each design point is equal to or less than the rate that existed prior to development of the site. The performance of each stormwater management basin during each of the design storms can be found in Appendix K of this report.

2. Water Quality Controls

Stormwater runoff from impervious surfaces is recognized as a significant contributor of pollution that can adversely affect the quality of the receiving water bodies. Therefore, treatment of stormwater runoff is important since most runoff related water quality contaminants are transported from land, particularly the impervious surfaces, during the initial stages of storm events.

The proposed water quality  $(WQ_v)$  controls have been sized based on the 90% rule methodology as described in Table 4.1 "New York Stormwater Sizing Criteria" of the *NYS Stormwater Management Design Manual* dated August 2003. The water quality  $(WQ_v)$  control is defined as:

$$WQ_v = \frac{[(P)(R_v)(A)]}{12}$$

Where:

Utilizing the above design equation, each of the devices has been sized accordingly to provide the required water quality volume  $(WQ_v)$  for its contributing drainage area. Design computations for the Stormwater Quality Control Components for each basin are presented in Appendix L.

# 8.0 CONCLUSION

The Chazen Companies have completed a Master Stormwater Pollution Prevention Plan for the planned Silo Ridge Country Club Resort Community. The intent of this Master SWPPP was to provide sufficient documentation for an overall SEQR determination, and to serve as the baseline for the final SWPPP that will be prepared for the proposed development, as approved.

The analyses included the review of watershed conditions, hydrologic and hydraulic analysis using computer modeling, and an evaluation of the proposed improvements across the subject site. The plan allows for the maintenance of existing drainage patterns while continuing the conveyance of stormwater runoff from upland watershed areas.

The plan controls increases in the stormwater rate of runoff resulting from the proposed development without adversely affecting downstream conditions. This is demonstrated by comparing pre- and post-development flows for various storm events. Table 4 "Summary of Pre- and Post-Development Peak Discharge Rates" summarizes the results of the analyses for such comparison.

The comparison of pre- and post-development watershed rate of runoff demonstrates that off-site peak flow conditions at the design points will pose no significant adverse impacts to the adjacent or downstream properties or receiving water courses.

The proposed stormwater collection system consisting of pipes, open drainage ways and on-site stormwater management facilities will adequately collect, treat, and convey the stormwater. Stormwater quality will be enhanced through the implementation of the proposed stormwater management facilities, erosion and sediment control measures and maintenance practices outlines herein.

In conclusion, it is our opinion that the proposed development will not adversely impact adjacent or downstream properties if the stormwater management facilities are properly constructed, and maintained in accordance with the requirements outlined herein.

# Appendix A: NYSDEC SPDES General Permit GP-02-01



#### NEW YORK STATE DEPARTMENT OF ENVIRONMENTAL CONSERVATION

#### SPDES GENERAL PERMIT FOR STORMWATER DISCHARGES

from

#### CONSTRUCTION ACTIVITY

Permit No. GP-02-01

Issued Pursuant to Article 17, Titles 7, 8 and Article 70 of the Environmental Conservation Law

Effective Date: January 8, 2003

#### Expiration: January 8, 2008

William R. Adriance Chief Permit Administrator Address:

NYS DEC Div. Environmental Permits 625 Broadway, 4th Floor Albany, N.Y. 12233-1750

Authorized Signature William R. Adriance

Date:

January 8, 2003

SPDES General Permit for Stormwater Runoff from Construction Activity, GP-02-01

Expiration: January 8, 2008

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## NEW YORK STATE DEPARTMENT OF ENVIRONMENTAL CONSERVATION SPDES GENERAL PERMIT FOR STORMWATER DISCHARGES FROM CONSTRUCTION ACTIVITY

#### **Preface**

Pursuant to Section 402 of the Clean Water Act ("CWA"), stormwater discharges from certain construction activities to waters of the United States<sup>1</sup> are unlawful unless they are authorized by a NPDES (National Pollutant Discharge Elimination System) permit or by a state permit program. New York's SPDES (State Pollutant Discharge Elimination System) is a NPDES-approved program with permits issued in accordance with the Environmental Conservation Law ("ECL"). Discharges of pollutants to all other "Waters of New York State" such as groundwaters are also unlawful unless they are authorized by a SPDES permit.

A discharger, owner, or operator may<sup>2</sup> obtain coverage under this general permit by submitting a Notice of Intent ("NOI") to the Department. Copies of this General Permit and the NOI for New York are available by calling (518) 402-8109 or at any Department of Environmental Conservation (the Department) regional office (see Appendix A on Page 23). They are also available on the Department's website at:

#### www.dec.state.ny.us

"Waters of the United States" means:

(a) All waters which are currently used, were used in the past, or may be susceptible to use in interstate or foreign commerce, including all waters which are subject to the ebb and flow of the tide; and

(b) All interstate waters, including interstate "wetlands"; and

(c) All other waters such as interstate lakes, rivers, streams (including intermittent streams), mudflats, sandflats, wetlands, sloughs, prairie potholes, wet meadows, playa lakes, or natural ponds the use, degradation, or destruction of which would affect or could affect interstate or foreign commerce including any such waters:

- (1) Which are or could be used by interstate or foreign travelers for recreational or other purposes; or
- (2) From which fish or shellfish are or could be taken and sold in interstate or foreign commerce; or
- (3) Which are used or could be used for industrial purposes by industries in interstate commerce; and
- (d) All impoundments of waters otherwise defined as waters of the United States under this definition; and
- (e) Tributaries of waters identified in paragraphs (a) through (d) of this definition; and
- (f) The territorial sea; and

Waste treatment systems, including treatment ponds or lagoons designed to meet the requirements of the CWA are not waters of the United States. This exclusion applies only to manmade bodies of water which neither were originally created in waters of the United States (such as disposal areas in wetlands) nor resulted from the impoundment of waters of the United States.

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<sup>2</sup> "may" refers to circumstances under which the discharger is ineligible for coverage under this general permit because of other provisions of this permit. Dischargers which are excluded from coverage under this general permit as provided for in Part I, Section C, for example, are not authorized to discharge under this permit. This also applies to possible situations in which an NOI has been submitted and/or a regulatory fee paid pursuant to Article 72 of the ECL. The submittal of an NOI and/or regulatory fee has no bearing or relevance whatsoever on the eligibility of the construction activity discharging stormwater runoff under the authority of this permit.

<sup>(</sup>g) Wetlands adjacent to waters (other than waters that are themselves wetlands) identified in paragraphs (a) through (f) of this definition.

#### Local Programs of a Regulated MS4

Under the federal Phase II stormwater program, many cities, villages, towns, and other public entities in New York State which are located within "Urbanized Areas" as defined by the U.S. Census and who operate a Municipal Separate Storm Sewer System ("MS4") will be required to obtain SPDES permit coverage for stormwater discharges under their jurisdiction and control (see 40CFR Part 122 §122.26.32). Additionally, MS4s may be designated by the Department as regulated MS4s. Among other requirements, the Phase 2 NPDES stormwater regulations require regulated MS4s to address stormwater runoff from construction activities. Construction activities covered under this general permit, which are subject to stormwater runoff controls of a regulated MS4, will also need to comply with the MS4's controls.

## Five (5) Day Coverage

Prior to the submission of an NOI, the owner or operator must have completed a Storm Water Pollution Prevention Plan (SWPPP) that complies with all requirements of this general permit. Submitting an NOI is an affirmation that a SWPPP has been prepared and will be implemented. If an applicant certifies that the SWPPP has been developed in conformance with the Department's technical standards, the applied-for activity may obtain coverage under this general permit in five (5) business days after the Department's receipt of the NOI provided, that the activity is eligible for coverage under this general permit and that the Department has not informed the applicant otherwise.

## Sixty (60) Day Coverage

While the Department's technical standards are appropriate statewide, it is recognized that there may be situations where stormwater management goals can best be met by alternative means that are more suitable given local conditions.

For construction projects in these situations, applicants must identify in their NOI each of the deviations from the Department's technical standards that they are seeking. Applicants must also explain why the deviations are needed or desired and what impacts to water quality, if any, can be expected if the deviation were allowed. Applicants must also explain the actions, if any, that local board(s) have taken with respect to the deviation(s). For applicants which cannot certify conformance with the Department's technical standards, the SWPPP must also be certified by a licensed/certified professional that the SWPPP has been developed in a manner which will insure compliance with water quality standards and with the substantive intent of this permit.

In cases of deviations from the Department's technical standards, applicants must allow sixty (60) business days after the receipt by the Department of a completed NOI and certification before gaining coverage under this general permit and before initiating any construction activity. During this 60 day period, the Department may conduct further review of the NOI and SWPPP. If additional information is needed to complete the review, the NOI will be considered incomplete and the applicant will be so advised. The intent of this provision is to require conformance the Department's technical standards wherever possible and appropriate. At the same time, alternative means to address stormwater control may be allowed under this general permit where they are more suitable for the site in question and where they will not diminish water quality protection.

There are other scenarios under which coverage under this general permit will not occur until 60 business days from the receipt of a completed NOI. For example, if the construction activity or post construction runoff causes the discharge of a pollutant of concern to a water identified on the 303(d) list or a watershed with an approved TMDL for that pollutant of concern, coverage under the general permit will not occur until sixty (60) business days from the receipt by the Department of a completed NOI. For these projects the operator may be required to submit the SWPPP and/or appropriate certification(s) to the Department for review. The flowchart shown in Figure 1 on page vi will help to describe the process under which certain conditions exist that require possible further analysis and water quality/quantity considerations.

#### **Computer Tool Available For Completion of SWPPPs and NOIs Under Development**

The Department is currently developing an interactive computer software tool entitled "How to Prepare SWPPPs and Notices of Intent" to assist applicants in both developing SWPPPs and completing NOIs. This will be available in the near future for use on the Department website as well as being packaged independently on compact discs. This tool will contain guidance as well as many useful links to reference materials and documents concerning erosion and sedimentation control, as well as to the design of stormwater management practices . The Department's website will contain the latest information and guidance on the various tools available.

## **The Department's Technical Standards**

The Department's technical standards for erosion and sediment control are contained in the document, "*New York Standards and Specifications for Erosion and Sediment Control*"<sup>3</sup> published by the Empire State Chapter of the Soil and Water Conservation Society. For the design of water quantity and water quality controls (post-construction stormwater control practices), the Department's technical standards are detailed in the "*New York State Stormwater Management Design Manual*." Both of these documents are available on the Department's website. If an applicant certifies that stormwater management practices will conform to the Department's technical standards, then coverage under the permit may occur sooner than otherwise would be the case if non-conformance with the manuals existed. See Figure 1 on page vi for more information.

<sup>&</sup>lt;sup>3</sup> Previously, the *"New York Guidelines for Urban Erosion and Sediment Control"*, also commonly referred to as the "Blue Book".

SPDES General Permit for Stormwater Runoff from Construction Activity, GP-02-01

#### Permit Valid for Any Size Disturbance

This permit may be used for construction activities involving any amount of disturbed acreage, provided that all other eligibility conditions in subsection B of Part I are satisfactorily met (see page 2 of this permit). Thus, this permit may apply to activities identified under 40 CFR Part 122, subsection 122.26(b)(14)(x) which are also referred to as "NPDES Phase 1 construction activities" involving soil disturbances of five (5) acres or more. This permit may also apply to activities identified under 40 CFR Part 122, subsection 122.26(b)(15) which are also referred to as "NPDES Phase 2 small construction activities" involving soil disturbances of between one (1) and five (5) acres. And, this permit may also apply to construction activities involving soil disturbances of less than one (1) acre if the Department determines that a SPDES permit is required pursuant to the ECL. In any and all cases, all of the eligibility provisions of this general permit must be met in order to gain coverage.

## **Notice of Termination**

After construction is completed as defined in the general permit (see Part II beginning on Page 7), cancellation of coverage is accomplished by the submittal of a Notice of Termination ("NOT"). Failure to submit a NOT may result in the continued obligation to pay a yearly Regulatory Fee established pursuant to Article 72 of the ECL and/or may be cause for suspension of permit coverage.

Previous versions of NOIs, NOTs and Notices of Intent, Transfer and Termination ("NOITT"s) cannot be used in conjunction with this general permit. There is a new NOI required for obtaining coverage under this general permit. Failure to include information identified as "mandatory" entries on the new NOI form may prevent and/or delay discharge authorization being sought under this permit.

The new NOT will also include an identification of any permanent structures that are being left on the site after stabilization occurs and after termination of permit coverage under this general permit. The NOT will also include a certification that the structures were constructed as described in the SWPPP and that an Operation and Maintenance ("O&M") manual has been prepared and has been made available to the owner of such permanent structures who is expected to conduct the necessary O&M over the life of the structure(s).

## **Ineligible Activities**

The submittal of a completed NOI and/or the payment of an annual regulatory fee by an applicant does not necessarily mean that an applicant is covered under this permit if the applicant is ineligible for coverage under this permit under the terms cited in Part I of this permit. In other words, submitting a completed NOI and paying an annual regulatory fee does not automatically gain an applicant permit coverage if the applicant is ineligible for coverage under this permit even if the Department fails to immediately inform the applicant of such ineligibility.

## Permit Expiration Date

Coverage under this general permit is available January 8, 2003 and will expire five (5) years after issuance on January 8, 2008.

#### Activities Previously Covered Under GP-93-06

In a separate proposal, the Department is also concurrently seeking to re-issue GP-93-06 with an expiration of August 1, 2003. The purpose of this action is to provide a transition period for permittees which have had SPDES permit coverage under GP-93-06 immediately prior to January 8, 2003, the effective date of GP-02-01. **Prior to August 1, 2003**, these activities will need to:

(1) stabilize their sites in accordance with GP-93-06 and submit an NOT; or, if necessary,(2) gain coverage under GP-02-01 by submitting a new NOI.

For <u>**new**</u> construction activities, coverage under GP-93-06 will not be available after the effective date of GP-02-01, January 8, 2003. Such discharges may be eligible for coverage under GP-02-01 (see Part I.B. on page 2 of this permit).

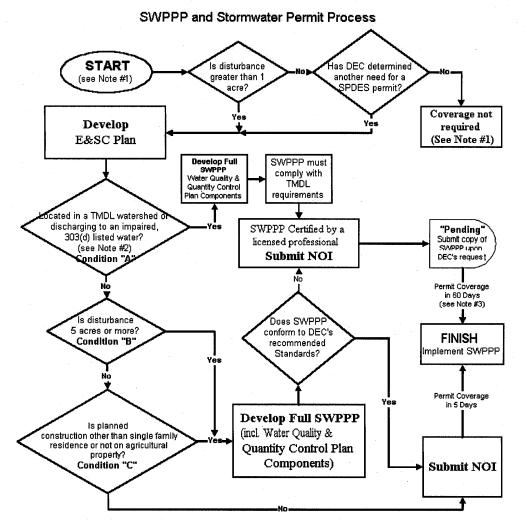
#### Water Quality Violations Not Permitted

This permit does not authorize any person to cause or contribute to a condition in contravention of any water quality standards that are contained in the Rules and Regulations of the State of New York (see Part I of this permit on page 2) even if the permittee is in compliance with all other provisions of this permit. Any violations of water quality standards may be considered by the Department to be violations of this permit and/or the ECL, including its accompanying regulations.

#### **Other Department Permits**

Construction activities may also require other Department permits in addition to the coverage provided by this general permit including, but not limited to, dam safety, wetlands and stream protection. Such other Department permits must be obtained separately from coverage under this general permit. Further information concerning these permits should be sought from the Regional Permit Administrator at the appropriate Department regional office (See Appendix A on page 23).

#### FIGURE 1



#### NOTES:

1. Under any of the above conditions other environmental permits may be required. DEC may require permit for construction disturbance < 1 acre on a case by case basis.

2. <u>and</u> the following exists: construction and/or stormwater discharges from the construction or post-construction site contain the pollutant of concern identified in the TMDL or 303(d) listing.

3. After receipt by DEC of completed application.

SPDES General Permit for Stormwater Runoff from Construction Activity, GP-02-01 Page vii Expiration: January 8, 2008

#### NEW YORK STATE DEPARTMENT OF ENVIRONMENTAL CONSERVATION SPDES GENERAL PERMIT FOR STORMWATER DISCHARGES

# FROM CONSTRUCTION ACTIVITIES

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# Part I. COVERAGE UNDER THIS PERMIT

A. <u>Maintaining Water Quality</u> - It shall be a violation of this general permit and the Environmental Conservation Law ("ECL") for any discharge authorized by this general permit to either cause or contribute to a violation of water quality standards as contained in Parts 700 through 705 of Title 6 of the Official Compilation of Codes, Rules and Regulations of the State of New York including, but not limited to:

1. There shall be no increase in turbidity that will cause a substantial visible contrast to natural conditions;

2. There shall be no increase in suspended, colloidal and settleable solids that will cause deposition or impair the waters for their best usages; and

3. There shall be no residue from oil and floating substances, nor visible oil film, nor globules of grease.

# B. <u>Eligibility Under This General Permit</u>

1. This permit may authorize all discharges of stormwater from construction  $activity^4$  to surface waters and groundwaters except for ineligible discharges identified under subparagraph C of this Part (see below). Discharge authorization under this permit requires the submittal of a completed NOI.

2. Except for non-stormwater discharges explicitly listed in the next paragraph, this permit only authorizes stormwater discharges from construction activities.

3. Notwithstanding paragraphs B.1 and B.2 above, the following nonstormwater discharges may be authorized by this permit: discharges from fire

<sup>&</sup>lt;sup>4</sup> This includes discharges of stormwater associated with industrial activity identified under 40 CFR Part 122, subsection 122.26(b)(14)(x), small construction activities identified under 40 CFR Part 122, subsection 122.26(b)(15) or any other stormwater from construction activities that are not otherwise ineligible for coverage under this permit (See Part I, subsection B beginning on page 2).

fighting activities; fire hydrant flushings; waters to which cleansers or other components have **not** been added that are used to wash vehicles or control dust in accordance with the SWPPP, routine external building washdown which does not use detergents; pavement washwaters where spills or leaks of toxic or hazardous materials have not occurred (unless all spilled material has been removed) and where detergents are not used; air conditioning condensate; springs; and foundation or footing drains where flows are not contaminated with process materials such as solvents. For those entities required to obtain coverage under this general permit, and who discharge as noted in this paragraph, and with the exception of flows from fire fighting activities, these discharges must be identified in the SWPPP(see Part III beginning on Page 7). Under all circumstances, the permittee must still comply with water quality standards (see Part I, subsection A on Page 2).

C. <u>Activities Which Are Ineligible for Coverage Under This General Permit</u> - All of the following stormwater discharges from construction activities are <u>not</u> authorized by this permit:

1. Discharges after construction activities have been completed and the site has undergone final stabilization<sup>5</sup>;

2. Discharges that are mixed with sources of non-stormwater other than those expressly authorized under subsection B.3. of this Part (see page 3) and identified in the SWPPP required by this permit;

3. Discharges that are subject to an existing SPDES individual or general permit or which are required to obtain an individual or alternative general permit pursuant to Part V, subparagraph K (see page 21) of this permit;

4. Discharges that are likely to adversely affect a listed, or proposed to be listed, endangered or threatened species, or its critical habitat;

5. Discharges which are subject to an existing effluent (limitation) guideline addressing stormwater and/or process wastewater unless said guidelines are contained herein; or

6. Discharges which either cause or contribute to a violation of water quality standards adopted pursuant to the ECL and its accompanying regulations (See subsection A of Part I on page 2).

<sup>&</sup>lt;sup>5</sup> "Final Stabilization" means that all soil disturbing activities at the site have been completed, and that a uniform perennial vegetative cover with a density of 80% has been established or equivalent stabilization measures (such as the use of mulches or geotextiles) have been employed on all unpaved areas and areas not covered by permanent structures.

# D. <u>Authorization Under This General Permit</u>

1. An operator<sup>6</sup> must submit a completed NOI form in order to be authorized to discharge under this general permit. The NOI form shall be one which is associated with this general permit, signed in accordance with Part V. H.(see Page 19) of this permit and submitted to the address indicated on the NOI form. NOIs and NOITTs used in association with either previous or other general permits are not valid for obtaining coverage under this general permit. The submittal of an NOI is an affirmation to the operators' understanding and belief that the activity is eligible for coverage under this permit and that a SWPPP has been prepared and will be implemented in accordance with Part III of this permit.

2. All contractors and subcontractors of the operator identified under Part III.E.1 (see page 17) must provide the certification cited under Part III.E.2 (see page 17). Such certifications shall become part of the SWPPP for the construction activity covered under this general permit.

3. Unless notified by the Department to the contrary, operators who are eligible for coverage under this permit **and** who submit an NOI in accordance with the requirements of this permit, may be authorized to discharge stormwater from construction activities under the terms and conditions of this permit, and in accordance with the following timetable:

- a. For construction activities which:
  - develop a SWPPP in conformance with the Department's technical standards (See subsection D of Part III on page 10), and do not or will not discharge a pollutant of concern to an impaired water or a TMDL watershed;
  - or
  - (2) as of the effective date of this general permit, GP-02-01, have obtained coverage under, and are operating in compliance with, GP-93-06; and do not or will not discharge a pollutant of concern to an impaired water or a TMDL watershed;

authorization to discharge under this permit may occur <u>five (5) business</u> days after the date on which the NOI is received by the Department.

<sup>6</sup> For the purposes of this permit, the term "operator" means the person, persons, or legal entity which owns or leases the property on which the construction activity is occurring. Also, see Part V., subsection H. on page 19 of this permit.

b. For activities which do not comply with the preceding subsection (i.e. Part I.D.3.a.), authorization to discharge under this permit will begin no sooner than <u>sixty (60)</u> business days from the receipt of the completed NOI unless notified differently by the Department pursuant to Part V, subsection K of this permit (see page 21). For activities not satisfying Part I.D.3.a.(1) above, or for construction site runoff subject to a TMDL (see Figure 1 on page vi), the SWPPP must be prepared by a licensed/certified professional<sup>7</sup> and include a certification stating that the SWPPP has been developed in a manner which will assure compliance with water quality standards (see Part I.A.) and with the substantive intent of this permit.

c. For construction activities which are subject to a sixty-day period provision identified in the preceding subparagraph b., the SWPPP shall include each of the components identified in Part III.A.1.b. (see page 8).

4. At its sole discretion, the Department may deny or terminate coverage under this permit and require coverage under another SPDES permit at any time based on a review of the NOI, the SWPPP or other relevant information (see Part V, subsection K of this permit on page 21).

5. A copy of the NOI and a brief description of the project shall be posted at the construction site in a prominent place for public viewing.

6. A signed copy of the NOI, the SWPPP, and any reports required by this permit shall also be submitted concurrently to the local governing body and any other authorized agency<sup>8</sup> having jurisdiction or regulatory control over the construction project.

7. New stormwater discharges from construction activities that require any other Uniform Procedures Act permit (Environmental Conservation Law, 6 NYCRR Part 621) cannot be covered under this general permit until the other required permits are obtained. Upon satisfaction of the State Environmental Quality Review Act ("SEQRA") for the proposed action and issuance of necessary permits, the applicant may submit an NOI to obtain coverage under this general

<sup>&</sup>lt;sup>1</sup> A "licensed/certified professional" means a person currently licensed to practice engineering in New York State or is a Certified Professional in Erosion and Sediment Control (CPESC).

<sup>8</sup> For the purposes of this general permit, "any other authorized agency" shall include any local, regional, or state entity or agency except the Department which has authority to review stormwater discharge from the project, including authority under any approved watershed protection plan or regulations.

permit.<sup>9</sup> In order to facilitate the Department's review of a multi-permitted project, an applicant should submit, at a minimum, a copy of the SWPPP which contains the information specified in Appendix B (see page 24). This information will assist the Department in determining whether or not coverage under this general permit or another SPDES permit is the more appropriate option. The Department may also require the submission of additional information in order to determine the SWPPP's conformance with the Department's technical standards.

8. Upon renewal of this general permit or issuance of a new general permit, the permittee is required to notify the Department of its intent to be covered by the new general permit. Coverage will continue under this permit for its term unless action is taken to terminate permit coverage as provided elsewhere in this permit. See also Part V. subsection B. on page 18 of this permit.

9. In the event of a transfer of ownership or responsibility for stormwater runoff, there can be no "automatic" transfer of permit coverage from one permittee to the next without appropriate notification from the dischargers. The former permittee must submit an NOT and notify the new discharger of the possible need for the new discharger to submit a new NOI (see Section E, subparagraph 2 below).

## E. **Deadlines for Notification**

1. Operators who intend to obtain coverage under this general permit for stormwater runoff from construction activities must submit an NOI in accordance with the requirements of this Part at least five (5), or sixty (60) business days, as appropriately determined from Part I, Section D.3 (see page 4) prior to the commencement of construction<sup>10</sup> activities.

2. For stormwater runoff from construction activities where the operator changes, a new NOI must be submitted by the new operator in accordance with the requirements of this permit. The former operator must submit a NOT in accordance with Part II (see page 7) of this permit and notify the new operator of the requirement to submit a new NOI to obtain coverage under this permit. The new operator must also review and sign the SWPPP in accordance with Part III.B.(see page 9) and continue implementation of the SWPPP as required by this

<sup>&</sup>lt;sup>9</sup> The purposes of this subsection is to assure that the requirements of SEQRA are fulfilled, if necessary, before any discharge authorization under this general permit is granted.

<sup>&</sup>lt;sup>10</sup> "Commencement of Construction" means the initial disturbance of soils associated with clearing, grading, or excavating activities, or other construction activities.

permit.

#### Part II. TERMINATION OF COVERAGE<sup>11</sup>

Where a site has been finally stabilized, the operator must submit a NOT form prescribed by the Department for use with this general permit. The NOT shall be signed in accordance with Part V. H.(see page 19) of this permit and submitted to the address indicated on the approved NOT form.

The permittee must identify all permanent stormwater management structures that have been constructed and provide the owner(s) of such structures with a manual describing the operation and maintenance practices that will be necessary in order for the structure to function as designed after the site has been stabilized. The permittee must also certify that the permanent structure(s) have been constructed as described in the SWPPP.

## Part III. STORMWATER POLLUTION PREVENTION PLANS ("SWPPP"s)

#### A. General

#### 1. SWPPP Preparation

A SWPPP shall be developed by the operator for construction a. activities at each site to be covered by this permit, prior to the initiation of activities requiring coverage under this permit. SWPPPs shall be prepared in accordance with sound engineering practices. The SWPPP shall identify potential sources of pollution which may reasonably be expected to affect the quality of stormwater discharges. In addition, the SWPPP shall describe and ensure the implementation of practices which will be used to reduce the pollutants in stormwater discharges and to assure compliance with the terms and conditions of this permit. Operators are encouraged to have their SWPPP reviewed for adequacy and completeness by the local soil and water conservation district ("SWCD") and/or other professionals qualified in erosion and sediment control practices<sup>12</sup> and stormwater management. Moreover, if the construction activity is identified under Part I, subsection D.3.b. (See page 5), or for construction site runoff subject to a TMDL (see Figure 1 on page vi), the SWPPP must include a certification by a licensed/certified professional.

<sup>&</sup>lt;sup>11</sup> Submittal of an NOT will terminate coverage under this general permit and will also remove the permittee from subsequent billings of the annual regulatory fee levied under Article 72 of the ECL.

<sup>&</sup>lt;sup>12</sup> For example, CPESC, Inc. administers a certified program of individuals under its CPESC (Certified Professional in Erosion and Sediment Control) program which is sponsored by the International Erosion Control Association (IECA) and the Soil and Water Conservation Society (SWCS) and is endorsed by USDA - Natural Resources Conservation Service. CPESC, Inc. also administers the CPSWQ (Certified Professional in Stormwater Quality) program.

b. All SWPPPs shall include erosion and sediment controls. For construction activities meeting either Condition "A", "B" or "C" described below, the SWPPP shall also include water quantity and water quality controls (post-construction stormwater control practices).(see Part III. D.).

(1) <u>Condition A</u> - Construction site or post construction runoff discharging a pollutant of concern to either an impaired water identified on DEC's 303(d) list or a TMDL watershed for which pollutants in stormwater have been identified as a source of the impairment.

(2) <u>Condition B</u> - Construction site runoff from Phase 1 construction activities (construction activities disturbing five (5) or more acres) identified under 40 CFR Part 122, 122.26(b)(14)(x).

(3) <u>Condition C</u> - Construction site runoff from construction activity disturbing between one (1) and five (5) acres of land during the course of the project, exclusive of the construction of single family residences and construction activities at agricultural properties.

2. <u>SWPPP Implementation</u> - Operators are responsible for implementing the provisions of the SWPPP and ensuring that all contractors and subcontractors who perform professional services at the site provide certification of the SWPPP in accordance with Part I.D.2. (see page 4) and Part III.E.2. (see page 17) of this permit. All contractors and subcontractors identified in the SWPPP in accordance with Part III.E.1. (see page 17) of this permit must agree to implement applicable provisions of the SWPPP and satisfy the certification requirement of Part III.E.2. (see page17). However, contractors and subcontractors who are not operators, as defined in this permit (see page 4), are not required to submit a NOI in addition to the NOI submitted by the operator.

3. <u>**Deadlines for SWPPP Preparation and Compliance**</u> - The SWPPP must be developed <u>prior</u> to the submittal of an NOI and provide for compliance with the terms and schedule of the SWPPP beginning with the initiation of construction activities. The operator shall also certify in the SWPPP that all appropriate stormwater control measures will be in place <u>before</u> commencement of construction of any segment of the project that requires such measures. 4. **Local Requirements** - Developing a SWPPP that complies with the requirements listed herein does not relieve an operator from the obligation of complying with stormwater management requirements of the local government having jurisdiction over the project.

5. <u>Activities Previously Covered Under GP-93-06</u> - For construction activities which are covered by GP-93-06 as of the effective date of this permit (GP-02-01), the continued implementation of their SWPPP that was developed and implemented in accordance with GP-93-06 is acceptable until such time as:

(a) an NOT is submitted;

(b) the Department notifies them otherwise in accordance with this permit, including Part V, subsection K (see page 21); or

(c) this permit expires.

# B. Signature and SWPPP Review

1. The SWPPP shall be signed in accordance with Part V. H.(see page 19), and be retained at the site where the construction activity occurs in accordance with Part IV (see retention of records on page 17) of this permit.

2. The permittee shall submit a copy of the SWPPP and any amendments thereto to the local governing body and any other authorized agency having jurisdiction or regulatory control over the construction activity. The operator shall make SWPPPs available upon request to the Department and any local agency having jurisdiction; or in the case of a stormwater discharge associated with industrial activity which discharges through a municipal separate storm sewer system, to the municipal operator of the system.

3. The Department, or its authorized representative, may notify the permittee at any time that the SWPPP does not meet one or more of the minimum requirements of this permit. Such notification shall identify those provisions of the permit which are not being met by the SWPPP and identify which provisions of the SWPPP require modifications in order to meet the minimum requirements of this permit. Within seven (7) days of such notification, (or as otherwise provided by the Department) the permittee shall make the required changes to the SWPPP and shall submit to the Department a written certification that the requested changes have been made. Notwithstanding the foregoing, the Department reserves all rights to enforce the terms of the ECL. C. <u>Keeping SWPPPs Current</u> - The permittee shall amend the SWPPP whenever:

1. There is a significant change in design, construction, operation, or maintenance which may have a significant effect on the potential for the discharge of pollutants to the waters of the United States and which has not otherwise been addressed in the SWPPP; or

2. The SWPPP proves to be ineffective in:

a. Eliminating or significantly minimizing pollutants from sources identified in the SWPPP required by this permit, or

b. Achieving the general objectives of controlling pollutants in stormwater discharges from permitted construction activity.

3. Additionally, the SWPPP shall be amended to identify any new contractor or subcontractor that will implement any measure of the SWPPP (see Part III.E, page 17 below). Amendments to the SWPPP may be reviewed by the Department in the same manner as provided by Part III.B (see page 9 above).

# D. <u>General Contents of SWPPPs</u> -

**1.** <u>Standards for construction activities covered under this permit</u> - The Department's technical standards for erosion and sediment controls are detailed in the "*New York Standards and Specifications for Erosion and Sediment Control*"<sup>13</sup> published by the Empire State Chapter of the Soil and Water Conservation Society. For the design of water quality and water quantity controls (post-construction stormwater control practices), the Department's technical standards are detailed in the "*New York State Stormwater Management Design Manual.*"

If an operator certifies that the SWPPP has been developed in conformance with the Department's technical standards referenced above, they may obtain coverage under this general permit in five (5) business days from the Department's receipt of the NOI, provided the construction activity does not meet Condition A in Part III.A.1.b. For SWPPPs which will not conform with the Department's technical standards, the SWPPP must be prepared by a licensed/certified professional and include a certification stating that the SWPPP has been developed in a manner which will assure compliance with the State's water quality standards and with the substantive intent of this permit. In addition, coverage under this general permit will not begin until sixty (60) business days from the receipt of a completed NOI.

<sup>&</sup>lt;sup>13</sup> Previously, the "New York Guidelines for Urban Erosion and Sediment Control," also commonly referred to as the "Blue Book."

2. <u>Minimum SWPPP Components</u> SWPPPs prepared pursuant to this general permit shall present fully designed and engineered stormwater management practices with all necessary maps, plans and construction drawings. The SWPPP must, at a minimum, include the following:

a. For all construction activities subject to this general permit -

(1). provide background information about the scope of the project, including the location, type and size of project.

(2) provide a site map/construction drawing(s) for the project, including a general location map. At a minimum, the site map should show the total site area; all improvements; areas of disturbance; areas that will not be disturbed; existing vegetation; onsite and adjacent off-site surface water(s), wetlands and drainage patterns that could be affected by the construction activity; existing and final slopes; locations of off-site material, waste, borrow or equipment storage areas; and location(s) of the stormwater discharge(s);

(3) provide a description of the soil(s) present at the site;

(4) provide a construction phasing plan describing the intended sequence of construction activities, including clearing and grubbing, excavation and grading, utility and infrastructure installation and any other activity at the site that results in soil disturbance. Consistent with the New York Guidelines for Urban Erosion and Sediment Control, there shall not be more than five (5) acres of disturbed soil at any one time without prior written approval from the Department;

(5) provide a description of the pollution prevention measures that will be used to control litter, construction chemicals and construction debris from becoming a pollutant source in the storm water discharges;

(6) provide a description of construction and waste materials expected to be stored on-site with updates as appropriate, and a description of controls to reduce pollutants from these materials including storage practices to minimize exposure of the materials to storm water, and spill prevention and response;

(7) describe the temporary and permanent structural and vegetative measures to be used for soil stabilization, runoff control and sediment control for each stage of the project from initial land

clearing and grubbing to project close-out;

(8) identify and show on a site map/construction drawing(s) the specific location(s), size(s), and length(s) of each erosion and sediment control practice;

(9) provide the dimensions, material specifications and installation details for all erosion and sediment control practices, including the siting and sizing of any temporary sediment basins;

(10) identify temporary practices that will be converted to permanent control measures;

(11) provide an implementation schedule for staging temporary erosion and sediment control practices, including the timing of initial placement and the duration that each practice should remain in place;

(12) provide a maintenance schedule to ensure continuous and effective operation of the erosion and sediment control practices;

(13) provide the names(s) of the receiving water(s);

(14) provide a delineation of SWPPP implementation responsibilities for each part of the site;

(15) provide a description of structural practices to divert flows from exposed soils, store flows, or otherwise limit runoff and the discharge of pollutants from exposed areas of the site to the degree attainable; and

(16) provide any existing data that describes the stormwater runoff characteristics at the site.

b. <u>For construction activities meeting Condition A, B or C in Part</u> <u>III.A.1.b.</u>

(1) provide all the information required in Parts III.D.2.a.1 - 16 above;

(2) provide a description of each post-construction stormwater control practice;

(3) identify and show on a site map/construction drawing(s) the specific location(s) and size(s) of each post-construction stormwater control practice;

(4) provide a hydrologic and hydraulic analysis for all structural components of the stormwater control system for the applicable design storms;

(5) provide a comparison of post-development stormwater runoff conditions with pre-development conditions;

(6) provide the dimensions, material specifications and installation details for each post-construction stormwater control practice;

(7) provide a maintenance schedule to ensure continuous and effective operation of each post-construction stormwater control practice.

The following three subsections, Part III.D. 3. through Part III.D. 5., apply only to construction activities covered under this general permit which meet Conditions "A", "B"<sup>14</sup> or "C" in Part III. A.1.b. Beginning with Part III.E. below (see page 17) the requirements set forth therein apply to all permittees covered under this permit.

# 3. <u>Site Assessment and Inspections</u> -

a. The operator shall have a qualified professional<sup>15</sup> conduct an assessment of the site prior to the commencement of construction and certify in an inspection report that the appropriate erosion and sediment controls described in the SWPPP and required by Part III.D. (see page 10) of this permit have been adequately installed or implemented to ensure overall preparedness of the site for the commencement of construction. Following the commencement of construction, site inspections shall be conducted by the qualified professional at least every 7 calendar days and within 24 hours of the end of a storm event of 0.5 inches or greater. During each inspection, the qualified professional shall record the following information:

(1) On a site map, indicate the extent of all disturbed site areas and drainage pathways. Indicate site areas that are expected to undergo initial disturbance or significant site work within the next 14-day period;

(2) Indicate on a site map all areas of the site that have undergone temporary or permanent stabilization;

(3) Indicate all disturbed site areas that have not undergone active site work during the previous 14-day period;

(4) Inspect all sediment control practices and record the approximate degree of sediment accumulation as a percentage of the sediment storage volume (for example, 10 percent, 20 percent, 50 percent);

(5) Inspect all erosion and sediment control practices and record all maintenance requirements such as verifying the integrity of barrier or diversion systems (earthen berms or silt fencing) and

<sup>&</sup>lt;sup>14</sup> Condition "B" includes construction activities covered under GP-93-06 and, therefore, are subject to Part III.D.3 through Part III.D. 5.

<sup>&</sup>lt;sup>15</sup> "Qualified professional" means a person knowledgeable in the principles and practice of erosion and sediment controls, such as a licensed professional engineer, Certified Professional in Erosion and Sediment Control (CPESC), or soil scientist.

containment systems (sediment basins and sediment traps). Identify any evidence of rill or gully erosion occurring on slopes and any loss of stabilizing vegetation or seeding/mulching. Document any excessive deposition of sediment or ponding water along barrier or diversion systems. Record the depth of sediment within containment structures, any erosion near outlet and overflow structures, and verify the ability of rock filters around perforated riser pipes to pass water;

- and
- (6) All deficiencies that are identified with the implementation of the SWPPP.

b. The operator shall maintain a record of all inspection reports in a site log book. The site log book shall be maintained on site and be made available to the permitting authority upon request. Prior to the commencement of construction,<sup>16</sup> the operator shall certify in the site log book that the SWPPP, prepared in accordance with Part III.D. (see page 10) of this permit, meets all Federal, State and local erosion and sediment control requirements.

The operator shall post at the site, in a publicly-accessible location, a summary of the site inspection activities on a monthly basis.

c. Prior to filing of the Notice of Termination or the end of permit term, the operator shall have the qualified professional perform a final site inspection. The qualified professional shall certify that the site has undergone final stabilization<sup>17</sup> using either vegetative or structural stabilization methods and that all temporary erosion and sediment controls (such as silt fencing) not needed for long-term erosion control have been removed.

d. The operator shall certify that the requirements of Parts III.D.3., III.D.4. and III.D.5 of this permit have been satisfied within 48 hours of actually meeting such requirements.

<sup>&</sup>lt;sup>16</sup> "Commencement of construction" means the initial removal of vegetation and disturbance of soils associated with clearing, grading or excavating activities or other construction activities.

<sup>&</sup>lt;sup>17</sup> "Final stabilization" means that all soil-disturbing activities at the site have been completed and a uniform, perennial vegetative cover with a density of eighty (80) percent has been established or equivalent stabilization measures (such as the use of mulches or geotextiles) have been employed on all unpaved areas and areas not covered by permanent structures.

4. <u>Stabilization<sup>18</sup></u> - The operator shall initiate stabilization measures as soon as practicable in portions of the site where construction activities have temporarily or permanently ceased, but in no case more than 14 days after the construction activity in that portion of the site has temporarily or permanently ceased. This requirement does not apply in the following instances:

a. Where the initiation of stabilization measures by the 14th day after construction activity temporarily or permanently ceased is precluded by snow cover or frozen ground conditions, stabilization measures shall be initiated as soon as practicable;

b. Where construction activity on a portion of the site is temporarily ceased, and earth-disturbing activities will be resumed within twenty-one (21) days, temporary stabilization measures need not be initiated on that portion of the site.

5. <u>Maintenance</u> - Sediment shall be removed from sediment traps or sediment ponds whenever their capacity has been reduced by fifty (50) percent from the design capacity.

<sup>&</sup>lt;sup>18</sup> "Stabilization" means covering or maintaining an existing cover over soil. Cover can be vegetative (e.g. grass, trees, seed and mulch, shrubs, or turf) or non-vegetative (e.g. geotextiles, riprap, or gabions).

# E. <u>Contractors</u>

1. The SWPPP must clearly identify for each measure identified in the SWPPP, the contractor(s) and subcontractor(s) that will implement the measure. All contractors and subcontractors identified in the SWPPP must sign a copy of the certification statement in Part III.E.2 (see below) of this permit in accordance with Part V.H.(see page 19) of this permit. All certifications must be included in the SWPPP. Additionally, new contractors and subcontractors (see subsection C.3. above) need to similarly certify.

2. <u>Certification Statement</u> - All contractors and subcontractors identified in a SWPPP in accordance with Part III.E.1 (see above) of this permit shall sign a copy of the following certification statement before undertaking any construction activity at the site identified in the SWPPP:

"I certify under penalty of law that I understand and agree to comply with the terms and conditions of the SWPPP for the construction site identified in such SWPPP as a condition of authorization to discharge stormwater. I also understand that the operator must comply with the terms and conditions of the New York State Pollutant Discharge Elimination System ("SPDES") general permit for stormwater discharges from construction activities and that it is unlawful for any person to cause or contribute to a violation of water quality standards."

The certification must include the name and title of the person providing the signature in accordance with Part V.H.(see page 19) of this permit; the name, address and telephone number of the contracting firm; the address (or other identifying description) of the site; and the date the certification is made.

# Part IV. MONITORING, REPORTING AND RETENTION OF RECORDS

A. The Department may, at its sole discretion, require monitoring of discharge(s) from the permitted construction activity after notifying the permittee in writing of the basis for such monitoring, the parameters and frequency at which monitoring shall occur and the associated reporting requirements, if any.

B. The operator shall retain copies of SWPPPs and any reports submitted in conjunction with this permit, and records of all data used to complete the NOI to be covered by this permit, for a period of at least three years from the date that the site is finally stabilized. This period may be extended by the Department, in its sole discretion, at any time upon written notification.

C. The operator shall retain a copy of the SWPPP required by this permit at the construction site from the date of initiation of construction activities to the date of final

stabilization.

D. The operator shall also prepare a written summary of its status with respect to compliance with this general permit at a minimum frequency of every three months during which coverage under this permit exists. The summary should address the status of achieving each component of the SWPPP. This summary shall be handled in the same manner as prescribed for SWPPPs under Part III, subsection B (see Page 9).

E. <u>Addresses</u> - Except for the submittal of NOIs and NOTs, all written correspondence under this permit directed to the Department, including the submittal of individual permit applications, shall be sent to the address of the appropriate Department Office as listed in Appendix A (see page 23).

## Part V. STANDARD PERMIT CONDITIONS

A. **Duty to Comply** - The operator must comply with all conditions of this permit. All contractors and subcontractors associated with the project must comply with the terms of the SWPPP. Any permit noncompliance constitutes a violation of the Clean Water Act (CWA) and the ECL and is grounds for an enforcement action against either the operator or the contractor/subcontractor; permit revocation or modification; or denial of a permit renewal application. Upon a finding of significant non-compliance with this permit or the applicable SWPPP, the Department may order an immediate stop to all construction activity at the site until the non-compliance is remedied. The stop work order shall be in writing, shall describe the non-compliance in detail, and shall be sent to the operator or the operator's on-site representative.

B. <u>Continuation of the Expired General Permit</u> - This permit expires five (5) years after issuance on January 8, 2008. However, coverage may be obtained under the expired general permit which will continue in force and effect until a new general permit is issued. After issuance of a new general permit, those with coverage under GP-02-01 will have six (6) months from the effective date of the new general permit to complete their project or obtain coverage under the new permit. Unless otherwise notified by the Department in writing, operators seeking authorization under a new general permit must submit a new NOI in accordance with the terms of such new general permit. See also Part I, subsection D.8. on page 6.

C. <u>Penalties for Violations of Permit Conditions</u> - There are substantial criminal, civil, and administrative penalties associated with violating the provisions of this permit. Fines of up to \$25,000 per day for each violation and imprisonment for up to fifteen (15) years may be assessed depending upon the nature and degree of the offense.

D. <u>Need to halt or reduce activity not a defense</u> - It shall not be a defense for a permittee in an enforcement action that it would have been necessary to halt or reduce the construction activity in order to maintain compliance with the conditions of this permit.

E. **Duty to Mitigate** - The permittee and its contractors and subcontractors shall take all reasonable steps to minimize or prevent any discharge in violation of this permit which has a reasonable likelihood of adversely affecting human health or the environment.

F. **Duty to Provide Information** - The permittee shall furnish any information requested by any agency with regulatory or review authority over this project for the purpose of determining compliance with this permit or compliance with any other regulatory requirements placed on the project in conjunction with this permit. Failure to provide requested information shall be a violation of this permit. Such regulating agencies include but are not limited to the Department, SWCDs,<sup>19</sup> local planning, zoning, health, and building departments that review and approve erosion and sediment control plans, grading plans, and Stormwater Management Plans, as well as MS4s into whose system runoff from the permitted project or activity discharges. The SWPPP and inspection reports required by this general permit are public documents that the operator must make available for inspection, review and copying by any person within five (5) business days of the operator receiving a written request by any such person to review the SWPPP and/or the inspection reports. Copying of documents will be done at the requester's expense.

G. <u>Other Information</u> - When the permittee becomes aware that he or she failed to submit any relevant facts or submitted incorrect information in the NOI or in any other report to the Department, he or she shall promptly submit such facts or information.

H. <u>Signatory Requirements</u> - All NOIs, NOTs, SWPPPs, reports, certifications or information required by this permit or submitted pursuant to this permit, shall be signed as follows:

1. All NOIs and NOTs shall be signed as follows:

a. For a corporation: by (1) a president, secretary, treasurer, or vicepresident of the corporation in charge of a principal business function, or any other person authorized to and who performs similar policy or decisionmaking functions for the corporation; or (2) the manager of one or more manufacturing, production or operating facilities employing more than 250 persons or having gross annual sales or expenditures exceeding \$25,000,000 (in second-quarter 1980 dollars) if authority to sign documents has been assigned or delegated to the manager in accordance with corporate procedures;

<sup>19</sup> "SWCD" means Soil and Water Conservation District

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b. For a partnership or sole proprietorship: by a general partner or the proprietor, respectively; or

c. For a municipality, State, Federal, or other public agency: by either a principal executive officer or ranking elected official. For purposes of this section, a principal executive officer of a Federal agency includes (1) the chief executive officer of the agency, or (2) a senior executive officer having responsibility for the overall operations of a principal geographic unit of the agency (e.g., Regional Administrators of EPA).

2. The SWPPP and all reports required by the permit and other information requested by the Department or local agency shall be signed by a person described above or by a duly authorized representative of that person. A person is a duly authorized representative only if:

a. The authorization is made in writing by a person described above and submitted to the Department.

b. The authorization specifies either an individual or a position having responsibility for the overall operation of the regulated facility or activity, such as the position of manager, operator, superintendent, or position of equivalent responsibility or an individual or position having overall responsibility for environmental matters for the company. (A duly authorized representative may thus be either a named individual or any individual occupying a named position).

c. <u>Certification</u> - Except for NOIs and NOTs, any person signing documents in accordance with this Part shall make the following certification:

"I certify under penalty of law that this document and all attachments were prepared under my direction or supervision in accordance with a system designed to assure that qualified personnel properly gathered and evaluated the information submitted. Based on my inquiry of the person or persons who manage the system, or those persons directly responsible for gathering the information, the information submitted is, to the best of my knowledge and belief, true, accurate, and complete. I am aware that false statements made herein are punishable as a class A misdemeanor pursuant to Section 210.45 of the Penal Law."

I. <u>**Property Rights**</u> - The issuance of this permit does not convey any property rights of any sort, nor any exclusive privileges, nor does it authorize any injury to private property nor any invasion of personal rights, nor any infringement of Federal, State or local laws or regulations.

J. <u>Severability</u> - The provisions of this permit are severable, and if any provision of this permit, or the application of any provision of this permit to any circumstance, is held invalid, the application of such provision to other circumstances, and the remainder of this permit shall not be affected thereby.

## K. Denial of Coverage Under This Permit

1. At its sole discretion, the Department may require any person authorized by this permit to apply for and/or obtain either an individual SPDES permit or an alternative SPDES general permit. Where the Department requires a discharger authorized to discharge under this permit to apply for an individual SPDES permit, the Department shall notify the discharger in writing that a permit application is required. This notification shall include a brief statement of the reasons for this decision, an application form, a statement setting a deadline for the discharger to file the application, and a statement that on the effective date of issuance or denial of the individual SPDES permit or the alternative general permit as it applies to the individual permittee, coverage under this general permit shall automatically terminate. Applications shall be submitted to the appropriate Department Office indicated in Appendix A of this permit. The Department may grant additional time to submit the application upon request of the applicant. If a discharger fails to submit in a timely manner an individual SPDES permit application as required by the Department under this paragraph, then the applicability of this permit to the individual SPDES permittee is automatically terminated at the end of the day specified by the Department for application submittal.

2. Any discharger authorized by this permit may request to be excluded from the coverage under this permit by applying for an individual permit. In such cases, the permittee shall submit an individual application in accordance with the requirements of 40 CFR 122.26(c)(1)(ii) and 6 NYCRR Part 621, with reasons supporting the request, to the Department at the address for the appropriate Department Office (see addresses in Appendix A on page 23 of this permit). The request may be granted by issuance of an individual permit or an alternative general permit at the discretion of the Department.

3. When an individual SPDES permit is issued to a discharger covered by this permit, or the discharger is authorized to discharge under an alternative SPDES general permit, the applicability of this permit to the individual SPDES permittee is automatically terminated on the effective date of the individual permit or the date of authorization of coverage under the alternative general permit, whichever the case may be. When an individual SPDES permit is denied to an operator otherwise subject to this permit, or the operator is denied for coverage under an alternative SPDES general permit, the applicability of this permit to the individual SPDES permittee is automatically terminated on the date of such denial, unless otherwise specified by the Department.

L. **Proper Operation and Maintenance** - The permittee shall at all times properly operate and maintain all facilities and systems of treatment and control (and related appurtenances) which are installed or used by the permittee to achieve compliance with the conditions of this permit and with the requirements of SWPPPs. Proper operation and maintenance also includes adequate laboratory controls and appropriate quality assurance procedures. Proper operation and maintenance requires the operation of backup or auxiliary facilities or similar systems, installed by a permittee only when necessary to achieve compliance with the conditions of the permit.

M. **Inspection and Entry** - The permittee shall allow the Department or an authorized representative of EPA, the State, or, in the case of a construction site which discharges through an MS4, an authorized representative of the MS4 receiving the discharge, upon the presentation of credentials and other documents as may be required by law, to:

1. Enter upon the permittee's premises where a regulated facility or activity is located or conducted or where records must be kept under the conditions of this permit;

2. Have access to and copy at reasonable times, any records that must be kept under the conditions of this permit; and

3. Inspect at reasonable times any facilities or equipment (including monitoring and control equipment).

N. <u>**Permit Actions**</u> - At the Department's sole discretion, this permit may, at any time, be modified, revoked, or renewed. The filing of a request by the permittee for a permit modification, revocation and reissuance, termination, a notification of planned changes or anticipated noncompliance does not stay compliance with any terms of this permit.

## APPENDIX A

## List of NYS DEC Regional Offices

Region	Covering the following counties:	DIVISION OF ENVIRONMENTAL PERMITS (DEP) <u>Permit Administrators</u>	DIVISION OF WATER (DOW) <u>Water (SPDES) Program</u>
1	Nassau and Suffolk	Bldg 40 - SUNY @ Stony Brook Stony Brook, NY 11790-2356 Tel. (631) 444-0365	Bldg 40 - SUNY @ Stony Brook Stony Brook, NY 11790-2356 Tel. (631) 444-0405
2	Bronx, Kings, New York, Queens and Richmond	1 Hunters Point Plaza, 47-40 21st St. Long Island City, NY 11101-5407 Tel. (718) 482-4997	1 Hunters Point Plaza, 47-40 21st St. Long Island City, NY 11101-5407 Tel. (718) 482-4933
3	Dutchess, Orange, Putnam, Rockland, Sullivan, Ulster and Westchester	21 South Putt Corners Road New Paltz, NY 12561-1696 Tel. (845) 256-3059	200 White Plains Road, 5 <sup>th</sup> Floor Tarrytown, NY 10591-5805 Tel. (845) 332-1835
4	Albany, Columbia, Delaware, Greene, Montgomery, Otsego, Rensselaer, Schenectady and Schoharie	1150 North Westcott Road Schenectady, NY 12306-2014 Tel. (518) 357-2069	1150 North Westcott Road Schenectady, NY 12306-2014 Tel. (518) 357-2045
5	Clinton, Essex, Franklin, Fulton, Hamilton, Saratoga, Warren and Washington	Route 86, PO Box 296 Ray Brook, NY 12977-0296 Tel. (518) 897-1234	232 Hudson Street Warrensburg, NY 12885-0220 Tel. (518) 623-1200
6	Herkimer, Jefferson, Lewis, Oneida and St. Lawrence	State Office Building 317 Washington Street Watertown, NY 13601-3787 Tel. (315) 785-2245	State Office Building 207 Genesee Street Utica, NY 13501-2885 Tel. (315) 793-2554
7	Broome, Cayuga, Chenango, Cortland, Madison, Onondaga, Oswego, Tioga and Tompkins	615 Erie Blvd. West Syracuse, NY 13204-2400 Tel. (315) 426-7438	615 Erie Blvd. West Syracuse, NY 13204-2400 Tel. (315) 426-7500
8	Chemung, Genesee, Livingston, Monroe, Ontario, Orleans, Schuyler, Seneca, Steuben, Wayne and Yates	6274 East Avon-Lima Road Avon, NY 14414-9519 Tel. (585) 226-2466	6274 East Avon-Lima Rd. Avon, NY 14414-9519 Tel. (585) 226-2466
9	Allegany, Cattaraugus, Chautauqua, Erie, Niagara and Wyoming	270 Michigan Avenue Buffalo, NY 14203-2999 Tel. (716) 851-7165	270 Michigan Ave. Buffalo, NY 14203-2999 Tel. (716) 851-7070

## **APPENDIX B**

## Information Required of Construction Activities Which Are Identified Under Part I, subsection D.7. (see page 5)

А.	The location (including a map) and the nature of the construction activity;
В.	The total area of the site and the area of the site that is expected to undergo excavation during the life of the permit;
C.	Proposed measures, including best management practices, to control pollutants in storm water discharges during construction, including a brief description of applicable State and local erosion and sediment control requirements;
D.	Proposed measures to control pollutants in storm water discharges that will occur after construction operations have been completed, including a brief description of applicable State or local erosion and sediment control requirements;
E.	An estimate of the runoff coefficient of the site and the increase in impervious area after the construction addressed in the permit application is completed, the nature of the fill material and existing data describing the soil or the quality of the discharge; and
F.	The name of the receiving water(s).

# Appendix B: Notice of Intent (NOI)



### NOTICE OF INTENT

### **New York State Department of Environmental Conservation**

**Division of Water** 

625 Broadway, 4th Floor

Albany, New York 12233-3505

Stormwater Discharges Associated with <u>Construction Activity</u> Under State Pollutant Discharge Elimination System (SPDES) General Permit # GP-02-01 All sections must be completed unless otherwise noted. Failure to complete all items may result in this form being returned to you, thereby delaying your coverage under this General Permit. Applicants must read and understand the conditions of the permit and prepare a Stormwater Pollution Prevention Plan prior to submitting this NOI. Applicants are responsible for identifying and obtaining other DEC permits that may be required. To properly complete this form, please refer to the Instruction Manual which can be accessed at www.dec.state.ny.us/website/dow/toolbox/instr\_man.pdf

### -IMPORTANT-

## THIS FORM FOR MACHINE PRINT ONLY/USE OTHER FORM FOR HANDPRINT DO NOT USE HANDWRITING ON THIS FORM

OWNER/OPERATOR MUST SIGN FORM

			C	wner/0	perator	Infor	mation				
Owner/	Operator	(Company	/ Name/Pri	vate O	wner Na	me/Mun	icipalit	ty Name)			
Owner/	Operator	Contact	Person La	st Name	e (NOT	CONSUL	TANT)				
Owner/	Operator	Contact	Person Fi	rst Na	me						
Owner/	Operator	Mailing	Address								
City											
State		Zip									
				-							
Phone	(Owner/Op	perator)				Fax	(Owner/0	Operator	)		
	-		-					-		-	
Email	(Owner/Op	perator)									



Proje	ect Site Information
Project/Site Name	
Street Address (NOT P.O. BOX)	
City/Town/Village (THAT ISSUES BUILDI	NG PERMIT)
State Zip	
-	
County	DEC Region (if known)
Name of Nearest Cross Street	
Distance to Nearest Cross Street (Fee	t) Direction to Nearest Cross Street
	$\bigcirc$ North $\bigcirc$ South $\bigcirc$ East $\bigcirc$ West

1. Provide the Geographic Coordinates for the project site in NYTM Units. To do this you **must** go to the NYSDEC Stormwater Interactive Map on the DEC website at:

#### www.dec.state.ny.us/website/imsmaps/stormwater/viewer.htm

Zoom into your Project Location such that you can accurately click on the centroid of your site. Once you have located your project site go to the dropdown menu on the left and choose "Get Coordinates". Click on the center of your site and a small window containing the X, Y coordinates in UTM will pop up. Transcribe these coordinates into the boxes below. For problems with the interactive map use the help function.

4	X Coordinates (Easting)	Y Coordinates (Northing)
		4

2. What is the nature of this construction project?

New Construction
 Redevelopment with increase in imperviousness
 Redevelopment with no increase in imperviousness



3. Select the predominant land use for both pre and post development conditions. **SELECT ONLY ONE CHOICE FOR EACH** 

Pre-Development Existing Land Use	Post-Development Future Land Use
○ FOREST	$\bigcirc$ SINGLE FAMILY HOME
$\bigcirc$ pasture/open land	$\bigcirc$ SINGLE FAMILY SUBDIVISION
$\bigcirc$ CULTIVATED LAND	$\bigcirc$ TOWN HOME RESIDENTIAL
$\bigcirc$ SINGLE FAMILY HOME	$\bigcirc$ MULTIFAMILY RESIDENTIAL
$\bigcirc$ SINGLE FAMILY SUBDIVISION	$\bigcirc$ INSTITUTIONAL $\$ SCHOOL
$\bigcirc$ TOWN HOME RESIDENTIAL	$\bigcirc$ INDUSTRIAL
$\bigcirc$ MULTIFAMILY RESIDENTIAL	○ COMMERCIAL
$\bigcirc$ INSTITUTIONAL\SCHOOL	○ ROAD\HIGHWAY
$\bigcirc$ INDUSTRIAL	○ RECREATIONAL\SPORTS FIELD
○ COMMERCIAL	○ BIKE PATH\TRAIL
$\bigcirc$ ROAD $\setminus$ HIGHWAY	○ SUBSURFACE UTILITY
○ RECREATIONAL\SPORTS FIELD	○ PARKING LOT
○ BIKE PATH\TRAIL	
○ SUBSURFACE UTILITY	
○ PARKING LOT	
OTHER	OTHER

4. Will future use of this site be an agricultural property as defined by the NYS Agriculture and Markets Law ?

### $\bigcirc$ Yes $\bigcirc$ No

 $\bigcirc$  No

 $\bigcirc$  No

5. Is this a remediation project conducted in accordance with a NYSDEC approved work plan?

6. Is this property owned by a state authority, state agency or local government?

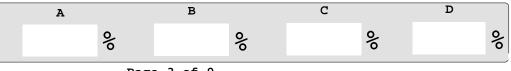
7. In accordance with the larger common plan of development or sale; enter the total project site acreage, the acreage to be disturbed and the future impervious area (acreage)within the disturbed area. Round to the nearest tenth of an acre.

Total Project Site Acreage	Acreage to be Disturbed	Impervious Area within Disturbed

8. Will there be more than 5 acres disturbed at any given time?

 $\bigcirc$  Yes  $\bigcirc$  No

9. Indicate the percentage of each Hydrologic Soil Group(HSG) at the site.



#### Page 3 of 9

○ Yes

O Yes



10.	Is	this	а	phased	project?	( <b>if</b>	yes,	The	SWPPP	must	address	all	planned
phas	ses)	)											

*	
⊖ Yes	○ No

11.	Er	nter	the	e planned	stai	rt	and	end
date	s	of	the	disturba	nce a	act	ivit	ies

Start Dat	e	End Date						
/		/		-		/	/	

#### Receiving System(s)

12. Provide the name of the surface waterbody(ies) into which construction site runoff will discharge.

For Questions 13 and 14 refer to the Instruction Manual for a subset of segments and TMDL watersheds subject to Condition A of the permit. These and watersheds have been identified for regulation within the stormwater to a pollutant of concern. The Instruction Manual can be accessed at www.dec.state.ny.us/website/dow/toolbox/instr_man.pdf	waterboo	
13. Has the surface waterbody(ies) in question 12 been identified as a 303(d) segment?	* O Yes	0 No
14. Is this project located in a TMDL Watershed?	* ○ Yes	() No
*NOTE: If you answered Yes to either question 13 or 14, Pursuant to Part the permit, you <u>must</u> have your SWPPP prepared and certified by a licensed professional and the SWPPP is subject to a 60-business day review.		-
.5. Does the site runoff enter a separate storm sewer system-		Inknown

including roadside drains, swales, ditches, culverts, etc? (if no, skip question 16)

$\bigcirc$ Yes	$\bigcirc$ No	$\bigcirc$ Unknown

16. What is the name of the municipality/entity that owns the separate storm sewer system?

17. Does any runoff from the site enter a sewer classified as a Combined Sewer?

$\bigcirc$ Yes $\bigcirc$ No	🔵 Unknown
------------------------------	-----------



18. Has the required Erosion and Sediment Control component of the SWPPP been developed in conformance with the current NYS Standards and Specifications for Erosion and Sediment Control (aka Blue Book) ?

19. Does this construction activity require the development of a SWPPP that includes Water Quality and Quantity Control components (Post-Construction Stormwater Management Practices) If no, Skip question 20

20. Have the Water Quality and Quantity Control components of the SWPPP been developed in comformance with the current NYS Stormwater Management Design Manual ?

NOTE: If you answered no to question 18 or 20, Pursuant to Part I.D.3.(b) of the permit, you <u>must</u> have your SWPPP prepared and certified by a licensed/certified professional and the SWPPP is subject to a 60-business day review. Please provide further details in the details/comment section on the last page of this form.

21. The Stormwater Pollution Prevention Plan (SWPPP) was prepared by:

○ Professional Engineer (P.E.)

 $\bigcirc$  Soil and Water Conservation District (SWCD)

O Registered Landscape Architect (R.L.A)

 $\bigcirc$  Certified Professional in Erosion and Sediment Control (CPESC)

○ Owner/Operator

Other

SWPPP Preparer	SWPPP Preparer Information (if different from Owner/Operator	
Contact Name (Last, Space,	First)	
Mailing Address		
City		
State Zip		
State Zip		
Phone	- Fax	
	-	-
Email		
<		

O Yes	○ <b>No</b>

\*

O No

O No

○ Yes

🔾 Yes



### Stormwater Pollution Prevention Plan (SWPPP)

#### Erosion and Sediment Control Practices

22. Has a construction sequence schedule for the planned management practices been prepared?

 $\bigcirc$  Yes  $\bigcirc$  No

23. Select **all** of the erosion and sediment control practices that will be employed on the project site.

Temporary Structural	Vegetative Measures		
$\bigcirc$ Check Dams	$\bigcirc$ Brush Matting		
$\bigcirc$ Construction Road Stabilization	$\bigcirc$ Dune Stabilization		
○ Dust Control	$\bigcirc$ Grassed Waterway		
$\bigcirc$ Earth Dike	○ Mulching		
○ Level Spreader	$\bigcirc$ Protecting Vegetation		
○ Perimeter Dike/Swale	$\bigcirc$ Recreation Area Improvement		
$\bigcirc$ Pipe Slope Drain	○ Seeding		
$\bigcirc$ Portable Sediment Tank	○ Sodding		
$\bigcirc$ Rock Dam	○ Straw/Hay Bale Dike		
$\bigcirc$ Sediment Basin	$\bigcirc$ Streambank Protection		
$\bigcirc$ Sediment Traps	$\bigcirc$ Temporary Swale		
$\bigcirc$ Silt Fence			
$\bigcirc$ Stabilized Construction Entrance	$\bigcirc$ Vegetating Waterways		
$\bigcirc$ Storm Drain Inlet Protection			
$\bigcirc$ Straw/Hay Bale Dike	Permanent Structural		
$\bigcirc$ Temporary Access Waterway Crossing			
$\bigcirc$ Temporary Stormdrain Diversion	O Debris Basin		
○ Temporary Swale	○ Diversion		
$\bigcirc$ Turbidity Curtain	$\bigcirc$ Grade Stabilization Structure		
$\bigcirc$ Water bars	$\bigcirc$ Land Grading		
	$\bigcirc$ Lined Waterway (Rock)		
Biotechnical	$\bigcirc$ Paved Channel (Concrete)		
Biotecimical	$\bigcirc$ Paved Flume		
$\bigcirc$ Brush Matting	$\bigcirc$ Retaining Wall		
○ Wattling	$\bigcirc$ Riprap Slope Protection		
	$\bigcirc$ Rock Outlet Protection		
Other	$\bigcirc$ Streambank Protection		



Stormwater Pollution Prevention Plan (SWPPP)

Water Quality and Quantity Control

#### Important:Completion of Questions 24-30 is not required if the project:

Disturbs less than 5 acres <u>and</u> is planned for single-family residential homes(including subdivisions) or construction on agricultural property <u>and</u> does not have a discharge to a 303(d) water or is not located within a TMDL watershed.

Additionally, sites where there will be no future impervious area within the disturbed area <u>and</u> that do not have a change(pre to post development) in hydrology do not need to complete questions 24-30.

24. Indicate **all** the permanent Stormwater Management Practice(s) that will be installed on this site

Post Construction Stormwate	r Management Practices			
Ponds	Wetlands			
$\bigcirc$ Micropool Extended Detention (P-1)	$\bigcirc$ Shallow Wetland (W-1)			
○Wet Pond (P-2)	$\bigcirc$ Extended Detention Wetland (W-2)			
$\bigcirc$ Wet Extended Detention (P-3)	○ Pond/Wetland System (W-3)			
$\bigcirc$ Multiple Pond System (P-4)	$\bigcirc$ Pocket Wetland (W-4)			
<pre>O Pocket Pond (P-5) Filtering</pre>	Infiltration			
	$\bigcirc$ Infiltration Trench (I-1)			
$\bigcirc$ Surface Sand Filter (F-1)	$\bigcirc$ Infiltration Basin (I-2)			
$\bigcirc$ Underground Sand Filter (F-2)	$\bigcirc$ Dry Well (I-3)			
$\bigcirc$ Perimeter Sand Filter (F-3)	Open Channel a			
$\bigcirc$ Organic Filter (F-4)	<u>Open Channels</u>			
O Bioretention (F-5)	$\bigcirc$ Dry Swale (O-1)			
	$\bigcirc$ Wet Swale (O-2)			
○ Other				
Describe other stormwater management practices not listed above or explain any deviations from the technicial standards. If the SWPPP does not conform to the				
technicial standards, the SWPPP must be pr licensed/certified professional and is sub				

Has a long term Operation and Maintenance plan for the post construction management practices been developed?

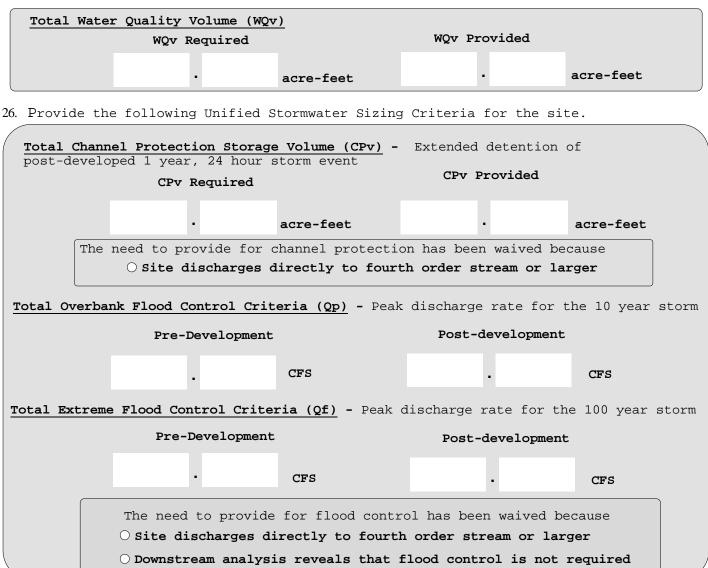
 $\bigcirc$  Yes  $\bigcirc$  No

If Yes, Identify the entity responsible for the long term Operation and Maintenance



Stormwater Pollution Prevention Plan (SWPPP) Water Quality and Quantity Control

25. Provide the total water quality volume required and the total provided for the site.



<u>IMPORTANT</u>: For questions 27 and 28 impervious area should be calculated considering the project site and all offsite areas that drain to the post-construction stormwater management practice(s) (Total Drainage Area = Project Site + Offsite areas)

27. Pre-Construction Impervious Area - As a percent of the <u>Total</u> <u>Drainage Area</u> enter the percentage of the existing impervious areas before construction begins.

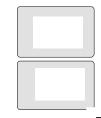
28. Post-Construction Impervious Area - As a percent of the <u>Total</u> <u>Drainage Area</u> enter the percentage of the future impervious areas that will be created/remain on the site after completion of construction.

29. Indicate the total number of permanent stormwater management practices to be installed

30. Provide the total number of stormwater discharge points from the site (include discharges to either surface waters or to seperate storm sewer systems)









31. Select any other DEC permits that are required for this project or **None** 

DEC Permits				
$\bigcirc$ Air Pollution Control	$\bigcirc$ Stream Protection/Article 15			
$\bigcirc$ Coastal Erosion	$\bigcirc$ Water Quality Certificate			
$\bigcirc$ Hazardous Waste	$\bigcirc$ Dam Safety			
$\bigcirc$ Long Island Wells	$\bigcirc$ Water Supply			
$\bigcirc$ Mined Land Reclamation	$\bigcirc$ Freshwater Wetlands			
$\bigcirc$ Other SPDES	$\bigcirc$ Tidal Wetlands			
$\bigcirc$ Solid Waste	$\bigcirc$ Wild, Scenic and Recreational Rivers			
Other				

If this NOI is being submitted for the purpose of <u>continuing coverage</u> under a general permit for stormwater runoff from construction activities, please indicate the former SPDES number assigned.

NYR

Details/Comments

Certification

I have read or been advised of the permit conditions and believe that I understand them. I also understand that, under the terms of the permit, there may be reporting requirements. I also certify under penalty of law that this document and the corresponding documents were prepared under my direction or supervision in accordance with a system designed to assure that qualified personnel properly gather and evaluate the information submitted. Based on my inquiry of the person(s) who manage the system, or those persons directly responsible for gathering the information, the information submitted is, to the best of my knowledge and belief, true, accurate and complete. I am aware that there are significant penalties for submitting false information, including the possibility of fine and imprisonment for knowing violations. I further understand that coverage under the general permit will be identified in the acknowledgment that I will receive as a result of submitting this NOI and can be as long as sixty (60) days as provided for in the general permit. I also understand that, by submitting this NOI, I am acknowledging that the SWPPP has been developed and will be implemented as the first element of construction. and agreeing to comply with all the terms and conditions of the general permit for which this NOI is being submitted.

Owner/Operator Signature	Date			
		/	/	

## Appendix C: Operator's and Contractor's Certification Forms

## Stormwater Pollution Prevention Plan Contractor's Certification Silo Ridge Country Club Golf Resort Community Town of Amenia Dutchess County, New York

The <u>Contractor</u> and/or <u>Subcontractor(s)</u> that will implement the pollutant control measures described in the SWPPP must be identified below. Each must sign a statement certifying that they understand the NPDES and NYSDEC general permit authorizing storm water discharges during construction. These statements must be maintained in the SWPPP file on site.

### **Contractor Implementing the Storm Water Pollution Prevention Plan:**

Business Name:
Business Address:
Telephone No.:
Name of Signatory:
Title of Signatory:
Signature:
Date:
Contractor's Responsibility(s):

### **Certification:**

(Note: signature requirements in Part VI.G. of the NPDES General Permit and Part III.E.1 of the NYSDEC SPDES Permit GP-02-01)

"I certify under penalty of law that I understand and agree to comply with the terms and conditions of the SWPPP for the construction site identified in such SWPPP as a condition of authorization to discharge storm water. I also understand that the Operator must comply with the terms and conditions of the New York State Pollution Discharge Elimination System ("SPDES") general permit for storm water discharges from construction discharges from construction activities and that it is unlawful for any person to cause or contribute to a violation of water quality standards."

<u>Signatory Requirements</u> - All NOIs, NOTs, SWPPPS, reports, certifications or information required by this permit or submitted pursuant to this permit, shall be signed as follows:

- 1 For a corporation: by a (1) president, secretary, treasurer, or vice-president of the corporation in charge of a principle business function, or any other person authorized to and who performs similar policy or decision-making functions for the corporation; or (2) the manager of one or more manufacturing, production or operating facilities employing more than 250 persons or having a gross annual sales or expenditures exceeding \$25,000,000 (in second-quarter 1980 dollars) if authority to sign documents has been assigned or delegated to manage in accordance with corporate procedures;
- 2 For a partnership or sole proprietorship: by a general partner or the proprietor, respectively; or
- <sup>3</sup> For a municipality, State, Federal, or other public agency; by either a principal executive officer ranking elected official. For purposes of this section, a principal executive officer of a Federal agency includes (1) the chief executive officer of the agency, or (2) a senior executive officer having responsibility for the overall operations of a principal geographic unit of the agency (e.g. Regional Administrators of EPA).

Chazen Engineering & Land Surveying Co., P.C.

EnviroPlan Associates, Inc.



Chazen Environmental Services, Inc TelePlan Associates, Inc.

Silo Ridge Country Club Golf Resort Community

## Stormwater Pollution Prevention Plan Operator's Certification Silo Ridge Country Club Golf Resort Community Town of Amenia Dutchess County, New York

The Operator that will implement the pollutant control measures described in the SWPPP must be identified below. Each must sign a statement certifying that they understand the NPDES and NYSDEC general permit authorizing storm water discharges during construction. These statements must be maintained in the SWPPP file on site.

### **Owner:**

isiness Name:
isiness Address:
elephone No.:
ame of Signatory:
tle of Signatory:
gnature:
ate:

### **Certification:**

(Note: signature requirements in Part V.H.2.c of the NYSDEC SPDES Permit GP-02-01)

"I certify under penalty of law that this document and all attachments were prepared under my direction or supervision in accordance with a system designed to assure that qualified personnel properly gathered and evaluated the information submitted. Based on my inquiry of the person or persons who manage the system, or those persons directly responsible for gathering the information, the information submitted is, to the best of my knowledge and belief, true, accurate, and complete. I am aware that false statements made herein are punishable as a class A misdemeanor pursuant to Section 210.45 of the Penal Law."

<u>Signatory Requirements</u> - All NOIs, NOTs, SWPPPS, reports, certifications or information required by this permit or submitted pursuant to this permit, shall be signed as follows:

- 1 For a corporation: by a (1) president, secretary, treasurer, or vice-president of the corporation in charge of a principle business function, or any other person authorized to and who performs similar policy or decision-making functions for the corporation; or (2) the manager of one or more manufacturing, production or operating facilities employing more than 250 persons or having a gross annual sales or expenditures exceeding \$25,000,000 (in second-quarter 1980 dollars) if authority to sign documents has been assigned or delegated to manage in accordance with corporate procedures;
- 2 For a partnership or sole proprietorship: by a general partner or the proprietor, respectively; or
- <sup>3</sup> For a municipality, State, Federal, or other public agency; by either a principal executive officer ranking elected official. For purposes of this section, a principal executive officer of a Federal agency includes (1) the chief executive officer of the agency, or (2) a senior executive officer having responsibility for the overall operations of a principal geographic unit of the agency (e.g. Regional Administrators of EPA).

Chazen Engineering & Land Surveying Co., P.C. EnviroPlan Associates, Inc.



Chazen Environmental Services, Inc TelePlan Associates, Inc.

# Appendix D: Construction Phase Inspections and Maintenance (Pocket)

### CONSTRUCTION PHASE INSPECTIONS AND MAINTENANCE

Between the time this SWPPP is implemented and final site stabilization is achieved, all disturbed areas and pollutant controls must be inspected at least once every seven calendar days and within 24 hours following a rainfall of 0.5 inches or greater. The purpose of site inspections is to assess performance of pollutant controls. The inspections will be conducted by the Operator's Engineer. Based on these inspections, the Operator's Engineer will decide whether it is necessary to modify this SWPPP, add or relocate sediment barriers, or whatever else may be needed in order to prevent pollutants from leaving the site via storm water runoff. The general contractor has the duty to cause pollutant control measures to be repaired, modified, maintained, supplemented, or whatever else is necessary in order to achieve effective pollutant control.

Examples of particular items to evaluate during site inspections are listed below. This list is not intended to be comprehensive. During each inspection the inspector must evaluate overall pollutant control system performance as well as particular details of individual system components. Additional factors should be considered as appropriate to the circumstances.

- 1. Locations where vehicles enter and exit the site must be inspected for evidence of off-site sediment tracking. A stabilized construction entrance will be constructed where vehicles enter and exit. This entrance will be maintained or supplemented as necessary to prevent sediment from leaving the site on vehicles.
- 2. Sediment barriers must be inspected and, if necessary, they must be enlarged or cleaned in order to provide additional capacity. All material from behind sediment barriers will be stockpiled on the up slope side. Additional sediment barriers must be constructed as needed.
- 3. Inspections will evaluate disturbed areas and areas used for storing materials that are exposed to rainfall for evidence of, or the potential for, pollutants entering the drainage system. If necessary, the materials must be covered or original covers must be repaired or supplemented. Also, protective berms must be constructed, if needed, in order to contain runoff from material storage areas.
- 4. Grassed areas will be inspected to confirm that a healthy stand of grass is maintained. The site has achieved final stabilization once all areas are covered with building foundation or pavement, or have a stand of grass with at least 80 percent density. The density of 80 percent or greater must be maintained to be considered as stabilized. Areas must be watered, fertilized, and reseeded as needed to achieve this goal.

5. All discharge points must be inspected to determine whether erosion control measures are effective in preventing significant impacts to receiving waters.

Based on inspection results, any modification necessary to increase effectiveness of this SWPPP to an acceptable level must be made within seven calendar days of the inspection. The inspection reports must be completed entirely and additional remarks should be included if needed to fully describe a situation. An important aspect of the inspection report is the description of additional measures that need to be taken to enhance plan effectiveness. The inspection report must identify whether the site was in compliance with the SWPPP at the time of inspection and specifically identify all incidents of non-compliance. An erosion and sediment control inspections and maintenance schedule is presented on the project drawings.

Inspection reports must be kept on file by the general contractor as an integral part of this SWPPP for at least three years from the date of completion of the project.

Ultimately, it is the responsibility of the general contractor to assure the adequacy of site pollutant discharge controls. Actual physical site conditions or contractor practices could make it necessary to install more structural controls than are shown on the accompanying plans. (For example, localized concentrations of runoff could make it necessary to install additional sediment barriers.) Assessing the need for additional controls and implementing them or adjusting existing controls will be a continuing aspect of this SWPPP until the site achieves final stabilization.

# Appendix E: Inspection Report (Sample Form)

### Silo Ridge Country Club Golf Resort Community Stormwater Pollution Prevention Plan Construction Site Log Book

Practice	Condition	Accumulation %	Conforming	<b>Actions Required</b>
Femporary Stockpiles	Good / Fair / Poor		Yes / No	
Dust Control	Good / Fair / Poor		Yes / No	
Sedimentation on Public Streets	Good / Fair / Poor		Yes / No	
Less than 5 Acres of Disturbance	Good / Fair / Poor		Yes / No	
Other:	Good / Fair / Poor		Yes / No	
Other:	Good / Fair / Poor		Yes / NJ	
Inspection Notes:				
Inspectors Signature: Inspectors Frintedure: Qualified Professional			Date	

Chazen Engineering & Land Surveying Co., P.C. EnviroPlan Associates, Inc.



Chazen Environmental Services, Inc. TelePlan Associates, Inc.

#### Silo Ridge Country Club Golf Resort Community Stormwater Pollution Prevention Plan Construction Site Log Book

Inspection Type	e (Circle One):	Routine Weekly	Following 1	1/2" or Greater Rainfall
	SWPPP Silo Ridge Cour	sment / Inspec Construction A ntry Club Golf Re Town of Amenia hess County, New	Activities sort Communit	у
Report Number:			Weather:	
Inspectors Name (Please	Print):		Temperature:	
Date:Tir	ne:		Page:	
<b>Observation Instruction</b>	0761			
		licturbod cito oroco	and drainage noth	s. Indicate te areas are
	ate the extent of all d initial disturbance or			
2 Indicate on a site ma	ap all areas of the site	that have undergone	e voorary or per	nanent cabilization.
3 Indicate on a site ma	ap all areas that have	not undergene a	e site v. 👌 during	th previous 14-day period.
<sup>4</sup> Inspect all sediment control practices and receive the approximate deg. In a sediment accumulation as a percentage of the sediment storage volume. Note in coubstantial increase in turbidity in downstream water courses/bodies exists.				
the integrity of sedin		or live on systems	silt fence, div	equirements such as verifying ersion swales, earthen berms,
6 Inspect all equipment, material handling storage are for evidence of apparent spills, leaks or deleterious materials.				
7 On a monthly basis, purposed of the Inspection Log in a publicly accessible location.				
Temporary Erosion and S. dimen strol J. ractices:				
Pra_tice	Condition	Accumulation %	Conforming	Actions Required
Stabilized 'onstruct' Entrance	od /r / Poor		Yes / No	
Temporary Parking	God / Fair / Poor		Yes / No	
Silt Fence	C .od / Fair / Poor	1	Yes / No	
Temporary Swales an Berms	Good / Fair / Poor		Yes / No	
Check Dams (Stone)	Good / Fair / Poor		Yes / No	
Slope Protection	Good / Fair / Poor		Yes / No	
Dewatering Operations	Good / Fair / Poor		Yes / No	
Sediment Traps	Good / Fair / Poor		Yes / No	

Chazen Engineering & Land Surveying Co., P.C. EnviroPlan Associates, Inc.

Good / Fair / Poor

Good / Fair / Poor

Inlet Protection

Mulching / Seeding



See attached pages for additional comments

Chazen Environmental Services, Inc. TelePlan Associates, Inc.

Yes / No

Yes / No

# Appendix F: Record of Stabilization and Construction Activity Dates (Sample Form)

### Site Stabilization & Construction Activities Dates Silo Ridge Country Club Golf Resort Community Town of Amenia Dutchess County, New York

Note: This form shall be completed by the Contractor and shall remain as part of the Storm water Pollution Prevention Plan that is to remain at the project site for the duration of construction.

A record of dates when major grading activities occur, when construction activities temporarily or permanently cease on a portion of the site, and when stabilization measures are initiate is a like non-intained until final site stabilization is achieved and the Notice of Termination is filed.

MAJOR GRADING ACTIVITIES:	Fage of
Description of Activity:	
Contractor:	
Location:	
Start Date:	Finis. Pate:
Description of Activity:	
Contractor:	
Location:	
Start Date:	Finish de:
Description of Activity:	
Contractor:	
Location:	·
Start Date:	_Finish Date:
Descriptio. of Activit	
Contractor:	
Location:	
Start Date:	_Finish Date:
Description of Activity:	
Contractor:	
Location:	
	_Finish Date:



Chazen Environmental Services, Inc. TelePlan Associates, Inc.

# Appendix G: Notice of Termination (NOT)

New York State Department of Environmental Conservation Division of Water 625 Broadway, 4th Floor Albany, New York 12233-3505				
<b>NOTICE OF TERMINATION</b> for Storm Water Discharges Associated with Construction Activity UNDER SPDES GENERAL PERMIT:   #GP-93-06 or  #GP-02-01				
Please indicate your permit identification number:   NYR				
I. Permittee Information				
1. Owner/Operator Name:		T		
2a. Mailing Address:	T	2b. City/State/Zig	p:	
3a. Contact Person:	3b. Phone:		3c. E-mail:	
II. Site /Activity Information				
4. Facility/Project Site Name:		T		
5a. Street Address:		5b. City/State/Zip:		
6. County:				
III. Reason for Termination				
<ul> <li>7a. □ Site has been finally stabilized in accordance with permit and SWPPP. Date site stabilization completed:</li></ul>				
IV. Final Site Information:				
<ul> <li>8a. Are there permanent stormwater management practices remaining on the site? □ yes □ no If the answer to question 8a. is no, go to question 8e. If the answer to question 8a. is yes, answer the following questions 8b., 8c., and 8d.: </li> <li>8b. Is the design and function of each permanent practice described in the final SWPPP? □ yes □ no 8c. Who will be responsible for long-term operation and maintenance of practice(s)? 8d. Has the individual(s) responsible for long-term operation and maintenance been given a copy of the operation and maintenance requirements? □ yes □ no 8e. Provide the total acreage of impervious surface (i.e. roof, pavement, concrete, gravel, etc.) constructed within the disturbance area?</li></ul>				
V. Certification				
I certify under penalty of law that this document was prepared under my direction or supervision in accordance with a system designed to assure that qualified personnel properly gather and evaluate the information submitted. Based on my inquiry of the person(s) who manage the system, or those persons directly responsible for gathering the information, the information submitted is, to the best of my knowledge and belief, true, accurate and complete. I am aware that there are significant penalties for submitting false information, including the possibility of fine and imprisonment for knowing violations.				
Printed Name:		Title/Position	n:	
Signature: Date:				

# Appendix H: Post-Construction Inspections and Maintenance (Pocket)

## POST CONSTRUCTION INSPECTIONS AND MAINTENANCE

### 1. SITE COVER

### a. Inspections

Site cover and associated structures and embankments should be inspected periodically for the first few months following construction and then on a biannual basis. Site inspections should also be performed following all major (i.e., intense storms, thunder storms, cloud burst, etc.) storm events. Items to check for include (but are not limited to):

- i. Differential settlement of embankments, cracking or erosion.
- ii. Lack of vigor and density of grass turf.
- iii. Accumulation of sediments or litter on lawn areas, paved areas, or within catch basin sumps.
- iv. Accumulation of pollutants, including oils or grease, in catch basin sumps.
- v. Damage or fatigue of storm sewer structures or associated components.

## b. Mowing and Sweeping

Vegetated areas and landscaping should be maintained to promote vigorous and dense growth. Lawn areas should be mowed at least three times a year (more frequent mowing may be desired for aesthetic reasons). Resultant yard waste shall be collected and disposed of off-site.

Paved areas should be swept at least twice a year. Additional sweeping may be appropriate in the early spring for removal of deicing materials

### c. Debris and Litter Removal

Accumulation of litter and debris should be removed during each mowing or sweep operation.

## d. Structural Repair or Replacement

Components of the system which require repair or replacement should be addressed immediately following identification.

## e. Catch Basins

The frequency for cleanout of catch basin sumps will depend on the efficiency of mowing, sweeping and debris and litter removal. Sumps should be cleaned when accumulation of sediments are within six inches of the catch basin outlet pipe.

## f. Grassed Swales

Swale maintenance will include periodic mowing, occasional spot reseeding and weed control to keep grass cover dense and vigorous. Resultant yard waste shall be collected and disposed of off-site. Application of fertilizers and pesticides should be restricted or limited.

## g. Winter Maintenance

To prevent impacts to storm water management facilities, the following winter maintenance limitations, restrictions or requirements are recommended:

- i. Remove snow and ice from inlet structures, basin inlet and outlet structures and away from culvert end sections.
- ii. Snow removed from paved areas should not be piled at inlets/outlets of the storm water management basin.
- iii. Use of deicing materials should be limited to sand and "environmentally friendly" chemical products. Use of salt mixtures should be kept to a minimum.
- iv. Sand used for deicing should be clean, course material free of fines, silt, and clay.
- v. Materials used for deicing should be removed during the early spring by sweeping and/ or vacuuming.

## 2. DETENTION BASINS

### a. Inspections

Detention Basins should be inspected periodically for the first few months after construction and then on an annual basis. Detention Basins should be inspected after major storm events to ensure inlets and outlets remain clear. Items to check for include (but are not limited to):

- i. Differential settlement of embankments.
- ii. Cracking, erosion or seepage through embankments.

- iii. Evidence of clogging at inlets or outlets.
- iv. Erosion of the flow path through the detention basin.
- v. Brush, shrub or tree growth on embankments.
- vi. Condition of the overflow spillway.
- vii. Lack of vigor and density of grass turf on the basin embankments.

## b. Mowing

The side slopes, embankments, inlets, and overflow spillways of the detention basins should be mowed at least three times a year and resultant yard wastes collected and disposed of off-site.

## c. Debris and Litter Control

Removal of debris and litter should be accomplished during mowing operations. Inlet and outlet structures should be cleared of all debris and litter.

## d. Structural repairs and Replacement

Components of the detention basin, which require repair or replacement, should be addressed immediately following identification.

## e. Erosion Control

Sources of sedimentation, specifically eroded areas in upland drainage areas, should be stabilized immediately upon identification. Stabilization should be with vegetative practices or other erosion control practices when vegetative measures do not prove effective.

Soil slumpage, erosion of the basin embankment or around inlets/outlets, and cracking should be stabilized and repaired immediately upon identification. Repair, replacement or addition of rip-rap aprons, channels or embankments should be pursued as required.

## f. Sediment removal

Sediments, which accumulate in the detention basin, should be removed periodically to prevent clogging of inlet or outlet structures. A typical clean-out cycle should be between 5 to 10 years with more frequent cleanings near inlet and outlet structures.

## 3. FOREBAYS AND WET POOLS

### a. Inspections

Forebays and Wet Pools should be inspected periodically for the first few months after construction and then on an annual basis. Forebays and Wet Pools should be inspected following all major storm events. Items to check for include (but are not limited to):

- i. Differential settlement of embankments.
- ii. Cracking, erosion or seepage through embankments.
- iii. Erosion of the flow path through the facility.
- iv. Brush, shrub or tree growth on embankments.
- v. Condition of the overflow spillway.
- vi. Accumulation of sediment.

## b. Mowing

Tree and brush growth must be prevented on basin embankments, side slopes, bottoms and around inlets and the overflow spillway(s). Mowing of the embankments shall be at least three times a year unless more frequent mowing is required to control vegetative growth. Resultant yard waste shall be collected and disposed of off-site.

### c. Debris and Litter Removal

Removal of debris and litter shall be accomplished during mowing operations. Inlet structures should be cleared of all debris and litter.

### d. Structural Repairs and Replacement

Components of the forebay or wet pool, which require repair or replacement, should be addressed immediately following identification.

### e. Erosion Control

Sources of sedimentation, specifically eroded areas in upland drainage areas, should be stabilized immediately upon identification. Stabilization should be with vegetative practices or other erosion control practices when vegetative measures do not prove effective.

Soil slumpage, erosion of the basin embankment or around inlets or overflow outlets, and cracking should be stabilized and repaired immediately upon identification. Repair, replacement or addition of rip-rap aprons, channels or embankments should be pursued as required.

## f. Sediment Removal

Cleanout frequency of Forebays and Wet Pools is dependent upon bottom cover, storage capacity, volume of inflow, and sediment load.

Sediment shall be removed from the Forebays and Wet Pools every 5 to 6 years or when accumulations reach 12 inches in depth. Monitoring the depth of sediments can be measured by installing permanent markers in the newly constructed facilities with a mark 12 inches above the permanent water surface. Markers should not be spaced more than 50 feet apart along the flow path through the facility. A log should be kept indicating the date that the facility was inspected and the distance to the bottom.

When sediment removal is required, the original grades depicted on the project drawings should be reestablished by a qualified contractor. If any of the impermeable material used in the construction of the basin bottom is removed it must be replaced with clean material consistent with the original material specifications.

### 4. Aquatic Benches

### a. Inspection

Aquatic Benches should be inspected periodically for the first few months after construction and then on an annual basis. Aquatic Benches should be inspected after all major storm events. Items to check for include (but are not limited to):

- i. Checking basin embankments for subsidence, erosion, cracking, tree growth and the presence of burrowing animals.
- ii. Health and vigor of wetland vegetation.
- iii. Accumulation of sediment.

### b. Mowing

Mowing is not desirable nor allowed in the Aquatic Benches. Trees and shrubs should be removed from around inlet and outlet structures. Removal should be biannual.

# c. Debris, Trash and Litter Control

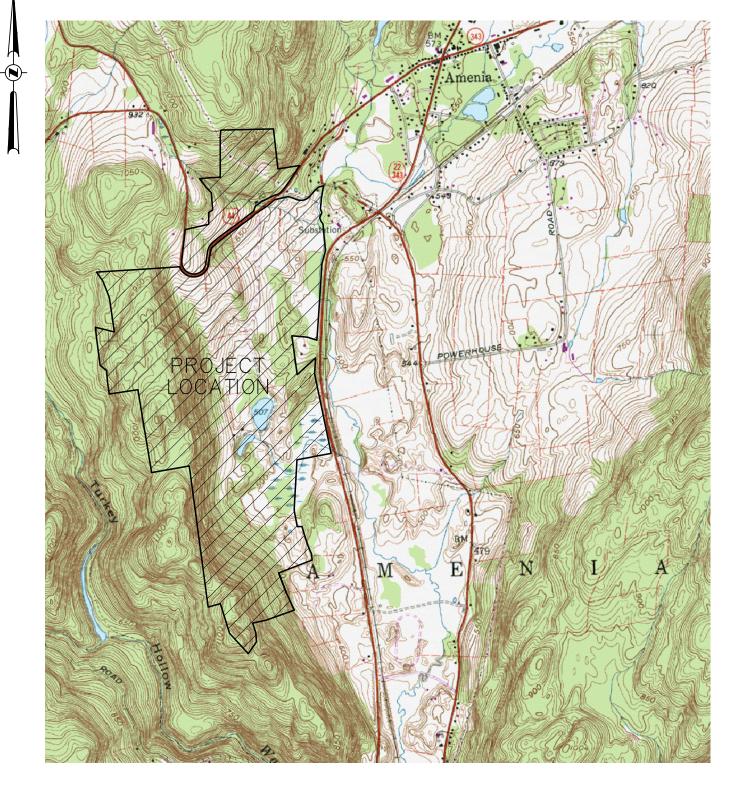
Debris, trash, and litter should be removed from the Aquatic Benches immediately upon discovery.

# d. Erosion Control

Soil slumpage, erosion of the Aquatic Bench embankment or around inlets or outlets, and cracking should be stabilized and repaired immediately upon identification.

# Appendix I: Figures

# Figure 1: Site Location Map

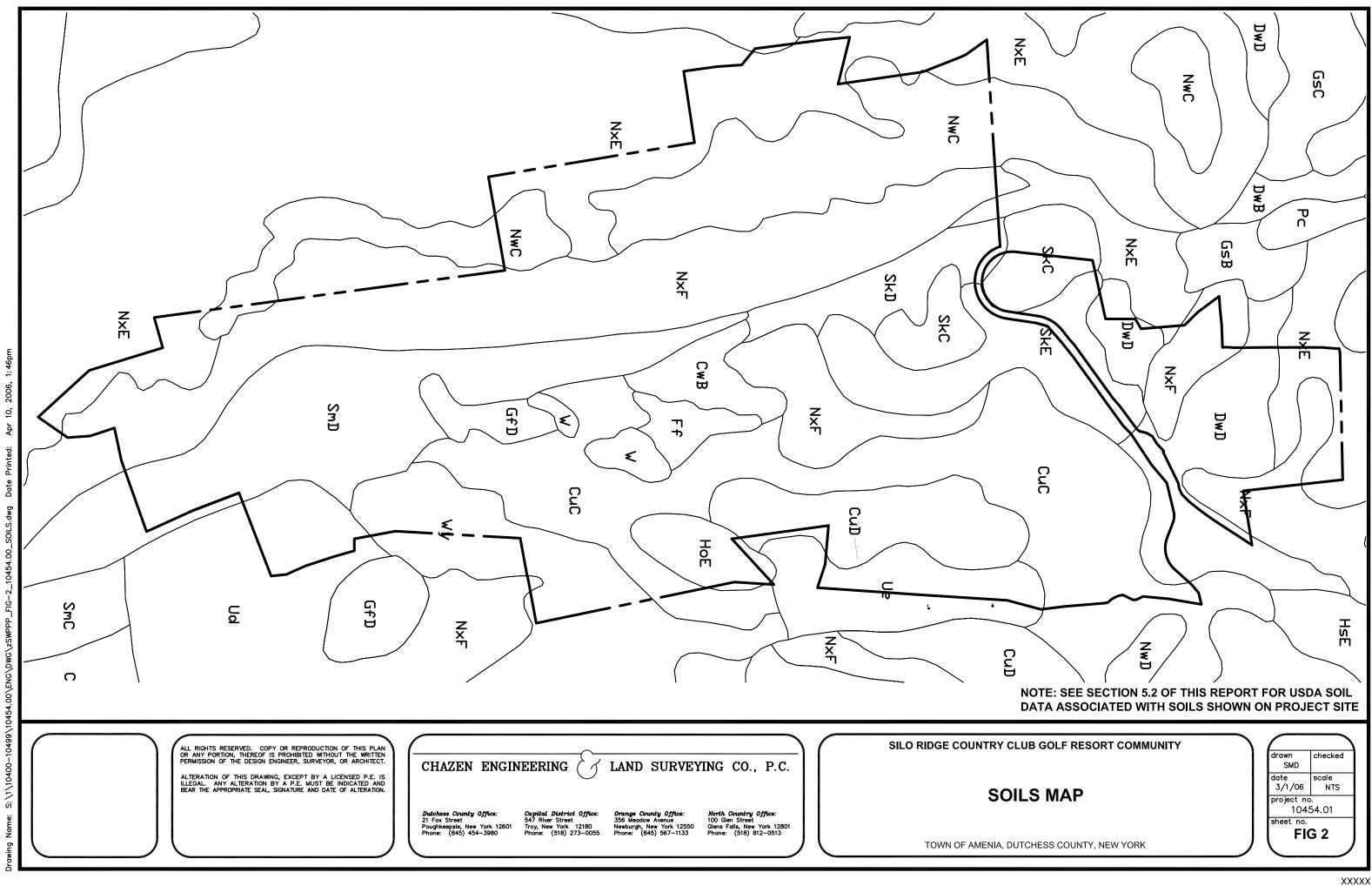


ALTERATION OF THIS DRAWING, EXCEPT BY A LICENSED P.E. IS ILLEGAL. ANY ALTERATION BY A P.E. MUST BE INDICATED AND BEAR THE APPROPRIATE SEAL, SIGNATURE AND DATE OF ALTERATION.

THE	Dutchess County Office: 21 Fox Street Poughkeepsie, NY 12601 Phone: (845) 454-3980	SILO RIDGE RESORT COMMUNITY	drawn	checked
COMPANIES Engineers/Surveyors Planners	Capital District Office: 547 River Street Troy, NY 12180 Phone: (518) 273-0055 Orange County Office: 356 Meadow Avenue Newburgh, NY 12550 Phone: (845) 567-1133	SITE LOCATION MAP	TCC date 2005 project no 1045 sheet no.	scale NTS 54.01
Environmental Scientists	North Country Office: 100 Glen Street Glens Falls, NY 12801 Phone: (518) 812-0513 OR REPRODUCTION OF THIS PLAN OR ANY PORT	TOWN OF AMENIA / DUTCHESS COUNTY / STATE OF NEW YORK	FIC	

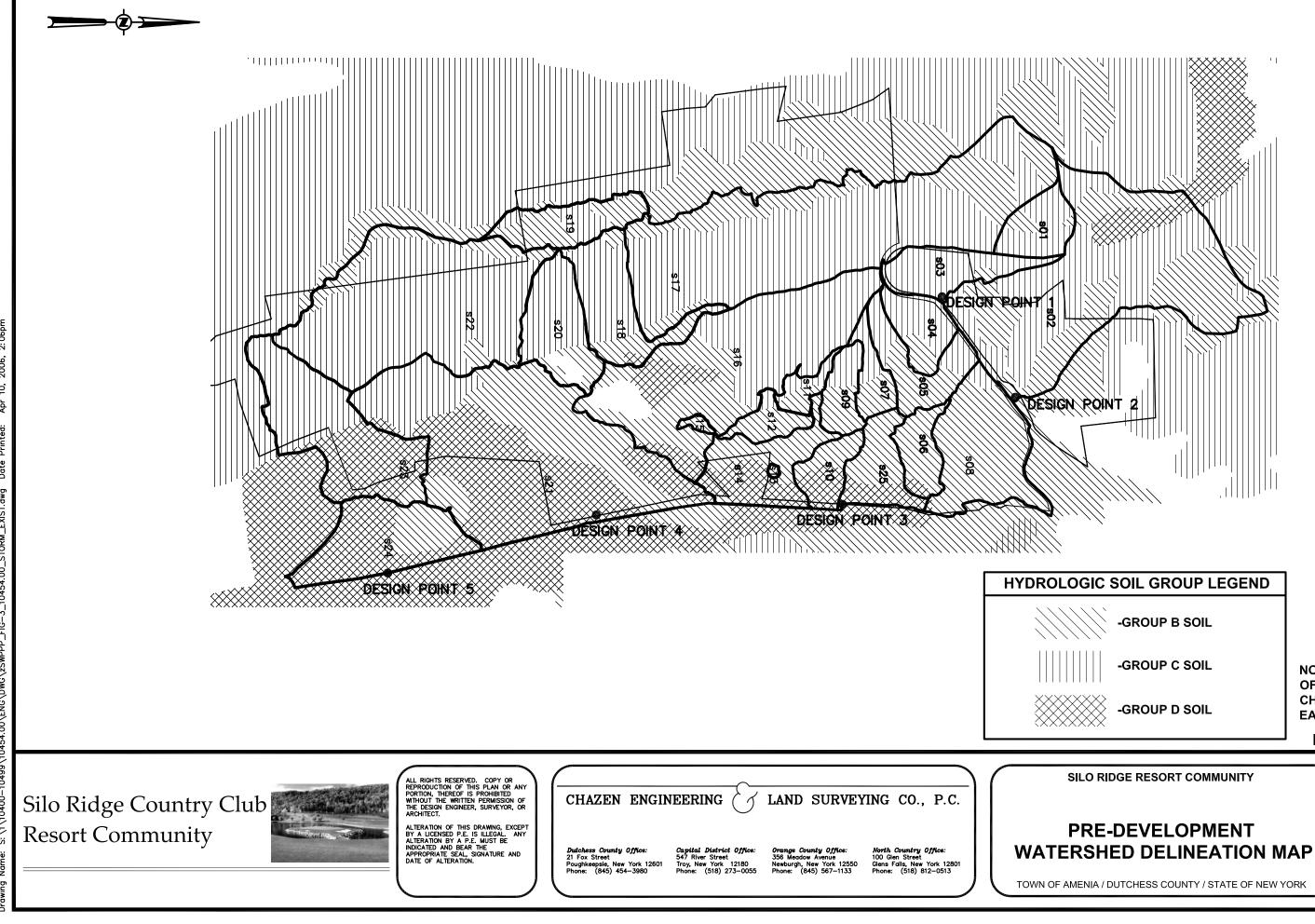
Drawing Name: S: \1\10400-10499\10454.00\ENG\DWG\zSWPPP\_FIG-1\_10454-00\_location.dwg Date Printed: Apr 10, 2006, 1:36pm

# Figure 2: Soils Map



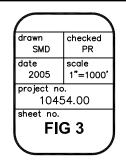
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# Figure 3: Pre-Development Watershed Delineation Map

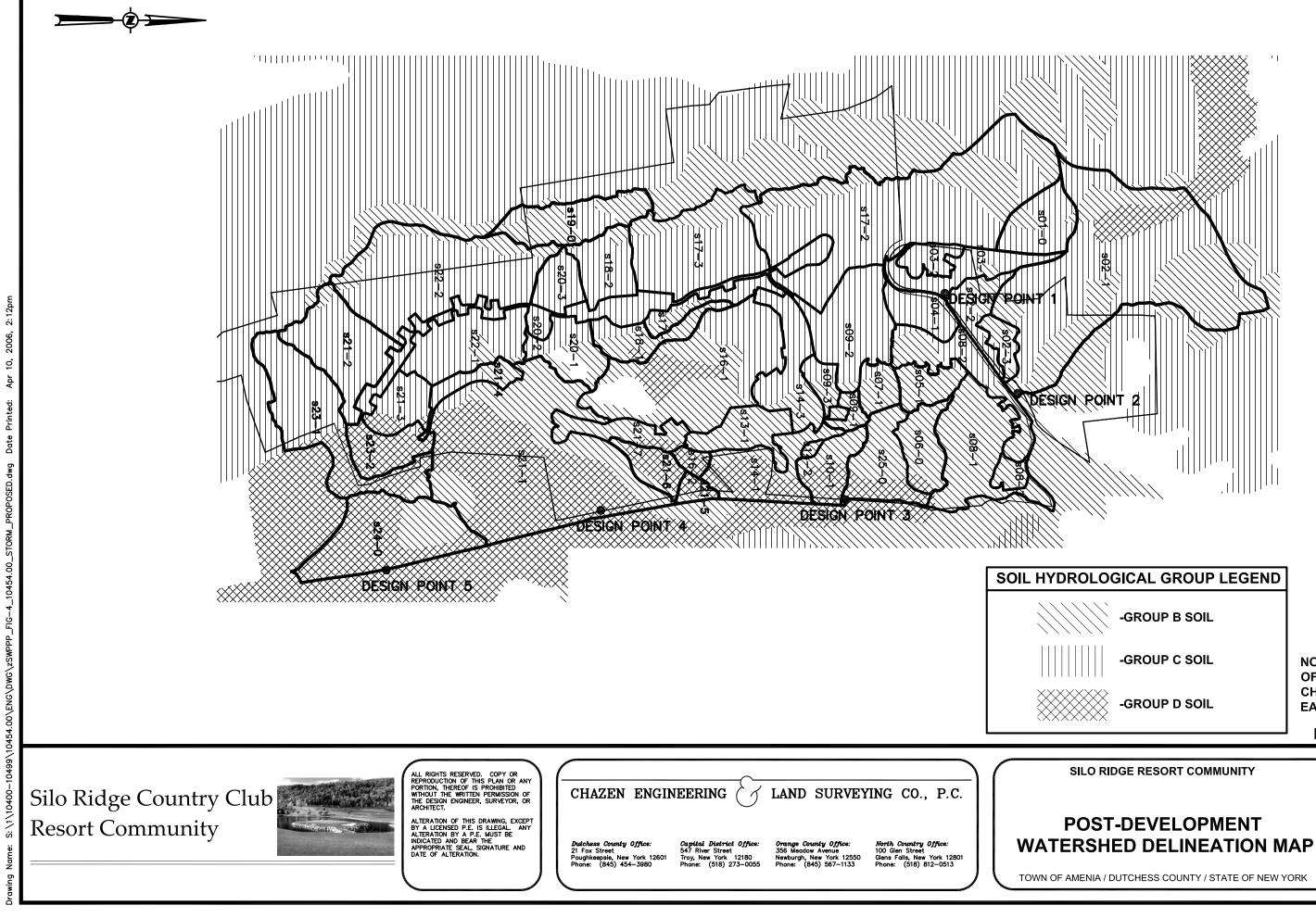


**NOTE: SEE APPENDIX L** OF THIS REPORT FOR CHARACTERISTICS OF EACH SUBCATCHMENT

**INTERNAL REVIEW** 



# Figure 4: Post-Development Watershed Delineation Map

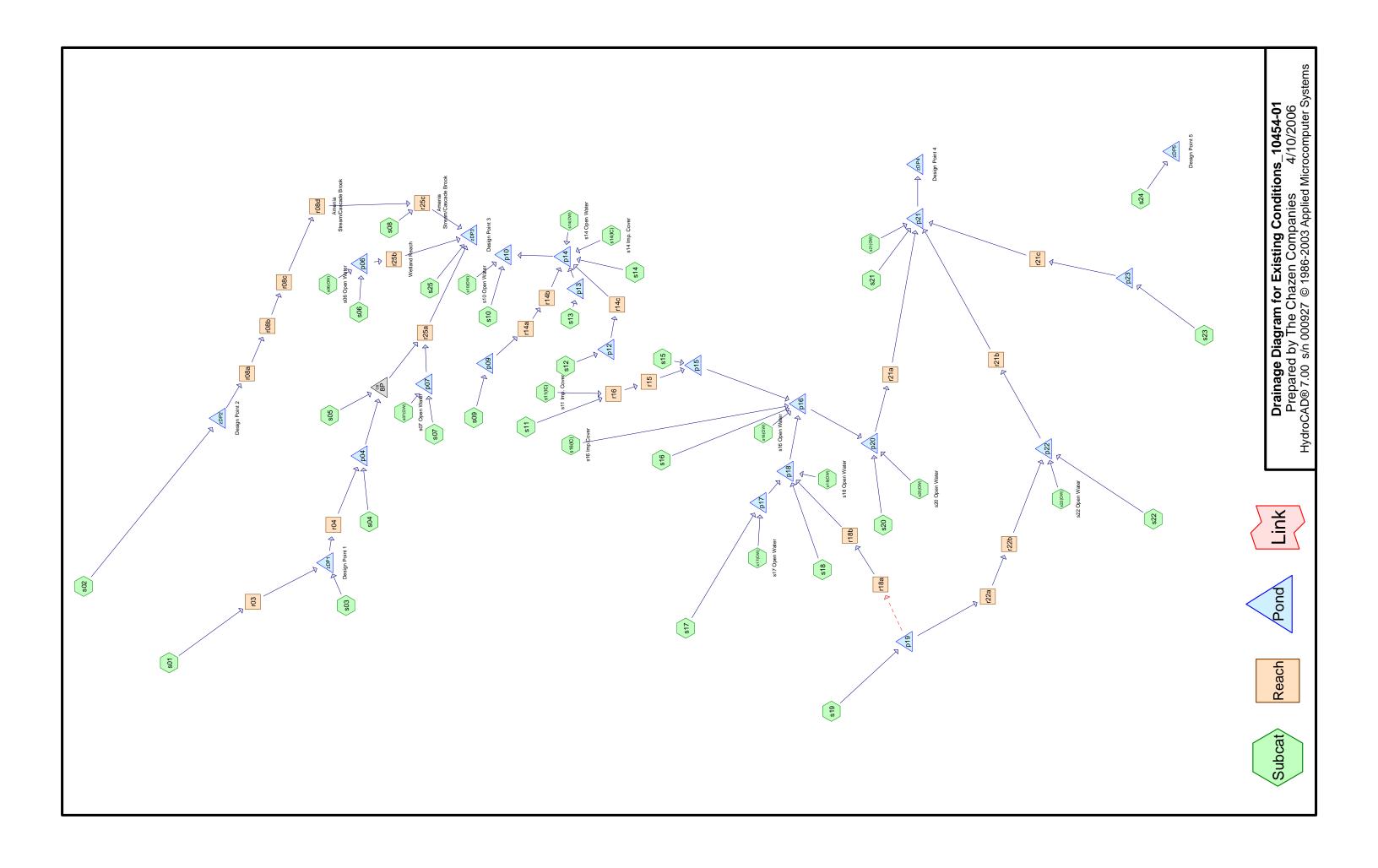


NOTE: SEE APPENDIX L OF THIS REPORT FOR **CHARACTERISTICS OF** EACH SUBCATCHMENT

**INTERNAL REVIEW** 

drawn checked SMD PR date scale 2005 1"=1000' project no. 10454.00 sheet no. FIG 4

# Appendix J: Pre-Development Watershed Conditions Modeling



# Pre-Development Conditions 1 year 24 hour Storm Event Model Computations

Existing Conditions_10454-01 Prepared by The Chazen Companies	Type III 24-hr 1-yr Rainfall=2.70" Page 1			
HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer System	ns 4/10/2006 2:33:07 PM			
Subcatchment s01:				
Runoff = 2.38 cfs @ 12.70 hrs, Volume= 0.458 af,	Depth= 0.48"			
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, o Type III 24-hr 1-yr Rainfall=2.70"	dt= 0.01 hrs			
Area (ac) CN Description 11.485 68				
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)				
42.8 Direct Entry,				
Subcatchment s02:				
Runoff = 11.49 cfs @ 13.01 hrs, Volume= 3.061 af,	Depth= 0.38"			
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, o Type III 24-hr 1-yr Rainfall=2.70"	dt= 0.01 hrs			
Area (ac) CN Description 97.712 65				
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)				
61.3 Direct Entry,				
Subcatchment s03:				
Runoff = 4.23 cfs @ 12.49 hrs, Volume= 0.652 af,	Depth= 0.52"			
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 1-yr Rainfall=2.70"				
Area (ac) CN Description				
15.174 69				
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)				
28.8 Direct Entry,				
Subcatchment s04:				
Runoff = 3.09 cfs @ 12.13 hrs, Volume= 0.357 af,	Depth= 0.38"			
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 1-yr Rainfall=2.70"				

	g Condit					Type II	ll 24-hr 1-yr Rainfall=2.70"
			n Compar 7 © 1986-2		d Microcomputer Sys	stems	Page 2 4/10/2006 2:33:07 PM
							1,10,2000 2.00.01 1 M
Area (	/		cription				
11.4	403 65	)					
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description		
6.5	х <i>г</i>		<b>.</b>		Direct Entry,		
				Subc	atchment s05:		
Runoff	=	1.62 cfs	s@ 12.4	7 hrs, Volu	ime= 0.322	af, Depth=	= 0.26"
	/ SCS TR 4-hr 1-yr			SCS, Time S	Span= 0.00-48.00 h	nrs, dt= 0.01	hrs
<u>Area (</u> 14.9			cription				
(min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description		
17.3					Direct Entry,		
Subcatchment s06:							
Runoff	=	1.16 cfs	s@ 12.4	4 hrs, Volu	ime= 0.215	af, Depth=	= 0.29"
	v SCS TR 4-hr 1-yr			SCS, Time S	Span= 0.00-48.00 h	nrs, dt= 0.01	hrs
Area (			cription				
9.0	007 62						
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description		
17.3					Direct Entry,		
			Subcat	chment s	606(OW): s06 Op	oen Water	
Runoff	=	1.39 cfs	s@ 12.0	0 hrs, Volu	ime= 0.096	af, Depth=	= 2.70"
	/ SCS TR 4-hr 1-yr			SCS, Time S	Span= 0.00-48.00 h	nrs, dt= 0.01	hrs
<u>Area (</u> 0.4	<u>ac) CN</u> 428 100		cription				

Existing Conditions_10454-01Type III 24-hr 1-yr Rainfall=2.70"Prepared by The Chazen CompaniesPage 3HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems4/10/2006 2:33:07 PM				
Subcatchment s07:				
Runoff = 1.26 cfs @ 12.30 hrs, Volume= 0.196 af, Depth= 0.34"				
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 1-yr Rainfall=2.70"				
Area (ac) CN Description 6.811 64				
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)				
13.9Direct Entry,				
Subcatchment s07(OW): s07 Open Water				
Runoff = 1.64 cfs @ 12.00 hrs, Volume= 0.114 af, Depth= 2.70"				
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 1-yr Rainfall=2.70"				
Area (ac) CN Description 0.506 100				
Subcatchment s08:				
Runoff = 1.75 cfs @ 12.60 hrs, Volume= 0.488 af, Depth= 0.18"				
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 1-yr Rainfall=2.70"				
Area (ac) CN Description				
31.719 58				
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)				
21.8 Direct Entry,				
Subcatchment s09:				
Runoff = 1.74 cfs @ 12.33 hrs, Volume= 0.265 af, Depth= 0.38"				
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 1-yr Rainfall=2.70"				
Area (ac) CN Description 8.452 65				

Existing Conditions_10454-01 Prepared by The Chazen Companies	Type III 24-hr 1-yr Rainfall=2.70" Page 4			
HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer System	ns 4/10/2006 2:33:07 PM			
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)				
16.4Direct Entry,				
Subcatchment s10:				
Runoff = 1.84 cfs @ 12.49 hrs, Volume= 0.300 af,	Depth= 0.44"			
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, Type III 24-hr 1-yr Rainfall=2.70"	dt= 0.01 hrs			
Area (ac) CN Description				
8.130 67				
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)				
27.9 Direct Entry,				
Subcatchment s10(OW): s10 Open	Water			
Runoff = 2.69 cfs @ 12.00 hrs, Volume= 0.187 af,	Depth= 2.70"			
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, Type III 24-hr 1-yr Rainfall=2.70"	dt= 0.01 hrs			
Area (ac) CN Description				
0.830 100				
Subcatchment s11:				
Runoff = 0.75 cfs @ 12.35 hrs, Volume= 0.102 af,	Depth= 0.52"			
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 1-yr Rainfall=2.70"				
Area (ac) CN Description				
2.364 69				
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)				
20.5 <b>Direct Entry</b> ,				
Subcatchment s11(IC): s11 Imp. Cover				
Runoff = 6.84 cfs @ 12.04 hrs, Volume= 0.481 af,	Depth= 2.47"			
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs				

Type III 24-hr 1-yr Rainfall=2.70"

Existing Conditions_10454-01	Type III 24-hr 1-yr Rainfall=2.70"
Prepared by The Chazen Companies HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Syster	Page 5 ms 4/10/2006 2:33:07 PM
Area (ac) CN Description 2.338 98	
2.000 00	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
2.8 <b>Direct Entry</b> ,	
Subcatchment s12:	
Runoff = 0.12 cfs @ 13.30 hrs, Volume= 0.065 af	, Depth= 0.12"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, Type III 24-hr 1-yr Rainfall=2.70"	dt= 0.01 hrs
Area (ac) CN Description	
6.420 55	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
42.0 Direct Entry,	
Subcatchment s13:	
Runoff = 0.01 cfs @ 12.49 hrs, Volume= 0.004 af	, Depth= 0.12"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, Type III 24-hr 1-yr Rainfall=2.70"	dt= 0.01 hrs
Area (ac) CN Description	
0.350 55	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
8.6 Direct Entry,	
Subcatchment s14:	
Runoff = 2.74 cfs @ 12.55 hrs, Volume= 0.519 af	, Depth= 0.34"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, Type III 24-hr 1-yr Rainfall=2.70"	dt= 0.01 hrs
Area (ac) CN Description	
18.066 64	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
28.1 Direct Entry,	

Existing Conditions_10454-01Type III 24-hr 1-yr Rainfall=Prepared by The Chazen CompaniesPHydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems4/10/2006 2:33:0				
Subcatchment s14(IC): s14 Imp. Cover				
Runoff = 7.09 cfs @ 12.03 hrs, Volume= 0.490 af, Depth= 2.47"				
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 1-yr Rainfall=2.70"				
Area (ac) CN Description				
2.380 98				
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)				
2.3 Direct Entry,				
Subcatchment s14(OW): s14 Open Water				
Runoff = 1.68 cfs @ 12.00 hrs, Volume= 0.117 af, Depth= 2.70"				
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 1-yr Rainfall=2.70"				
Area (ac) CN Description				
0.518 100				
Subcatchment s15:				
Runoff = 0.01 cfs @ 14.95 hrs, Volume= 0.005 af, Depth= 0.06"				
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 1-yr Rainfall=2.70"				
Area (ac) CN Description				
1.068 51				
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)				
13.1   Direct Entry,				
Subcatchment s16:				
Runoff = 14.70 cfs @ 12.42 hrs, Volume= 2.316 af, Depth= 0.41"				
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 1-yr Rainfall=2.70"				
Area (ac) CN Description				
67.994 66				

Existing Conditions_10454-01	Type III 24-hr 1-yr Rainfall=2.70"			
Prepared by The Chazen Companies HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Syster	Page 7 ns <u>4/10/2006 2:33:07 PM</u>			
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)				
22.3 Direct Entry,				
Subcatchment s16(IC): s16 Imp.0	Cover			
Runoff = 7.66 cfs @ 12.04 hrs, Volume= 0.541 af,	Depth= 2.47"			
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, Type III 24-hr 1-yr Rainfall=2.70"	dt= 0.01 hrs			
Area (ac) CN Description				
2.629 98				
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)				
2.9 Direct Entry,				
Subcatchment s16(OW): s16 Oper	Water			
Runoff = 17.34 cfs @ 12.00 hrs, Volume= 1.204 af,	Depth= 2.70"			
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, Type III 24-hr 1-yr Rainfall=2.70"	dt= 0.01 hrs			
Area (ac) CN Description 5.351 100				
Subcatchment s17:				
Runoff = 9.99 cfs @ 14.05 hrs, Volume= 3.945 af,	Depth= 0.41"			
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 1-yr Rainfall=2.70"				
Area (ac) CN Description				
115.827 66				
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)				
125.2 Direct Entry,				
Subcatchment s17(OW): s17 Open Water				
Runoff = 0.53 cfs @ 12.00 hrs, Volume= 0.037 af,	Depth= 2.70"			
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 1-yr Rainfall=2.70"				

5 =	ype III 24-hr 1-yr Rainfall=2.70"		
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Area (ac) CN Description			
0.164 100			
Subcatchment s18:			
Runoff = 5.39 cfs @ 12.44 hrs, Volume= 0.836 af, D	epth= 0.44"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= Type III 24-hr 1-yr Rainfall=2.70"	= 0.01 hrs		
Area (ac) CN Description			
22.654 67			
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)			
24.4   Direct Entry,			
Subcatchment s18(OW): s18 Open W	Vater		
Runoff = 1.53 cfs @ 12.00 hrs, Volume= 0.106 af, D	Depth= 2.70"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= Type III 24-hr 1-yr Rainfall=2.70"	= 0.01 hrs		
Area (ac) CN Description			
0.472 100			
Subcatchment s19:			
Runoff = 1.03 cfs @ 12.80 hrs, Volume= 0.301 af, D	epth= 0.23"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 1-yr Rainfall=2.70"			
Area (ac) CN Description			
15.520 60			
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)			
40.4Direct Entry,			
Subcatchment s20:			
Runoff = 4.84 cfs @ 12.58 hrs, Volume= 0.802 af, D	0.52"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= Type III 24-hr 1-yr Rainfall=2.70"	= 0.01 hrs		

Type III 24-hr 1-yr Rainfall=2.70"

Existing Conditions_10454-01 Prepared by The Chazen Companies HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems	Type III 24-hr 1-yr Rainfall=2.70" Page 9 s 4/10/2006 2:33:08 PM
Area (ac) CN Description	
18.655 69	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
33.9 Direct Entry,	
Subcatchment s20(OW): s20 Open	Water
Runoff = 6.38 cfs @ 12.00 hrs, Volume= 0.443 af,	Depth= 2.70"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, d Type III 24-hr 1-yr Rainfall=2.70"	lt= 0.01 hrs
Area (ac) CN Description 1.968 100	
Subcatchment s21:	
Runoff = 24.77 cfs @ 12.47 hrs, Volume= 3.830 af,	Depth= 0.48"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, d Type III 24-hr 1-yr Rainfall=2.70"	lt= 0.01 hrs
Area (ac) CN Description 96.056 68	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
26.7 Direct Entry,	
Subcatchment s21(OW):	
Runoff = 39.66 cfs @ 12.00 hrs, Volume= 2.753 af,	Depth= 2.70"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, d Type III 24-hr 1-yr Rainfall=2.70"	lt= 0.01 hrs
Area (ac) CN Description 12.235 100	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
0.0 Direct Entry,	

Existing Conditions_10454-01	Type III 24-hr 1-yr Rainfall=2.70"			
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Subcatchment s22:				
Runoff = 22.13 cfs @ 12.53 hrs, Volume= 3.536 af,	Depth= 0.52"			
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 1-yr Rainfall=2.70"				
Area (ac) CN Description				
82.287 69				
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)				
31.3 Direct Entry,				
Subcatchment s22(OW): s22 Open	Water			
Runoff = 0.44 cfs @ 12.00 hrs, Volume= 0.031 af,	Depth= 2.70"			
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, Type III 24-hr 1-yr Rainfall=2.70"	dt= 0.01 hrs			
Area (ac) CN Description				
0.136 100				
Subcatchment s23:				
Runoff = 12.15 cfs @ 12.74 hrs, Volume= 2.204 af,	Depth= 0.64"			
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, Type III 24-hr 1-yr Rainfall=2.70"	dt= 0.01 hrs			
Area (ac) CN Description				
41.587 72				
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)				
47.0 Direct Entry,				
Subcatchment s24:				
Runoff = 8.52 cfs @ 12.52 hrs, Volume= 1.308 af,	Depth= 0.55"			
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 1-yr Rainfall=2.70"				
Area (ac) CN Description				
28.325 70				

Existing Conditions_10454-01	Type III 24-hr 1-yr Rainfall=2.70"			
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Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)				
30.7 Direct Entry,				
Subcatchment s25:				
Runoff = 3.03 cfs @ 12.37 hrs, Volume= 0.462 af,	Depth= 0.41"			
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, Type III 24-hr 1-yr Rainfall=2.70"	dt= 0.01 hrs			
Area (ac) CN Description				
13.562 66				
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)				
20.2 Direct Entry,				
Reach r03:				
Overland Flow Reach Requires more survey				
Inflow Area = $11.485 \text{ ac}$ , Inflow Depth = $0.48"$ for 1-yr eventInflow = $2.38 \text{ cfs} @$ $12.70 \text{ hrs}$ , Volume= $0.458 \text{ af}$ Outflow = $2.35 \text{ cfs} @$ $12.76 \text{ hrs}$ , Volume= $0.458 \text{ af}$	Atten= 1%, Lag= 3.7 min			
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 3.0 fps, Min. Travel Time= 4.3 min Avg. Velocity = 1.4 fps, Avg. Travel Time= 9.2 min				
Peak Depth= 0.27' @ 12.76 hrs Capacity at bank full= 92.14 cfs Inlet Invert= 845.00', Outlet Invert= 728.00' 10.00' x 1.50' deep Parabolic Channel, n= 0.060 Length= 785.0' Slope= 0.1490 '/'				
Reach r04:				
Channel				
Inflow Area = $26.659 \text{ ac}$ , Inflow Depth = $0.50"$ for 1-yr eventInflow = $6.01 \text{ cfs} @$ $12.58 \text{ hrs}$ , Volume= $1.110 \text{ af}$ Outflow = $5.98 \text{ cfs} @$ $12.63 \text{ hrs}$ , Volume= $1.110 \text{ af}$	Atten= 1%, Lag= 2.6 min			
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 4.0 fps, Min. Travel Time= 2.8 min Avg. Velocity = 1.9 fps, Avg. Travel Time= 6.0 min				

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Peak Depth= 0.52' @ 12.63 hrs Capacity at bank full= 446.15 cfs Inlet Invert= 685.50', Outlet Invert= 608.00' 12.00' x 4.00' deep Parabolic Channel, n= 0.060 Length= 675.0' Slope= 0.1148 '/'

### Reach r08a:

Man Made Ditch Inverts of pipe to be surveyed

Inflow Are	a =	97.712 ac, Inflow Depth = 0.38" for 1-y	event
Inflow	=	11.49 cfs @ 13.02 hrs, Volume= 3	3.061 af
Outflow	=	11.48 cfs @ 13.03 hrs, Volume= 3	8.061 af, Atten= 0%, Lag= 0.5 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 6.8 fps, Min. Travel Time= 0.6 min Avg. Velocity = 3.6 fps, Avg. Travel Time= 1.0 min

Peak Depth= 0.40' @ 13.03 hrs Capacity at bank full= 81.88 cfs Inlet Invert= 607.00', Outlet Invert= 587.00' 10.00' x 1.00' deep Parabolic Channel, n= 0.027 Length= 226.0' Slope= 0.0885 '/'

#### Reach r08b:

24" HDPE Inverts to be surveyed

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 17.6 fps, Min. Travel Time= 0.3 min Avg. Velocity = 9.7 fps, Avg. Travel Time= 0.5 min

Peak Depth= 0.52' @ 13.03 hrs Capacity at bank full= 77.17 cfs Inlet Invert= 587.00', Outlet Invert= 557.75' 24.0" Diameter Pipe n= 0.012 Length= 295.0' Slope= 0.0992 '/'

### Reach r08c:

Ditch Pipe inverts to be surveyed

 Inflow Area =
 97.712 ac, Inflow Depth =
 0.38" for 1-yr event

 Inflow =
 11.48 cfs @
 13.03 hrs, Volume=
 3.061 af

 Outflow =
 11.47 cfs @
 13.06 hrs, Volume=
 3.061 af, Atten= 0%, Lag= 1.5 min

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Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 6.5 fps, Min. Travel Time= 1.5 min Avg. Velocity = 3.3 fps, Avg. Travel Time= 3.0 min

Peak Depth= 0.41' @ 13.06 hrs Capacity at bank full= 76.65 cfs Inlet Invert= 557.75', Outlet Invert= 512.00' 10.00' x 1.00' deep Parabolic Channel, n= 0.027 Length= 590.0' Slope= 0.0775 '/'

#### Reach r08d: Amenia Stream/Cascade Brook

(Steam) parallel to Route 22 Base Flow estimated from field observation (see field note 21)

97.712 ac, Inflow Depth = 19.87" for 1-yr event Inflow Area = Inflow 51.47 cfs @ 13.06 hrs, Volume= 161.772 af, Incl. 40.00 cfs Base Flow = 51.40 cfs @ 13.12 hrs, Volume= 161.528 af, Atten= 0%, Lag= 3.7 min Outflow = Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 3.2 fps, Min. Travel Time= 4.1 min Avg. Velocity = 3.0 fps, Avg. Travel Time= 4.4 min Peak Depth= 2.83' @ 13.12 hrs Capacity at bank full= 104.49 cfs Inlet Invert= 512.00', Outlet Invert= 504.00' 10.00' x 4.00' deep Parabolic Channel, n= 0.060 Length= 805.0' Slope= 0.0099 '/'

### Reach r14a:

Grass lined channel

 Inflow Area =
 8.452 ac, Inflow Depth =
 0.32" for 1-yr event

 Inflow =
 0.73 cfs @
 12.83 hrs, Volume=
 0.226 af

 Outflow =
 0.73 cfs @
 12.86 hrs, Volume=
 0.226 af, Atten= 0%, Lag= 1.8 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 2.7 fps, Min. Travel Time= 2.2 min Avg. Velocity = 1.2 fps, Avg. Travel Time= 5.1 min

Peak Depth= 0.17' @ 12.86 hrs Capacity at bank full= 325.42 cfs Inlet Invert= 542.00', Outlet Invert= 526.00' 10.00' x 3.00' deep Parabolic Channel, n= 0.027 Length= 360.0' Slope= 0.0444 '/'

#### Reach r14b:

30" HDPE Under Main Entrance Road

 Inflow Area =
 8.452 ac, Inflow Depth =
 0.32" for 1-yr event

 Inflow =
 0.73 cfs @
 12.86 hrs, Volume=
 0.226 af

 Outflow =
 0.73 cfs @
 12.87 hrs, Volume=
 0.226 af, Atten= 0%, Lag= 0.9 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 5.7 fps, Min. Travel Time= 1.3 min Avg. Velocity = 2.5 fps, Avg. Travel Time= 3.0 min

Peak Depth= 0.16' @ 12.87 hrs Capacity at bank full= 94.91 cfs Inlet Invert= 526.00', Outlet Invert= 505.70' 30.0" Diameter Pipe n= 0.012 Length= 445.0' Slope= 0.0456 '/'

#### Reach r14c:

#### **Overland Flow**

Inflow Area = Inflow = Outflow =	6.420 ac, Inflow Depth = 0.00"       for 1-yr event         0.00 cfs @       0.00 hrs, Volume=       0.000 af         0.00 cfs @       0.00 hrs, Volume=       0.000 af, Atten= 0%, Lag= 0.0 min	
Max. Velocity= 0.0	or-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs fps, Min. Travel Time= 0.0 min fps, Avg. Travel Time= 0.0 min	

# Reach r15:

Brush Overbanks with Rocky Bottom Needs to be surveyed

Inflow Area =4.702 ac, Inflow Depth = $1.49^{"}$  for 1-yr eventInflow =6.65 cfs @12.06 hrs, Volume=0.583 afOutflow =6.61 cfs @12.07 hrs, Volume=0.583 af, Atten= 1%, Lag= 0.5 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 7.9 fps, Min. Travel Time= 0.6 min Avg. Velocity = 2.7 fps, Avg. Travel Time= 1.8 min

Peak Depth= 0.58' @ 12.07 hrs Capacity at bank full= 188.47 cfs Inlet Invert= 554.00', Outlet Invert= 528.00' 5.00' x 3.00' deep Parabolic Channel, n= 0.027 Length= 290.0' Slope= 0.0897 '/'

# Reach r16:

#### Pipe Reach

Inflow Area =       4.702 ac, Inflow Depth =       1.49" for 1-yr event         Inflow =       6.95 cfs @       12.04 hrs, Volume=       0.583 af         Outflow =       6.65 cfs @       12.06 hrs, Volume=       0.583 af, Atten= 4%, Lag= 1.1 min						
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 8.6 fps, Min. Travel Time= 1.7 min Avg. Velocity = 2.8 fps, Avg. Travel Time= 5.1 min						
Peak Depth= 0.54' @ 12.06 hrs Capacity at bank full= 66.05 cfs Inlet Invert= 573.00', Outlet Invert= 554.00' 30.0" Diameter Pipe n= 0.012 Length= 860.0' Slope= 0.0221 '/'						
Reach r18a:						
Overland Flow Reach						
Inflow         =         0.00 cfs @         0.00 hrs, Volume=         0.000 af           Outflow         =         0.00 cfs @         0.00 hrs, Volume=         0.000 af, Atten= 0%, Lag= 0.0 min						
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 0.0 fps, Min. Travel Time= 0.0 min Avg. Velocity = 0.0 fps, Avg. Travel Time= 0.0 min						
Peak Depth= 0.00' @ 0.00 hrs Capacity at bank full= 379.63 cfs Inlet Invert= 973.60', Outlet Invert= 530.00' 50.00' x 1.00' deep Parabolic Channel, n= 0.060 Length= 1,220.0' Slope= 0.3636 '/'						
Reach r18b:						
Overland Flow Reach						
Inflow         =         0.00 cfs @         0.00 hrs, Volume=         0.000 af           Outflow         =         0.00 cfs @         0.00 hrs, Volume=         0.000 af, Atten= 0%, Lag= 0.0 min						
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 0.0 fps, Min. Travel Time= 0.0 min Avg. Velocity = 0.0 fps, Avg. Travel Time= 0.0 min						
Peak Depth= 0.00' @ 0.00 hrs Capacity at bank full= 151.94 cfs Inlet Invert= 530.60', Outlet Invert= 514.00' 50.00' x 1.00' deep Parabolic Channel, n= 0.060 Length= 285.0' Slope= 0.0582 '/'						

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# Reach r21a:

Man Made Ditch

Inflow Area =       241.484 ac, Inflow Depth =       0.07" for 1-yr event         Inflow =       1.23 cfs @       14.55 hrs, Volume=       1.339 af         Outflow =       1.23 cfs @       14.61 hrs, Volume=       1.337 af, Atten= 0%, Lag= 3.6 min				
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 2.2 fps, Min. Travel Time= 4.9 min Avg. Velocity = 1.3 fps, Avg. Travel Time= 8.3 min				
Peak Depth= 0.28' @ 14.61 hrs Capacity at bank full= 191.76 cfs Inlet Invert= 504.00', Outlet Invert= 494.00' 10.00' x 3.00' deep Parabolic Channel, n= 0.027 Length= 648.0' Slope= 0.0154 '/'				
Reach r21b:				
Grass Ditch Geometry to be confirmed by survey (inverts at pipe)				
Inflow Area =       97.943 ac, Inflow Depth =       0.44" for 1-yr event         Inflow =       20.54 cfs @       12.65 hrs, Volume=       3.568 af         Outflow =       20.52 cfs @       12.66 hrs, Volume=       3.568 af, Atten= 0%, Lag= 0.5 min				
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 5.7 fps, Min. Travel Time= 0.7 min Avg. Velocity = 1.6 fps, Avg. Travel Time= 2.3 min				
Peak Depth= 0.64' @ 12.66 hrs Capacity at bank full= 239.90 cfs Inlet Invert= 499.00', Outlet Invert= 491.10' 15.00' x 2.00' deep Parabolic Channel, n= 0.027 Length= 230.0' Slope= 0.0343 '/'				
Reach r21c:				
Overland Flow Reach				

**Overland Flow Reach** 

Inflow Area = 41.587 ac, Inflow Depth = 0.37" for 1-yr event Inflow = 4.95 cfs @ 13.57 hrs, Volume= 1.285 afOutflow = 4.91 cfs @ 13.59 hrs, Volume= 1.285 af, Atten= 1%, Lag= 1.4 min Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrsMax. Velocity= 2.1 fps, Min. Travel Time= 1.3 minAvg. Velocity = 1.1 fps, Avg. Travel Time= 2.4 min Existing Conditions\_10454-01 Type III 2 Prepared by The Chazen Companies HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems

Peak Depth= 0.17' @ 13.59 hrs Capacity at bank full= 227.81 cfs Inlet Invert= 506.70', Outlet Invert= 485.75' 50.00' x 1.00' deep Parabolic Channel, n= 0.060 Length= 160.0' Slope= 0.1309 '/'

### Reach r22a:

**Overland Flow Reach** 

Inflow Area = Inflow = Outflow =	15.520 ac, Inflow Depth =0.23"for1-yr event0.45 cfs @14.68 hrs, Volume=0.299 af0.45 cfs @14.81 hrs, Volume=0.299 af, Atten= 0%, Lag= 8.0 min					
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 1.5 fps, Min. Travel Time= 10.7 min Avg. Velocity = 0.8 fps, Avg. Travel Time= 19.2 min						
Peak Depth= 0.04' @ 14.81 hrs Capacity at bank full= 409.31 cfs Inlet Invert= 970.00', Outlet Invert= 560.00'						

50.00' x 1.00' deep Parabolic Channel, n= 0.060 Length= 970.0' Slope= 0.4227 '/'

### Reach r22b:

**Overland Flow Reach** 

Inflow Area = Inflow = Outflow =	15.520 ac, Inflow Depth = 0.23"for 1-yr event0.45 cfs @ 14.81 hrs, Volume=0.299 af0.45 cfs @ 14.98 hrs, Volume=0.299 af, Atten= 0%,	Lag= 9.7 min				
Max. Velocity= 0	or-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs fps, Min. Travel Time= 12.3 min fps, Avg. Travel Time= 23.8 min					
Peak Depth= 0.06' @ 14.98 hrs Capacity at bank full= 175.17 cfs Inlet Invert= 560.00', Outlet Invert= 512.00'						

50.00' x 1.00' deep Parabolic Channel, n= 0.060 Length= 620.0' Slope= 0.0774 '/'

#### Reach r25a:

Ditch Pipe inverts need to be surveyed

 Inflow Area =
 60.314 ac, Inflow Depth =
 0.40" for 1-yr event

 Inflow =
 8.82 cfs @
 12.52 hrs, Volume=
 2.005 af

 Outflow =
 8.75 cfs @
 12.56 hrs, Volume=
 2.005 af, Atten= 1%, Lag= 2.4 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 6.0 fps, Min. Travel Time= 3.0 min Avg. Velocity = 1.7 fps, Avg. Travel Time= 10.8 min Peak Depth= 0.46' @ 12.56 hrs Capacity at bank full= 205.50 cfs Inlet Invert= 570.00', Outlet Invert= 504.00' 10.00' x 2.00' deep Parabolic Channel, n= 0.027 Length= 1,090.0' Slope= 0.0606 '/'

### Reach r25b: Wetland Reach

Wetland Reach Has wetland vegetation within reach

Inflow Area = Inflow = Outflow =	9.435 ac, Inflow Depth = 0.36" 0.29 cfs @ 15.40 hrs, Volume= 0.28 cfs @ 15.62 hrs, Volume=	0.285 af	Lag= 13.3 min			
Max. Velocity= 0.6	tor-Ind method, Time Span= 0.00-48 6 fps, Min. Travel Time= 21.7 min 3 fps, Avg. Travel Time= 38.1 min	3.00 hrs, dt= 0.01 hrs				
Peak Depth= 0.16' @ 15.62 hrs Capacity at bank full= 156.51 cfs Inlet Invert= 504.00', Outlet Invert= 499.50' 20.00' x 3.00' deep Parabolic Channel, n= 0.045 Length= 750.0' Slope= 0.0060 '/'						

### Reach r25c: Amenia Stream/Cascade Brook

(Steam) parallel to Route 22 Base Flow estimated from field observation (see field note 21)

 Inflow Area =
 129.431 ac, Inflow Depth = 29.74"
 for
 1-yr event

 Inflow =
 92.48 cfs @
 13.10 hrs, Volume=
 320.727 af, Incl. 40.00 cfs Base Flow

 Outflow =
 92.31 cfs @
 13.19 hrs, Volume=
 320.045 af, Atten= 0%, Lag= 5.1 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 2.7 fps, Min. Travel Time= 6.0 min Avg. Velocity = 2.6 fps, Avg. Travel Time= 6.2 min

Peak Depth= 4.74' @ 13.19 hrs Capacity at bank full= 67.14 cfs Inlet Invert= 504.00', Outlet Invert= 500.00' 10.00' x 4.00' deep Parabolic Channel, n= 0.060 Length= 975.0' Slope= 0.0041 '/

# Pond 8P:

No field note. Water spills over cart path; no storage.

Inflow Area =	52.997 ac, Inflow Depth = 0.39"	for 1-yr event
Inflow =	8.65 cfs @ 12.52 hrs, Volume=	1.723 af
Outflow =	8.65 cfs @ 12.52 hrs, Volume=	1.723 af, Atten= 0%, Lag= 0.0 min
Primary =	8.65 cfs @ 12.52 hrs, Volume=	1.723 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 575.09' @ 12.52 hrs Flood Elev= 574.70' Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= (not calculated)

#	Routing	Invert	Outlet Devices
1	Primary	574.70'	177.0 deg Sharp-Crested Vee/Trap Weir C= 2.46

Primary OutFlow Max=8.65 cfs @ 12.52 hrs HW=575.09' TW=570.45' (Dynamic Tailwater) ←1=Sharp-Crested Vee/Trap Weir (Weir Controls 8.65 cfs @ 1.5 fps)

### Pond p04:

Field Note #13

Water ponding behind a golf cart path. Overflow dimensions are assumed based on aerial topo, and should be upgraded once survey is available.

Inflow Area =	38.062  ac,  Inflow Depth = 0.46"	for 1-yr event
Inflow =	7.08 cfs @ 12.53 hrs, Volume=	1.467 af
Outflow =	7.07 cfs @ 12.54 hrs, Volume=	1.401 af, Atten= 0%, Lag= 0.5 min
Primary =	7.07 cfs @ 12.54 hrs, Volume=	1.401 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 605.73' @ 12.54 hrs Surf.Area= 5,649 sf Storage= 3,389 cf Flood Elev= 605.50' Surf.Area= 4,803 sf Storage= 2,882 cf Plug-Flow detention time= 35.7 min calculated for 1.401 af (95% of inflow) Center-of-Mass det. time= 12.4 min (938.7 - 926.3)

#	Invert	Avail.St	orage Stor	age De	scription		
1	604.20'	26,8	397 cf <b>Cus</b>	stom Sta	age Data (Conic)	Listed below	
Elevati (fe	ion eet)	Surf.Area (sq-ft)	Inc. (cubic	Store -feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
604 606 608	.00	0 6,650 17,060		0 3,990 2,907	0 3,990 26,897	0 6,655 17,092	
#F	Routing	Invert	Outlet Devic	ces			
1 F	Primary	605.50'	179.0 deg S	Sharp-C	rested Vee/Trap	<b>Weir</b> C= 2.46	

Primary OutFlow Max=7.07 cfs @ 12.54 hrs HW=605.73' TW=575.08' (Dynamic Tailwater) ↓ 1=Sharp-Crested Vee/Trap Weir (Weir Controls 7.07 cfs @ 1.2 fps) HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems

### Pond p06:

Field Note #22 Geometry to be confirmed by survey.

Inflow Area =	9.435 ac,	Inflow Depth = 0.40"	for 1-yr event
Inflow =	1.40 cfs @	12.36 hrs, Volume=	0.311 af
Outflow =	0.29 cfs @	15.40 hrs, Volume=	0.285 af, Atten= 80%, Lag= 181.9 min
Primary =	0.29 cfs @	15.40 hrs, Volume=	0.285 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 506.80' Surf.Area= 18,600 sf Storage= 42,160 cf Peak Elev= 507.10' @ 15.40 hrs Surf.Area= 19,941 sf Storage= 48,459 cf (6,299 cf above start) Flood Elev= 507.10' Surf.Area= 19,958 sf Storage= 48,537 cf (6,377 cf above start) Plug-Flow detention time= (not calculated) Center-of-Mass det. time= (not calculated)

#	Invert	Avail.St	torage	Storage Des	Storage Description				
1	500.00'	67,	669 cf	Custom Sta	Custom Stage Data (Conic) Listed below				
	ation (feet)	Surf.Area (sq-ft)	()	Inc.Store cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)			
	0.00	0		0	0	0			
	)6.80 )8.00	18,600		42,160	42,160	18,672 24,138			
50	0.00	24,030		25,509	67,669	24,130			
#	Routing	Invert	Outlet	Devices					
1	1 Primary 506.80' 12.0" x 20.0' long Culvert CMP, projecting, no headwall, Ke= 0.900								
2	Outlet Invert= 506.00'         S= 0.0400 '/'         n= 0.024         Cc= 0.900           2         Primary         507.10' <b>178.0 deg Sharp-Crested Vee/Trap Weir</b> C= 2.46								

Primary OutFlow Max=0.29 cfs @ 15.40 hrs HW=507.10' TW=504.16' (Dynamic Tailwater) -1=Culvert (Inlet Controls 0.29 cfs @ 1.5 fps) -2=Sharp-Crested Vee/Trap Weir (Controls 0.00 cfs)

# Pond p07:

Field Note # 29 Outlet geometry to be confirmed by survey.

Inflow Area	=	7.317 ac, Inflow Depth = 0.51"	for 1-yr event
Inflow	=	1.70 cfs @ 12.25 hrs, Volume=	0.309 af
Outflow	=	0.29 cfs @ 14.88 hrs, Volume=	0.283 af, Atten= 83%, Lag= 157.5 min
Primary	=	0.29 cfs @ 14.88 hrs, Volume=	0.283 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 572.80' Surf.Area= 21,640 sf Storage= 56,264 cf Peak Elev= 573.06' @ 14.88 hrs Surf.Area= 22,888 sf Storage= 62,732 cf (6,468 cf above start) Flood Elev= 573.50' Surf.Area= 24,936 sf Storage= 73,351 cf (17,087 cf above start) Plug-Flow detention time= (not calculated) Center-of-Mass det. time= (not calculated)

# Existing Conditions\_10454-01

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#Inv	ert Avail.Sto	orage Storage Des	Storage Description			
1 565.0	00' 85,5	57 cf Custom Sta	age Data (Conic) L	isted below		
Elevation (feet) 565.00 572.80 574.00	Surf.Area (sq-ft) 0 21,640 27,290	Inc.Store (cubic-feet) 0 56,264 29,293	Cum.Store (cubic-feet) 0 56,264 85,557	Wet.Area (sq-ft) 0 21,735 27,424		
# Routing	Invert	Outlet Devices				
1 Primary	1 Primary 572.80' 18.0" x 20.0' long Culvert CMP, projecting, no headwall, Ke= 0.900					
Outlet Invert= 572.00'         S= 0.0400 '/'         n= 0.024         Cc= 0.900           2         Primary         573.50' <b>177.0 deg Sharp-Crested Vee/Trap Weir X 2.00</b> C= 2.46						

Primary OutFlow Max=0.29 cfs @ 14.88 hrs HW=573.06' TW=570.25' (Dynamic Tailwater) 1=Culvert (Inlet Controls 0.29 cfs @ 1.4 fps) 2=Sharp Crosted Vac/Trap Wair (Controls 0.00 cfc)

-2=Sharp-Crested Vee/Trap Weir (Controls 0.00 cfs)

# Pond p09:

Field Note #31 Geometry to be confirmed by survey.

Inflow Area =		8.452 ac, Inflow Depth = 0.38"	for 1-yr event
Inflow	=	1.74 cfs @ 12.33 hrs, Volume=	0.265 af
Outflow	=	0.73 cfs @ 12.83 hrs, Volume=	0.226 af, Atten= 58%, Lag= 30.0 min
Primary	=	0.73 cfs @ 12.83 hrs, Volume=	0.226 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 548.87' @ 12.83 hrs Surf.Area= 3,003 sf Storage= 2,794 cf Flood Elev= 551.20' Surf.Area= 8,534 sf Storage= 15,673 cf Plug-Flow detention time= 144.3 min calculated for 0.226 af (85% of inflow) Center-of-Mass det. time= 78.4 min (1,002.6 - 924.2)

#	Invert	Avail.St	torage Storage	Storage Description				
1	547.50'	21,	989 cf Custom	9 cf Custom Stage Data (Conic) Listed below				
Eleva (f	ition eet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)		Wet.Area (sq-ft)			
547	7.50	0	C	) 0	0			
548	3.00	1,080	180	180	1,080			
550	0.00	5,510	6,020	6,200	5,527			
552	2.00	10,550	15,790	21,989	10,606			
#	Routing	Invert	Outlet Devices					
1	Primary	548.50'	<b>30.0" x 70.0' long Culvert</b> CMP, projecting, no headwall, Ke= 0.900 Outlet Invert= 542.00' S= 0.0929 '/' n= 0.012 Cc= 0.900					
2	Primary	551.20' <b>168.0 deg Sharp-Crested Vee/Trap Weir</b> C= 2.46						

Primary OutFlow Max=0.73 cfs @ 12.83 hrs HW=548.87' TW=542.17' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 0.73 cfs @ 1.6 fps) -2=Sharp-Crested Vee/Trap Weir (Controls 0.00 cfs)

# Pond p10:

Field Note #25 Need to get full story on how this pond works

Inflow Are	a =	45.146 ac, Inf	flow Depth = 0.13"	for 1-yr event	
Inflow	=	2.72 cfs @ 1	12.00 hrs, Volume=	0.487 af	
Outflow	=	0.00 cfs @	0.00 hrs, Volume=	0.000 af,	Atten= 100%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 498.40' Surf.Area= 36,110 sf Storage= 101,108 cf Peak Elev= 498.94' @ 25.62 hrs Surf.Area= 38,236 sf Storage= 122,314 cf (21,206 cf above start) Plug-Flow detention time= (not calculated) Center-of-Mass det. time= (not calculated)

#	Invert	Avail.Stora	ge Storage Des	cription				
1	1 490.00' 581,029 cf		cf Custom Sta	Custom Stage Data (Conic) Listed below				
Elevatio		Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)			
490.0	00	0	0	0	0			
498.4	40	36,110	101,108	101,108	36,221			
500.0	00	42,400	62,741	163,849	42,610			
502.0	00	54,880	97,012	260,861	55,187			
504.0	00	78,730	132,895	393,755	79,107			
506.0	00	109,382	187,274	581,029	109,836			

Pond p12:

No field note. Natural depression.

Inflow Area =		6.420 ac, Ir	nflow Depth = 0.12"	for 1-yr event				
Inflow	=	0.12 cfs @	13.30 hrs, Volume=	0.065 af				
Outflow	=	0.00 cfs @	0.00 hrs, Volume=	0.000 af,	Atten= 100%, Lag= 0.0 min			
Primary	=	0.00 cfs @	0.00 hrs, Volume=	0.000 af				

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 544.90' @ 26.42 hrs Surf.Area= 2,999 sf Storage= 2,852 cf Flood Elev= 547.50' Surf.Area= 13,848 sf Storage= 21,762 cf Plug-Flow detention time= (not calculated) Center-of-Mass det. time= (not calculated)

# Existing Conditions\_10454-01

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#	Invert	Avail.S	torage	Storage De	escription			
1	543.50'	26.	986 cf		tage Data (Conic) L	isted below		
		- ,			<b>J</b>			
Elev	ation	Surf.Area		Inc.Store	Cum.Store	Wet.Area		
	(feet)	(sq-ft)	(0	cubic-feet)	(cubic-feet)	(sq-ft)		
54	13.50	0		0	0	0		
54	14.00	1,140		190	190	1,140		
54	16.00	5,260		5,899	6,089	5,278		
54	18.00	16,710		20,897	26,986	16,750		
#	Routing	Invert	Outlet	Devices				
1	Primary	547.50'	173.0 d	deg Sharp-O	Crested Vee/Trap W	<b>/eir</b> C= 2.46		
	-			•	-			
Prim	ary OutFlow	/ Max=0.0	0 cfs @	0.00 hrs H	W=543.50' TW=54	4.00' (Dynamic	Tailwater)	
⁻_1=	Sharp-Cres	sted Vee/Tr	ap Wei	r (Controls	0.00 cfs)			
Pond p13:								
	ield Note							
Natu	ral depressio	on.						
Infloy		0.250 a	o loflou	v Donth (	240" for $4x$ mayo	nt		
Inflov	v Area =			49 hrs Voli	0.004 0.002 0.004 0.004	-		

Type III 24-hr 1-yr Rainfall=2.70"

Inflow Area =	$0.350 \text{ ac}$ , inflow Depth = $0.12^{\circ}$	for 1-yr event
Inflow =	0.01 cfs @ 12.49 hrs, Volume=	0.004 af
Outflow =	0.00 cfs @ 0.00 hrs, Volume=	0.000 af, Atten= 100%, Lag= 0.0 min
Primary =	0.00 cfs @ 0.00 hrs, Volume=	0.000 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 512.09' @ 24.49 hrs Surf.Area= 435 sf Storage= 155 cf Flood Elev= 519.50' Surf.Area= 4,313 sf Storage= 16,523 cf Plug-Flow detention time= (not calculated) Center-of-Mass det. time= (not calculated)

#	Invert	Avail.St	orage	Storage Des	scription		
1	511.40'	18,4	490 cf	Custom Sta	ige Data (Conic) L	isted below	
Elevat (fe	tion eet)	Surf.Area (sq-ft)	(c	Inc.Store	Cum.Store (cubic-feet)	Wet.Area (sq-ft <u>)</u>	
511	.40	0		0	0	0	
512	2.00	390		78	78	391	
514	.00	1,360		1,652	1,730	1,381	
516	6.00	2,180		3,508	5,238	2,253	
518	8.00	3,240		5,385	10,623	3,375	
520	0.00	4,670		7,867	18,490	4,872	
	Routing			Devices			
1 F	1 Primary 519.50' 176.0 deg Sharp-Crested Vee/Trap Weir C= 2.46						

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=511.40' TW=497.40' (Dynamic Tailwater) ☐ 1=Sharp-Crested Vee/Trap Weir (Controls 0.00 cfs) HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems

# Pond p14:

Field Note #26 Need to figure out how this pond works

Inflow Area = Inflow = Outflow = Primary =	36.186 ac, Inflow Depth =       0.45" for 1-yr event         8.49 cfs @       12.03 hrs, Volume=       1.351 af         0.00 cfs @       0.00 hrs, Volume=       0.000 af, Atten= 100%, Lag= 0.0 min         0.00 cfs @       0.00 hrs, Volume=       0.000 af						
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 497.40' Surf.Area= 22,200 sf Storage= 54,760 cf Peak Elev= 499.17' @ 48.00 hrs Surf.Area= 41,352 sf Storage= 113,598 cf (58,838 cf above start)							

Plug-Flow detention time= (not calculated)

Center-of-Mass	det.	time=	(not	calculated)	

#	Invert	Avail.Sto	orage Storage I	Description		
1	490.00'	805,0	62 cf Custom	Stage Data (Conic	) Listed below	
<b>F</b> lave	4: a.a.	Curf Area	las Ctore	Curra Chara		
Eleva		Surf.Area	Inc.Store		Wet.Area	
(fe	eet)	(sq-ft)	(cubic-feet)	(cubic-feet)	(sq-ft)	
490	0.00	0	0	0	0	
497	7.40	22,200	54,760	54,760	22,286	
498	3.00	25,330	14,249	69,009	25,433	
500	0.00	52,810	76,476	145,485	52,948	
502	2.00	73,360	125,608	271,093	73,574	
504	4.00	84,070	157,308	428,402	84,467	
506	6.00	92,130	176,139	604,540	92,797	
508	3.00	108,618	200,522	805,062	109,437	
#	Routing	Invert	Outlet Devices			
1	Primary	500.00'	24.0" x 80.0' lon	g Culvert CPP, e	nd-section conform	ming to fill, Ke= 0.500
			Outlet Invert= 50	2.00' S= -0.0250 '	/' n= 0.012 Cc=	0.900

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=497.40' TW=498.40' (Dynamic Tailwater)

## Pond p15:

Field Note # 43 Infiltration basin

Inflow Area =	5.770 ac, Inflow Depth = 1.22"	for 1-yr event
Inflow =	6.61 cfs @ 12.07 hrs, Volume=	0.588 af
Outflow =	3.31 cfs @ 12.22 hrs, Volume=	0.332 af, Atten= 50%, Lag= 9.1 min
Primary =	3.31 cfs @ 12.22 hrs, Volume=	0.332 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 536.08' @ 12.22 hrs Surf.Area= 3,204 sf Storage= 11,413 cf Flood Elev= 536.00' Surf.Area= 3,160 sf Storage= 11,127 cf Plug-Flow detention time= 240.5 min calculated for 0.332 af (57% of inflow)

#	Invert	Avail.St	orage	Storage De	scription		
1	526.80'	18,5	577 cf	Custom St	age Data (Conic)	Listed below	
Eleva (fe	tion eet)	Surf.Area (sq-ft)	(0	Inc.Store	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
526	6.80	0		0	0	0	
528	3.00	310		124	124	312	
530	0.00	660		948	1,072	694	
532	2.00	1,180		1,815	2,887	1,256	
534	1.00	1,990		3,135	6,022	2,113	
536	6.00	3,160		5,105	11,127	3,337	
538	3.00	4,320		7,450	18,577	4,575	
#	Routing	Invert	Outlet	Devices			
1	Primary	536.00'	171.0 c	deg x 50.0' l	ong Sharp-Crest	ed Vee/Trap Wei	<b>r</b> C= 2.46

Center-of-Mass det. time= 115.6 min (906.2 - 790.6)

Primary OutFlow Max=3.31 cfs @ 12.22 hrs HW=536.08' TW=507.28' (Dynamic Tailwater) ←1=Sharp-Crested Vee/Trap Weir (Weir Controls 3.31 cfs @ 0.8 fps)

#### Pond p16:

Field Note # 49 Large pond with man-made island. Geometry to be verified by survey. In particular, we are making big guesses about the outlets. Also need to find out about valves...

Inflow Area =	220.861 ac, Inflow Depth = 0.51"	for 1-yr event
Inflow =	24.01 cfs @ 12.00 hrs, Volume=	9.313 af
Outflow =	0.13 cfs @ 31.55 hrs, Volume=	0.224 af, Atten= 99%, Lag= 1,172.8 min
Primary =	0.13 cfs @ 31.55 hrs, Volume=	0.224 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 507.00' Surf.Area= 199,799 sf Storage= 878,320 cf Peak Elev= 509.17' @ 31.55 hrs Surf.Area= 232,124 sf Storage= 1,279,535 cf (401,215 cf above start) Flood Elev= 510.50' Surf.Area= 271,550 sf Storage= 1,623,217 cf (744,897 cf above start) Plug-Flow detention time= (not calculated) Center-of-Mass det. time= (not calculated)

#	Invert	Avail.Storage	<ul> <li>Storage De</li> </ul>	scription	
1	500.00'	2,062,087 c	Custom Sta	age Data (Conic) Li	sted below
Elevat	ion	Surf.Area	Inc.Store	Cum.Store	Wet.Area
(fe	et)	(sq-ft)	(cubic-feet)	(cubic-feet)	(sq-ft)
500	.00	0	0	0	0
503	.00	140,344	140,344	140,344	140,358
509	.20	232,500	1,143,862	1,284,206	232,994
510	.00	249,400	192,720	1,476,927	249,951
512	.00	338,000	585,160	2,062,087	338,634

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#	Routing	Invert	Outlet Devices
1	Primary	509.00'	18.0" x 110.0' long Culvert CMP, projecting, no headwall, Ke= 0.900
			Outlet Invert= 505.70' S= 0.0300 '/' n= 0.024 Cc= 0.900
2	Primary	500.00'	8.0" x 100.0' long assumed equalization pipe w/ valve X 0.00
			CMP, projecting, no headwall, Ke= 0.900
			Outlet Invert= 500.00' S= 0.0000 '/' n= 0.013 Cc= 0.900
3	Primary	510.50'	175.0 deg Sharp-Crested Vee/Trap Weir X 2.00 C= 2.46

Primary OutFlow Max=0.13 cfs @ 31.55 hrs HW=509.17' TW=505.20' (Dynamic Tailwater) -1=Culvert (Inlet Controls 0.13 cfs @ 1.1 fps) -2=assumed equalization pipe w/ valve (Controls 0.00 cfs)

-3=Sharp-Crested Vee/Trap Weir (Controls 0.00 cfs)

### Pond p17:

Field Note #45 Golf pond Geometry to be confirmed by surveyed

Inflow Area =	115.991 ac, Inflow Depth = $0.41$ "	for 1-yr event	
Inflow =	10.01 cfs @ 14.05 hrs, Volume=	3.982 af	
Outflow =	9.87 cfs @ 14.21 hrs, Volume=	3.982 af, Atten= 1%, Lag= 9.3 min	
Primary =	9.87 cfs @ 14.21 hrs, Volume=	3.982 af	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 523.80' Surf.Area= 7,290 sf Storage= 9,234 cf Peak Elev= 524.87' @ 14.21 hrs Surf.Area= 9,541 sf Storage= 19,174 cf (9,940 cf above start) Flood Elev= 524.30' Surf.Area= 8,074 sf Storage= 13,623 cf (4,389 cf above start) Plug-Flow detention time= 67.7 min calculated for 3.769 af (95% of inflow) Center-of-Mass det. time= 25.6 min (1,042.7 - 1,017.0)

#	Invert	Avail.St	torage	Storage Des	scription		
1	520.00'	30,	224 cf	Custom Sta	age Data (Conic) L	isted below	
Elevati (fe	-	Surf.Area (sq-ft)	(0	Inc.Store cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
520.		0		0	0	0	
523.	.80	7,290		9,234	9,234	7,313	
524.	.00	7,300		1,459	10,693	7,374	
526.	.00	12,460		19,531	30,224	12,581	
_# R	Routing	Invert	Outlet	Devices			
1 P	Primary	523.80'			adth Broad-Creste		Veir
			Head (feet) 0.20 0.40 0.60 0.80 1.00 Coef. (English) 2.80 2.92 3.08 3.30 3.32				
2 P	Primary	524.30'	143.0 c	143.0 deg Sharp-Crested Vee/Trap Weir C= 2.47			
3 P	Primary	525.20'	178.0 o	deg x 60.0' k	ong Sharp-Crestee	d Vee/Trap Weir	C= 2.46

Primary OutFlow Max=9.86 cfs @ 14.21 hrs HW=524.87' TW=515.04' (Dynamic Tailwater) -1=Broad-Crested Rectangular Weir (Weir Controls 8.07 cfs @ 3.4 fps) -2=Sharp-Crested Vee/Trap Weir (Weir Controls 1.80 cfs @ 1.9 fps) -3=Sharp-Crested Vee/Trap Weir (Controls 0.00 cfs)

## Pond p18:

Field Note #46 Golf pond Geometry to be confirmed by surveyed

Inflow Area =	139.117 ac, Inflow Depth = 0.42"	for 1-yr event
Inflow =	11.08 cfs @ 14.17 hrs, Volume=	4.924 af
Outflow =	9.71 cfs @ 14.78 hrs, Volume=	4.920 af, Atten= 12%, Lag= 36.2 min
Primary =	9.71 cfs @ 14.78 hrs, Volume=	4.920 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 513.90' Surf.Area= 20,680 sf Storage= 26,884 cf Peak Elev= 515.15' @ 14.78 hrs Surf.Area= 25,043 sf Storage= 56,894 cf (30,010 cf above start) Flood Elev= 514.81' Surf.Area= 23,768 sf Storage= 48,709 cf (21,825 cf above start) Plug-Flow detention time= 177.2 min calculated for 4.303 af (87% of inflow) Center-of-Mass det. time= 74.2 min (1,089.2 - 1,015.0)

#	Invert	Avail.St	torage Storage D	Description		
1	510.00'	148,	288 cf Custom S	Stage Data (Conic)	Listed below	
	ation (feet)	Surf.Area (sq-ft)		Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
51	0.00	0	0	0	0	
51	3.90	20,680	26,884	26,884	20,704	
51	4.00	20,690	2,068	28,952	20,756	
51	6.00	28,290	48,782	77,735	28,436	
51	8.00	42,760	70,554	148,288	42,967	
#	Routing	Invert	Outlet Devices			
1	Primary	513.90'	0	readth Broad-Cres	0	Weir
			· · · ·	0.40 0.60 0.80 1		
			( <b>U</b>	.80 2.92 3.08 3.3		
2	Primary	514.81'	<b>U</b> 1	Crested Vee/Trap		
3	Primary	515.32'	175.0 deg x 10.0	' long Sharp-Crest	ted Vee/Trap Wei	C = 2.46

Primary OutFlow Max=9.71 cfs @ 14.78 hrs HW=515.15' TW=507.95' (Dynamic Tailwater) 1=Broad-Crested Rectangular Weir (Weir Controls 9.23 cfs @ 3.7 fps) 2=Sharp-Crested Vee/Trap Weir (Weir Controls 0.48 cfs @ 1.4 fps)

-3=Sharp-Crested Vee/Trap Weir (Controls 0.00 cfs)

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#### Pond p19:

Wetland Geometry to be confirmed by survey Based off aerial topo, and assumed topo contour

Inflow Area = Inflow = Outflow = Primary = Secondary =	1.03 cfs @ 0.45 cfs @	12.80 hrs, Volum 14.68 hrs, Volum 14.68 hrs, Volum	ne= 0.299 a ne= 0.299 a	f f, Atten= 56%, Lag= 112.7 min f				
Starting Elev= 972 Peak Elev= 972.0 Plug-Flow detenti Center-of-Mass d	Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 972.00' Surf.Area= 86,000 sf Storage= 57,333 cf Peak Elev= 972.03' @ 14.68 hrs Surf.Area= 86,763 sf Storage= 60,440 cf (3,107 cf above start) Plug-Flow detention time= (not calculated) Center-of-Mass det. time= (not calculated)							
# Invert	Avail.Stora	**						
1 970.00'	282,329	cf Custom Stag	<b>ge Data (Conic)</b> Lis	ted below				
Elevation (feet) 970.00	Surf.Area (sq-ft) 0	Inc.Store (cubic-feet) 0	Cum.Store (cubic-feet) 0	Wet.Area (sq-ft) 0				
972.00	86,000	57,333	57,333	86,006				
974.00	141,270	224,996	282,329	141,327				
<ul><li># Routing</li><li>1 Secondary</li><li>2 Primary</li></ul>	973.60' 178			Vee/Trap Weir C= 2.46 d Rectangular Weir				
,		, ,	40 0.60 0.80 1.00 2.92 3.08 3.30 3					

Primary OutFlow Max=0.45 cfs @ 14.68 hrs HW=972.03' TW=970.04' (Dynamic Tailwater) ←2=Broad-Crested Rectangular Weir (Weir Controls 0.45 cfs @ 0.5 fps)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=972.00' TW=973.60' (Dynamic Tailwater) —1=Sharp-Crested Vee/Trap Weir (Controls 0.00 cfs)

## Pond p20:

Field Note #50 Spring Fed Pond Geometry to be confirmed by surveyed

 Inflow Area =
 241.484 ac, Inflow Depth = 0.07" for 1-yr event

 Inflow =
 6.50 cfs @ 12.00 hrs, Volume=
 1.469 af

 Outflow =
 1.23 cfs @ 14.55 hrs, Volume=
 1.339 af, Atten= 81%, Lag= 153.2 min

 Primary =
 1.23 cfs @ 14.55 hrs, Volume=
 1.339 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

# Existing Conditions\_10454-01

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*Type III 24-hr 1-yr Rainfall=2.70"* Page 29 s 4/10/2006 2:33:09 PM

Starting Elev= 505.10' Surf.Area= 89,370 sf Storage= 138,524 cf Peak Elev= 505.39' @ 14.55 hrs Surf.Area= 89,373 sf Storage= 164,408 cf (25,884 cf above start) Plug-Flow detention time= (not calculated) Center-of-Mass det. time= (not calculated)

#	Invert	Avail.St	torage Storage Description
1	502.00'	615,	682 cf Custom Stage Data (Prismatic) Listed below
<b>F</b> Lau			
	ation	Surf.Area	
	(feet)	(sq-ft)	(cubic-feet) (cubic-feet)
50	02.00	0	0 0
50	05.10	89,370	138,524 138,524
50	06.00	89,380	80,437 218,961
50	08.00	99,280	188,660 407,621
5′	10.00	108,781	208,061 615,682
		,	
#	Routing	Invert	Outlet Devices
1	Primary	505.10'	3.0' long x 1.5' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50
			3.00
			Coef. (English) 2.62 2.64 2.64 2.68 2.75 2.86 2.92 3.07 3.07 3.03 3.28
			3.32
2	Primary	506.20'	6.5' long x 1.5' breadth Broad-Crested Rectangular Weir
-	i iiiiai y	000.20	Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50
			3.00
			Coef. (English) 2.62 2.64 2.64 2.68 2.75 2.86 2.92 3.07 3.07 3.03 3.28
			3.32 COEL (English) 2.02 2.04 2.04 2.06 2.75 2.00 2.92 3.07 3.07 3.03 3.26
2			
3	Primary	506.00'	176.0 deg x 97.0' long Sharp-Crested Vee/Trap Weir C= 2.46
<b>D</b> #::	am / O. 451	Max 1.0	a fa @ 11 FE has INN FOF 201 TIN FO1 201 (Duramia Taihustan)
Film	ary Outriow	iviax=1.2	3 cfs @ 14.55 hrs HW=505.39' TW=504.28' (Dynamic Tailwater)

Primary OutFlow Max=1.23 cfs @ 14.55 hrs HW=505.39' TW=504.28' (Dynamic Tailwater) -1=Broad-Crested Rectangular Weir (Weir Controls 1.23 cfs @ 1.4 fps) -2=Broad-Crested Rectangular Weir (Controls 0.00 cfs) -3=Sharp-Crested Vee/Trap Weir (Controls 0.00 cfs)

#### Pond p21:

Inflow Area	a =	489.305 ac, Inflow Depth = 0.31"	for 1-yr event
Inflow	=	46.21 cfs @ 12.62 hrs, Volume=	12.773 af
Outflow	=	9.05 cfs @ 16.70 hrs, Volume=	12.062 af, Atten= 80%, Lag= 244.8 min
Primary	=	9.05 cfs @ 16.70 hrs, Volume=	12.062 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 482.41' @ 16.70 hrs Surf.Area= 260,898 sf Storage= 246,169 cf Plug-Flow detention time= 413.7 min calculated for 12.059 af (94% of inflow) Center-of-Mass det. time= 360.9 min (1,314.4 - 953.5)

#	Invert	Avail.Storage	Storage Description
1	480.40'	5,244,885 cf	Custom Stage Data (Conic) Listed below

## Existing Conditions\_10454-01

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Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
480.40	0	0	0	0
482.00	202,230	107,856	107,856	202,234
484.00	485,198	667,114	774,970	485,231
486.00	1,275,481	1,698,237	2,473,208	1,275,541
488.00	1,499,208	2,771,678	5,244,885	1,499,423
# Routing	Invert Ou	utlet Devices		

1 Primary 480.40' **30.0" x 70.0' long Culvert** CMP, projecting, no headwall, Ke= 0.900 Outlet Invert= 480.40' S= 0.0000 '/' n= 0.024 Cc= 0.900

Primary OutFlow Max=9.05 cfs @ 16.70 hrs HW=482.41' TW=0.00' (Dynamic Tailwater) ←1=Culvert (Barrel Controls 9.05 cfs @ 2.9 fps)

### Pond p22:

Field Note #54 Golf Pond Geometry to be confirmed by survey

Inflow Area	l =	97.943 ac, Inflow Depth = 0.47" for 1-yr event	
Inflow	=	22.18 cfs @ 12.53 hrs, Volume= 3.865 af	
Outflow	=	20.54 cfs @ 12.65 hrs, Volume= 3.568 af, Atten= 7%, Lag= 7.	5 min
Primary	=	20.54 cfs @ 12.65 hrs, Volume= 3.568 af	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 498.10' Surf.Area= 6,520 sf Storage= 10,106 cf Peak Elev= 500.96' @ 12.65 hrs Surf.Area= 9,902 sf Storage= 33,862 cf (23,756 cf above start) Plug-Flow detention time= 142.0 min calculated for 3.336 af (86% of inflow) Center-of-Mass det. time= 58.5 min (992.6 - 934.2)

#	Invert	Avail.St	orage Storag	e Desc	cription	
1 4	495.00'	143,	770 cf Custo	m Stag	ge Data (Prisma	atic) Listed below
Elevation	-	Surf.Area	Inc.Sto		Cum.Store	
(feet)	)	(sq-ft)	(cubic-fe	et)	(cubic-feet)	
495.00	)	0		0	0	
498.10	)	6,520	10,1	06	10,106	
500.00	)	8,390	14,1	64	24,270	
502.00	)	11,530	19,9	20	44,190	
504.00	)	14,530	26,0	60	70,250	
506.00	)	18,340	32,8	70	103,120	
508.00	)	22,310	40,6	50	143,770	
#Rou	uting	Invert	Outlet Devices	6		
1 Prir	nary	499.75'	18.0" x 21.0' l	ong C	ulvert CMP, p	rojecting, no headwall, Ke= 0.900
	-		Outlet Invert=	499.75	S = 0.0000 '/'	n= 0.024 Cc= 0.900
2 Prir	mary	500.50'	1.0' long x 15	.0' brea	adth Broad-Cre	ested Rectangular Weir
	-		Head (feet) 0.	20 0.4	0 0.60 0.80 1	1.00 1.20 1.40 1.60
			Coef. (English	) 2.68	2.70 2.70 2.6	64 2.63 2.64 2.64 2.63

Existing Conditions_10454-01	Type III 24-hr 1-yr Rainfall=2.70"
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20.0' long x 13.5' breadth Broad-Crested Rectangular Weir 3 Primary 500.50' Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.62 2.66 2.70 2.66 2.65 2.66 2.65 2.63

Primary OutFlow Max=20.53 cfs @ 12.65 hrs HW=500.96' TW=499.64' (Dynamic Tailwater) -1=Culvert (Barrel Controls 2.85 cfs @ 2.5 fps) -2=Broad-Crested Rectangular Weir (Weir Controls 0.85 cfs @ 1.8 fps)

-3=Broad-Crested Rectangular Weir (Weir Controls 16.83 cfs @ 1.8 fps)

### Pond p23:

Inflow Area =	41.587 ac, Inflow Depth = 0.64"	for 1-yr event
Inflow =	12.15 cfs @ 12.74 hrs, Volume=	2.204 af
Outflow =	4.95 cfs @ 13.57 hrs, Volume=	1.285 af, Atten= 59%, Lag= 49.9 min
Primary =	4.95 cfs @ 13.57 hrs, Volume=	1.285 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 507.74' @ 13.57 hrs Surf.Area= 18,395 sf Storage= 40,658 cf Plug-Flow detention time= 241.0 min calculated for 1.285 af (58% of inflow) Center-of-Mass det. time= 110.8 min (1,030.2 - 919.4)

#	Invert	Avail.St	orage	Storage De	escription	
1	503.50'	100,3	303 cf	Custom St	age Data (Prisma	natic) Listed below
Eleva (f	ition eet)	Surf.Area (sq-ft)	(c	Inc.Store	Cum.Store (cubic-feet)	
503	3.50	0		0	0	
506	5.00	11,170		13,963	13,963	
508	3.00	19,460		30,630	44,593	
510	0.00	36,250		55,710	100,303	
#	Routing	Invert	Outlet [	Devices		
1	Primary	507.70'	178.0 d	leg x 178.0'	long Sharp-Cre	ested Vee/Trap Weir C= 2.46

Primary OutFlow Max=4.95 cfs @ 13.57 hrs HW=507.74' TW=506.87' (Dynamic Tailwater) **1=Sharp-Crested Vee/Trap Weir** (Weir Controls 4.95 cfs @ 0.6 fps)

## Pond zDP1: Design Point 1

Field note #10. Culvert dimensions to be confirmed by survey.

Inflow Area =	26.659 ac, Inflow Depth = 0.50"	for 1-yr event
Inflow =	6.01 cfs @ 12.58 hrs, Volume=	1.110 af
Outflow =	6.01 cfs @ 12.58 hrs, Volume=	1.110 af, Atten= 0%, Lag= 0.0 min
Primary =	6.01 cfs @ 12.58 hrs, Volume=	1.110 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 720.98' @ 12.58 hrs Surf.Area= 42 sf Storage= 26 cf Flood Elev= 727.00' Surf.Area= 1,105 sf Storage= 2,619 cf Plug-Flow detention time= 0.3 min calculated for 1.110 af (100% of inflow)

Invert

#

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Avail Storage Storage Description

$\pi$	invent	Avail.Old	Jiage Otolage De	Sonption		
1	720.10'	3,7	06 cf Custom St	tage Data (Conic)	Listed below	
Elevatio		Surf.Area	Inc.Store	Cum.Store	Wet.Area	
(fee	et)	(sq-ft)	(cubic-feet)	(cubic-feet)	(sq-ft)	
720.1	0	0	0	0	0	
722.0	0	90	57	57	96	
724.0	00	340	403	460	364	
726.0	0	760	1,072	1,533	815	
728.0	00	1,450	2,173	3,706	1,543	
#Rc	outing	Invert	Outlet Devices			
1 Pr	imary		<b>42.0" x 120.0' lon</b> Outlet Invert= 700.	-		
2 Pr	imary		155.0 deg Sharp-C			1.300

Center-of-Mass det. time= 0.2 min (925.6 - 925.4)

Primary OutFlow Max=6.01 cfs @ 12.58 hrs HW=720.98' TW=686.01' (Dynamic Tailwater) -1=Culvert (Inlet Controls 6.01 cfs @ 3.2 fps) -2=Sharp-Crested Vee/Trap Weir (Controls 0.00 cfs)

## Pond zDP2: Design Point 2

Field Note #15 Culvert dimensions to be confirmed by survey Overflow to ditch is currently discarded... We may have to model that area...

Inflow Area = Inflow = Outflow = Discarded = Primony =	97.712 ac, Inflow Depth = 0.38" 11.49 cfs @ 13.01 hrs, Volume= 11.49 cfs @ 13.02 hrs, Volume= 0.00 cfs @ 0.00 hrs, Volume= 11.40 cfs @ 13.02 hrs, Volume=	3.061 af 3.061 af, Atten= 0%, Lag= 0.3 min 0.000 af
Primary =	11.49 cfs @ 13.02 hrs, Volume=	3.061 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 621.19' @ 13.02 hrs Surf.Area= 159 sf Storage= 129 cf Flood Elev= 624.50' Surf.Area= 925 sf Storage= 1,728 cf Plug-Flow detention time= 0.3 min calculated for 3.061 af (100% of inflow) Center-of-Mass det. time= 0.2 min (966.0 - 965.8)

#	Invert	Avail.Storag	ge Storage Des	cription	
1	619.60'	7,280	cf Custom Sta	<b>ge Data (Conic)</b> Li	sted below
Elevati (fe	ion et)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
619.		0	0	0	0
620.		10	1	1	10
622.		260	214	215	269
624.	.00	760	976	1,192	793
626.	.00	1,420	2,146	3,338	1,492
628.	.00	2,580	3,943	7,280	2,694

	#	Routing	Invert	Outlet Devices
-	1	Primary	619.60'	24.0" x 150.0' long Culvert RCP, end-section conforming to fill, Ke= 0.500
				Outlet Invert= 608.00' S= 0.0773 '/' n= 0.012 Cc= 0.900
2	2	Discarded	624.50'	166.0 deg Sharp-Crested Vee/Trap Weir C= 2.46

Discarded OutFlow Max=0.00 cfs @ 0.00 hrs HW=619.60' (Free Discharge) ←2=Sharp-Crested Vee/Trap Weir (Controls 0.00 cfs)

**Primary OutFlow** Max=11.49 cfs @ 13.02 hrs HW=621.19' TW=607.40' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 11.49 cfs @ 4.3 fps)

### Pond zDP3: Design Point 3

Inflow Area	a =	212.742 ac, l	nflow Depth = 18.21	' for	1-yr event		
Inflow	=	98.90 cfs @	13.04 hrs, Volume	=	322.796 af		
Primary	=	98.90 cfs @	13.04 hrs, Volume	=	322.796 af,	Atten= 0%,	Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

### Pond zDP4: Design Point 4

Inflow Area	a =	489.305 ac, Inflow Depth = 0.30'	" for 1-yr event
Inflow	=	9.05 cfs @ 16.70 hrs, Volume	= 12.062 af
Primary	=	9.05 cfs @ 16.70 hrs, Volume	= 12.062 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

## Pond zDP5: Design Point 5

Inflow Area =	28.325 ac, Inflow Depth = 0.55"	for 1-yr event
Inflow =	8.52 cfs @ 12.52 hrs, Volume=	1.308 af
Primary =	8.52 cfs @ 12.52 hrs, Volume=	1.308 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

# Pre-Development Conditions 2 year 24 hour Storm Event Model Computations

Existing Conditions_10454-01TyPrepared by The Chazen CompaniesHydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems	ype III 24-hr 2-yr Rainfall=3.40" Page 34 4/10/2006 2:33:18 PM		
Subcatchment s01:			
Runoff = 4.77 cfs @ 12.66 hrs, Volume= 0.808 af, De	epth= 0.84"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= Type III 24-hr 2-yr Rainfall=3.40"	0.01 hrs		
Area (ac) CN Description			
11.485 68			
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)			
42.8 Direct Entry,			
Subcatchment s02:			
Runoff = 25.54 cfs @ 12.95 hrs, Volume= 5.701 af, De	epth= 0.70"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= Type III 24-hr 2-yr Rainfall=3.40"	0.01 hrs		
Area (ac) CN Description			
97.712 65			
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)			
61.3Direct Entry,			
Subcatchment s03:			
Runoff = 8.25 cfs @ 12.45 hrs, Volume= 1.131 af, De	epth= 0.89"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 2-yr Rainfall=3.40"			
Area (ac) CN Description			
15.174 69			
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)			
28.8 Direct Entry,			
Subcatchment s04:			
Runoff = 7.52 cfs @ 12.11 hrs, Volume= 0.665 af, De	epth= 0.70"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 2-yr Rainfall=3.40"			

Existing Conditions_10454-01	Type III 24-hr 2-yr Rainfall=3.40"		
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	1, 10,2000 2100.101 M		
Area (ac) CN Description			
11.403 65			
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)			
6.5 Direct Entry,			
Subcatchment s05:			
Runoff = $4.60 \text{ cfs} @ 12.32 \text{ hrs}$ , Volume= $0.658 \text{ af}$ ,	Depth= 0.53"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, Type III 24-hr 2-yr Rainfall=3.40"	dt= 0.01 hrs		
Area (ac) CN Description			
14.935 61			
Tc Length Slope Velocity Capacity Description			
(min) (feet) (ft/ft) (ft/sec) (cfs)			
17.3 Direct Entry,			
Subcatchment s06:			
Runoff = 3.12 cfs @ 12.31 hrs, Volume= 0.427 af,	Depth= 0.57"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, Type III 24-hr 2-yr Rainfall=3.40"	dt= 0.01 hrs		
Area (ac) CN Description			
9.007 62			
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)			
(min) (feet) (ft/ft) (ft/sec) (cfs) 17.3 Direct Entry,			
Subcatchment s06(OW): s06 Open Water			
Runoff = 1.75 cfs @ 12.00 hrs, Volume= 0.121 af,	Depth= 3.40"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, Type III 24-hr 2-yr Rainfall=3.40"	dt= 0.01 hrs		
Area (ac) CN Description			
0.428 100			

Subcatchment s07:			
Runoff = 3.18 cfs @ 12.23 hrs, Volume= 0.372 af, Depth= 0.66"			
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 2-yr Rainfall=3.40"			
Area (ac) CN Description 6.811 64			
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)			
13.9Direct Entry,			
Subcatchment s07(OW): s07 Open Water			
Runoff = 2.07 cfs @ 12.00 hrs, Volume= 0.143 af, Depth= 3.40"			
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 2-yr Rainfall=3.40"			
Area (ac) CN Description			
0.506 100			
Subcatchment s08:			
Runoff = 6.12 cfs @ 12.47 hrs, Volume= 1.095 af, Depth= 0.41"			
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 2-yr Rainfall=3.40"			
Area (ac) CN Description			
31.719 58			
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)			
21.8 Direct Entry,			
Subcatchment s09:			
Runoff = 4.10 cfs @ 12.27 hrs, Volume= 0.493 af, Depth= 0.70"			
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 2-yr Rainfall=3.40"			
Area (ac) CN Description 8.452 65			

Existing Conditions_10454-01 Prepared by The Chazen Companies	Type III 24-hr 2-yr Rainfall=3.40" Page 37		
HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer System	ns 4/10/2006 2:33:18 PM		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)			
16.4 Direct Entry,			
Subcatchment s10:			
Runoff = 3.83 cfs @ 12.46 hrs, Volume= 0.538 af,	Depth= 0.79"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, Type III 24-hr 2-yr Rainfall=3.40"	dt= 0.01 hrs		
Area (ac) CN Description			
8.130 67			
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)			
27.9 Direct Entry,			
Subcatchment s10(OW): s10 Open	Water		
Runoff = 3.39 cfs @ 12.00 hrs, Volume= 0.235 af,	Depth= 3.40"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, Type III 24-hr 2-yr Rainfall=3.40"	dt= 0.01 hrs		
Area (ac) CN Description			
0.830 100			
Subcatchment s11:			
Runoff = 1.48 cfs @ 12.32 hrs, Volume= 0.176 af,	Depth= 0.89"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 2-yr Rainfall=3.40"			
Area (ac) CN Description			
2.364 69			
Tc Length Slope Velocity Capacity Description			
(min) (feet) (ft/ft) (ft/sec) (cfs) 20.5 Direct Entry,			
Subcatchment s11(IC): s11 Imp. Cover			
Runoff = 8.67 cfs @ 12.04 hrs, Volume= 0.617 af,	Depth= 3.17"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, Type III 24-br 2-yr, Rainfall=3.40"	dt= 0.01 hrs		

Type III 24-hr 2-yr Rainfall=3.40"

Existing Conditions_10454-01	Type III 24-hr 2-yr Rainfall=3.40"
Prepared by The Chazen Companies HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer System	Page 38 ms 4/10/2006 2:33:19 PM
Area (ac) CN Description 2.338 98	
2.000 90	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
2.8 <b>Direct Entry</b> ,	
Subcatchment s12:	
Runoff = 0.59 cfs @ 12.83 hrs, Volume= 0.167 af,	, Depth= 0.31"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, Type III 24-hr 2-yr Rainfall=3.40"	dt= 0.01 hrs
Area (ac) CN Description	
6.420 55	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
42.0 Direct Entry,	
Subcatchment s13:	
Runoff = $0.05 \text{ cfs} @ 12.34 \text{ hrs}$ , Volume= $0.009 \text{ af}$ ,	, Depth= 0.31"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, Type III 24-hr 2-yr Rainfall=3.40"	dt= 0.01 hrs
Area (ac) CN Description	
0.350 55	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
8.6 <b>Direct Entry</b> ,	
Subcatchment s14:	
Runoff = 6.49 cfs @ 12.48 hrs, Volume= 0.986 af,	, Depth= 0.66"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, Type III 24-hr 2-yr Rainfall=3.40"	dt= 0.01 hrs
Area (ac) CN Description	
18.066 64	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
28.1 <b>Direct Entry</b> ,	

Existing Conditions_10454-01Type III 24-hr 2-yrRainfall=3.40"Prepared by The Chazen CompaniesPage 39HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems4/10/2006 2:33:19 PM
Subcatchment s14(IC): s14 Imp. Cover
Runoff = 8.98 cfs @ 12.03 hrs, Volume= 0.628 af, Depth= 3.17"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 2-yr Rainfall=3.40"
Area (ac) CN Description 2.380 98
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
2.3Direct Entry,
Subcatchment s14(OW): s14 Open Water
Runoff = 2.11 cfs @ 12.00 hrs, Volume= 0.147 af, Depth= 3.40"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 2-yr Rainfall=3.40"
Area (ac) CN Description
0.518 100
Subcatchment s15:
Runoff = 0.06 cfs @ 12.50 hrs, Volume= 0.018 af, Depth= 0.20"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 2-yr Rainfall=3.40"
Area (ac) CN Description
1.068 51
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
13.1 Direct Entry,
Subcatchment s16:
Runoff = 32.25 cfs @ 12.36 hrs, Volume= 4.230 af, Depth= 0.75"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 2-yr Rainfall=3.40"
Area (ac) CN Description
67.994 66

Existing Conditions_10454-01	Type III 24-hr 2-yr Rainfall=3.40"
Prepared by The Chazen Companies HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer System	Page 40 ns 4/10/2006 2:33:19 PM
	<u>1, 10,2000 2,00,10 1 m</u>
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
22.3 Direct Entry,	
Subcatchment s16(IC): s16 Imp.0	Cover
Runoff = 9.71 cfs @ 12.04 hrs, Volume= 0.694 af,	Depth= 3.17"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, Type III 24-hr 2-yr Rainfall=3.40"	dt= 0.01 hrs
Area (ac) CN Description	
2.629 98	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
2.9 Direct Entry,	
Subcatchment s16(OW): s16 Open	Water
Runoff = 21.84 cfs @ 12.00 hrs, Volume= 1.516 af,	Depth= 3.40"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, Type III 24-hr 2-yr Rainfall=3.40"	dt= 0.01 hrs
Area (ac) CN Description	
5.351 100	
Subcatchment s17:	
Runoff = 20.67 cfs @ 13.91 hrs, Volume= 7.207 af,	Depth= 0.75"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, Type III 24-hr 2-yr Rainfall=3.40"	dt= 0.01 hrs
Area (ac) CN Description	
115.827 66	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
125.2 Direct Entry,	
Subcatchment s17(OW): s17 Open	Water
Runoff = 0.67 cfs @ 12.00 hrs, Volume= 0.046 af,	Depth= 3.40"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, Type III 24-hr 2-yr Rainfall=3.40"	dt= 0.01 hrs

Existing Conditions_10454-01Type III 24-hr 2-yrRainfall=3.40Prepared by The Chazen CompaniesPage 4HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems4/10/2006 2:33:19 PI
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Area (ac) CN Description
0.164 100
Subcatchment s18:
Runoff = 11.29 cfs @ 12.39 hrs, Volume= 1.500 af, Depth= 0.79"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 2-yr Rainfall=3.40"
Area (ac) CN Description
22.654 67
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
24.4     Direct Entry,
Subcatchment s18(OW): s18 Open Water
Runoff = 1.93 cfs @ 12.00 hrs, Volume= 0.134 af, Depth= 3.40"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 2-yr Rainfall=3.40"
Area (ac) CN Description
0.472 100
Subcatchment s19:
Runoff = 3.05 cfs @ 12.70 hrs, Volume= 0.633 af, Depth= 0.49"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 2-yr Rainfall=3.40"
Area (ac) CN Description
15.520 60
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
40.4 Direct Entry,
Subcatchment s20:
Runoff = 9.42 cfs @ 12.54 hrs, Volume= 1.391 af, Depth= 0.89"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 2-yr Rainfall=3.40"

Type III 24-hr 2-yr Rainfall=3.40"

Existing Conditions_10454-01 Prepared by The Chazen Companies HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems	Type III 24-hr 2-yr Rainfall=3.40" Page 42 s 4/10/2006 2:33:19 PM
Area (ac) CN Description	
18.655 69	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
33.9 Direct Entry,	
Subcatchment s20(OW): s20 Open	Water
Runoff = 8.03 cfs @ 12.00 hrs, Volume= 0.558 af,	Depth= 3.40"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, d Type III 24-hr 2-yr Rainfall=3.40"	lt= 0.01 hrs
Area (ac) CN Description 1.968 100	
Subcatchment s21:	
Runoff = 50.01 cfs @ 12.43 hrs, Volume= 6.755 af,	Depth= 0.84"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, d Type III 24-hr 2-yr Rainfall=3.40"	lt= 0.01 hrs
Area (ac) CN Description 96.056 68	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
26.7 Direct Entry,	
Subcatchment s21(OW):	
Runoff = 49.94 cfs @ 12.00 hrs, Volume= 3.467 af,	Depth= 3.40"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, d Type III 24-hr 2-yr Rainfall=3.40"	lt= 0.01 hrs
Area (ac) CN Description 12.235 100	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
0.0 Direct Entry,	

Existing Conditions_10454-01Type III 24-hr 2-yrRainfall=3.40"Prepared by The Chazen CompaniesPage 43HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems4/10/2006 2:33:19 PM
Subcatchment s22:
Runoff = 43.09 cfs @ 12.49 hrs, Volume= 6.135 af, Depth= 0.89"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 2-yr Rainfall=3.40"
Area (ac) CN Description 82.287 69
TcLengthSlopeVelocityCapacityDescription(min)(feet)(ft/ft)(ft/sec)(cfs)
31.3 Direct Entry,
Subcatchment s22(OW): s22 Open Water
Runoff = 0.56 cfs @ 12.00 hrs, Volume= 0.039 af, Depth= 3.40"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 2-yr Rainfall=3.40"
Area (ac) CN Description
0.136 100
Subcatchment s23:
Runoff = 21.80 cfs @ 12.69 hrs, Volume= 3.660 af, Depth= 1.06"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 2-yr Rainfall=3.40"
Area (ac) CN Description
41.587 72
Tc         Length         Slope         Velocity         Capacity         Description           (min)         (feet)         (ft/ft)         (ft/sec)         (cfs)
47.0 Direct Entry,
Subcatchment s24:
Runoff = 16.05 cfs @ 12.48 hrs, Volume= 2.235 af, Depth= 0.95"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 2-yr Rainfall=3.40"
Area (ac) CN Description 28.325 70

Existing Conditions_10454-01 Prepared by The Chazen Companies HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer System	<i>Type III 24-hr 2-yr Rainfall=3.40"</i> Page 44 s 4/10/2006 2:33:19 PM
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
30.7 Direct Entry,	
Subcatchment s25:	
Runoff = 6.68 cfs @ 12.33 hrs, Volume= 0.844 af,	Depth= 0.75"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, c Type III 24-hr 2-yr Rainfall=3.40"	dt= 0.01 hrs
Area (ac) CN Description	
13.562 66	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
20.2 Direct Entry,	
Reach r03:	
Overland Flow Reach Requires more survey	
Inflow Area =11.485 ac, Inflow Depth = $0.84$ "for 2-yr eventInflow =4.77 cfs @12.66 hrs, Volume=0.808 afOutflow =4.74 cfs @12.71 hrs, Volume=0.808 af,	Atten= 1%, Lag= 2.9 min
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 Max. Velocity= 3.8 fps, Min. Travel Time= 3.5 min Avg. Velocity = 1.6 fps, Avg. Travel Time= 8.1 min	hrs
Peak Depth= 0.38' @ 12.71 hrs Capacity at bank full= 92.14 cfs Inlet Invert= 845.00', Outlet Invert= 728.00' 10.00' x 1.50' deep Parabolic Channel, n= 0.060 Length= 785.0' S	Slope= 0.1490 '/'
Reach r04:	
Channel	
Inflow Area = $26.659 \text{ ac}$ , Inflow Depth = $0.87"$ for 2-yr eventInflow = $12.00 \text{ cfs}$ @ $12.53 \text{ hrs}$ , Volume= $1.939 \text{ af}$ Outflow = $11.96 \text{ cfs}$ @ $12.56 \text{ hrs}$ , Volume= $1.939 \text{ af}$	Atten= 0%, Lag= 2.0 min
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 Max. Velocity= 5.0 fps, Min. Travel Time= 2.3 min Avg. Velocity = 2.1 fps, Avg. Travel Time= 5.3 min	hrs

Existing Conditions\_10454-01 Type III 24 Prepared by The Chazen Companies HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems

Peak Depth= 0.71' @ 12.56 hrs Capacity at bank full= 446.15 cfs Inlet Invert= 685.50', Outlet Invert= 608.00' 12.00' x 4.00' deep Parabolic Channel, n= 0.060 Length= 675.0' Slope= 0.1148 '/'

#### Reach r08a:

Man Made Ditch Inverts of pipe to be surveyed

Inflow Are	a =	97.712 ac, Inflow Depth = 0.70	" for 2-yr event
Inflow	=	25.49 cfs @ 13.00 hrs, Volume	= 5.701 af
Outflow	=	25.49 cfs @ 13.00 hrs, Volume	= 5.701 af, Atten= 0%, Lag= 0.3 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 8.6 fps, Min. Travel Time= 0.4 min Avg. Velocity = 4.2 fps, Avg. Travel Time= 0.9 min

Peak Depth= 0.58' @ 13.00 hrs Capacity at bank full= 81.88 cfs Inlet Invert= 607.00', Outlet Invert= 587.00' 10.00' x 1.00' deep Parabolic Channel, n= 0.027 Length= 226.0' Slope= 0.0885 '/'

#### Reach r08b:

24" HDPE Inverts to be surveyed

Inflow Are	a =	97.712 ac, I	nflow Depth	n = 0.70"	for 2-yr event		
Inflow	=	25.49 cfs @	13.00 hrs,	Volume=	5.701 af		
Outflow	=	25.49 cfs @	13.00 hrs,	Volume=	5.701 af,	Atten= 0%,	Lag= 0.2 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 22.0 fps, Min. Travel Time= 0.2 min Avg. Velocity = 11.2 fps, Avg. Travel Time= 0.4 min

Peak Depth= 0.79' @ 13.00 hrs Capacity at bank full= 77.17 cfs Inlet Invert= 587.00', Outlet Invert= 557.75' 24.0" Diameter Pipe n= 0.012 Length= 295.0' Slope= 0.0992 '/'

#### Reach r08c:

Ditch Pipe inverts to be surveyed

 Inflow Area =
 97.712 ac, Inflow Depth =
 0.70" for 2-yr event

 Inflow =
 25.49 cfs @
 13.00 hrs, Volume=
 5.701 af

 Outflow =
 25.48 cfs @
 13.02 hrs, Volume=
 5.701 af, Atten= 0%, Lag= 0.8 min

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Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 8.2 fps, Min. Travel Time= 1.2 min Avg. Velocity = 3.8 fps, Avg. Travel Time= 2.6 min

Peak Depth= 0.60' @ 13.02 hrs Capacity at bank full= 76.65 cfs Inlet Invert= 557.75', Outlet Invert= 512.00' 10.00' x 1.00' deep Parabolic Channel, n= 0.027 Length= 590.0' Slope= 0.0775 '/'

#### Reach r08d: Amenia Stream/Cascade Brook

(Steam) parallel to Route 22 Base Flow estimated from field observation (see field note 21)

97.712 ac, Inflow Depth = 20.19" for 2-yr event Inflow Area = Inflow 65.48 cfs @ 13.02 hrs, Volume= 164.412 af, Incl. 40.00 cfs Base Flow = 65.35 cfs @ 13.06 hrs, Volume= 164.168 af, Atten= 0%, Lag= 2.6 min Outflow = Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 3.5 fps, Min. Travel Time= 3.9 min Avg. Velocity = 3.1 fps, Avg. Travel Time= 4.4 min Peak Depth= 3.18' @ 13.06 hrs Capacity at bank full= 104.49 cfs Inlet Invert= 512.00', Outlet Invert= 504.00' 10.00' x 4.00' deep Parabolic Channel, n= 0.060 Length= 805.0' Slope= 0.0099 '/'

## Reach r14a:

Grass lined channel

Inflow Area =	=	8.452 ac, Inflow Depth = $0.64$	for 2-yr event	
Inflow =		2.90 cfs @ 12.52 hrs, Volume	= 0.454 af	
Outflow =		2.89 cfs @ 12.54 hrs, Volume	= 0.454 af,	Atten= 0%, Lag= 1.1 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 4.1 fps, Min. Travel Time= 1.5 min Avg. Velocity = 1.3 fps, Avg. Travel Time= 4.6 min

Peak Depth= 0.32' @ 12.54 hrs Capacity at bank full= 325.42 cfs Inlet Invert= 542.00', Outlet Invert= 526.00' 10.00' x 3.00' deep Parabolic Channel, n= 0.027 Length= 360.0' Slope= 0.0444 '/'

#### Reach r14b:

30" HDPE Under Main Entrance Road

 Inflow Area =
 8.452 ac, Inflow Depth =
 0.64"
 for 2-yr event

 Inflow =
 2.89 cfs @
 12.54 hrs, Volume=
 0.454 af

 Outflow =
 2.89 cfs @
 12.55 hrs, Volume=
 0.454 af, Atten= 0%, Lag= 0.6 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 8.7 fps, Min. Travel Time= 0.9 min Avg. Velocity = 2.7 fps, Avg. Travel Time= 2.7 min

Peak Depth= 0.30' @ 12.55 hrs Capacity at bank full= 94.91 cfs Inlet Invert= 526.00', Outlet Invert= 505.70' 30.0" Diameter Pipe n= 0.012 Length= 445.0' Slope= 0.0456 '/'

#### Reach r14c:

#### **Overland Flow**

Inflow Area = Inflow = Outflow =	6.420 ac, Inflow Depth = 0.00"       for 2-yr event         0.00 cfs @       0.00 hrs, Volume=       0.000 af         0.00 cfs @       0.00 hrs, Volume=       0.000 af, Atten= 0%, Lag= 0.0 min
Max. Velocity= 0.0	or-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs fps, Min. Travel Time= 0.0 min fps, Avg. Travel Time= 0.0 min

## Reach r15:

Brush Overbanks with Rocky Bottom Needs to be surveyed

 Inflow Area =
 4.702 ac, Inflow Depth = 2.02" for 2-yr event

 Inflow =
 8.73 cfs @ 12.06 hrs, Volume=
 0.793 af

 Outflow =
 8.69 cfs @ 12.07 hrs, Volume=
 0.793 af, Atten= 1%, Lag= 0.4 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 8.5 fps, Min. Travel Time= 0.6 min Avg. Velocity = 2.9 fps, Avg. Travel Time= 1.7 min

Peak Depth= 0.66' @ 12.07 hrs Capacity at bank full= 188.47 cfs Inlet Invert= 554.00', Outlet Invert= 528.00' 5.00' x 3.00' deep Parabolic Channel, n= 0.027 Length= 290.0' Slope= 0.0897 '/'

# Reach r16:

#### Pipe Reach

Inflow Area =       4.702 ac, Inflow Depth = 2.02" for 2-yr event         Inflow =       9.07 cfs @       12.04 hrs, Volume=       0.793 af         Outflow =       8.73 cfs @       12.06 hrs, Volume=       0.793 af, Atten= 4%, Lag= 1.1 min
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 9.3 fps, Min. Travel Time= 1.5 min Avg. Velocity = 3.1 fps, Avg. Travel Time= 4.7 min
Peak Depth= 0.61' @ 12.06 hrs Capacity at bank full= 66.05 cfs Inlet Invert= 573.00', Outlet Invert= 554.00' 30.0" Diameter Pipe n= 0.012 Length= 860.0' Slope= 0.0221 '/'
Reach r18a:
Overland Flow Reach
Inflow         =         0.00 cfs @         0.00 hrs, Volume=         0.000 af           Outflow         =         0.00 cfs @         0.00 hrs, Volume=         0.000 af, Atten= 0%, Lag= 0.0 min
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 0.0 fps, Min. Travel Time= 0.0 min Avg. Velocity = 0.0 fps, Avg. Travel Time= 0.0 min
Peak Depth= 0.00' @ 0.00 hrs Capacity at bank full= 379.63 cfs Inlet Invert= 973.60', Outlet Invert= 530.00' 50.00' x 1.00' deep Parabolic Channel, n= 0.060 Length= 1,220.0' Slope= 0.3636 '/'
Reach r18b:
Overland Flow Reach
Inflow         =         0.00 cfs @         0.00 hrs, Volume=         0.000 af           Outflow         =         0.00 cfs @         0.00 hrs, Volume=         0.000 af, Atten= 0%, Lag= 0.0 min
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 0.0 fps, Min. Travel Time= 0.0 min Avg. Velocity = 0.0 fps, Avg. Travel Time= 0.0 min
Peak Depth= 0.00' @ 0.00 hrs Capacity at bank full= 151.94 cfs Inlet Invert= 530.60', Outlet Invert= 514.00' $50.00' \times 1.00'$ deep Parabolic Channel, n= 0.060 Length= 285.0' Slope= 0.0582 '/'

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## Reach r21a:

Man Made Ditch

Inflow Area =       241.484 ac, Inflow Depth =       0.33" for 2-yr event         Inflow =       3.25 cfs @       24.98 hrs, Volume=       6.552 af         Outflow =       3.25 cfs @       25.03 hrs, Volume=       6.544 af, Atten= 0%, Lag= 2.7 min								
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 2.9 fps, Min. Travel Time= 3.7 min Avg. Velocity = 2.1 fps, Avg. Travel Time= 5.1 min								
Peak Depth= 0.44' @ 25.03 hrs Capacity at bank full= 191.76 cfs Inlet Invert= 504.00', Outlet Invert= 494.00' 10.00' x 3.00' deep Parabolic Channel, n= 0.027 Length= 648.0' Slope= 0.0154 '/'								
Reach r21b:								
Grass Ditch Geometry to be confirmed by survey (inverts at pipe)								
Inflow Area =       97.943 ac, Inflow Depth = 0.80" for 2-yr event         Inflow =       42.65 cfs @ 12.53 hrs, Volume=       6.506 af         Outflow =       42.64 cfs @ 12.54 hrs, Volume=       6.506 af, Atten= 0%, Lag= 0.4 min								
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 7.1 fps, Min. Travel Time= 0.5 min Avg. Velocity = 1.9 fps, Avg. Travel Time= 2.1 min								
Peak Depth= 0.89' @ 12.54 hrs Capacity at bank full= 239.90 cfs Inlet Invert= 499.00', Outlet Invert= 491.10' 15.00' x 2.00' deep Parabolic Channel, n= 0.027 Length= 230.0' Slope= 0.0343 '/'								

#### Reach r21c:

**Overland Flow Reach** 

Inflow Area = 41.587 ac, Inflow Depth =  $0.79^{"}$  for 2-yr event Inflow = 19.72 cfs @ 12.87 hrs, Volume= 2.742 afOutflow = 19.61 cfs @ 12.89 hrs, Volume= 2.742 af, Atten= 1%, Lag= 0.9 min Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrsMax. Velocity= 3.2 fps, Min. Travel Time= 0.8 minAvg. Velocity = 1.3 fps, Avg. Travel Time= 2.0 min Existing Conditions\_10454-01 Type III 24 Prepared by The Chazen Companies HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems

Peak Depth= 0.32' @ 12.89 hrs Capacity at bank full= 227.81 cfs Inlet Invert= 506.70', Outlet Invert= 485.75' 50.00' x 1.00' deep Parabolic Channel, n= 0.060 Length= 160.0' Slope= 0.1309 '/'

#### Reach r22a:

**Overland Flow Reach** 

Inflow Area = Inflow = Outflow =	15.520 ac, Inflow Depth =0.49"for2-yr event1.34 cfs @13.55 hrs, Volume=0.631 af1.34 cfs @13.65 hrs, Volume=0.631 af, Atten= 0%,	Lag= 6.2 min					
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 2.1 fps, Min. Travel Time= 7.6 min Avg. Velocity = 1.0 fps, Avg. Travel Time= 16.8 min							
Peak Depth= 0.0 Capacity at bank Inlet Invert= 970.							

50.00' x 1.00' deep Parabolic Channel, n= 0.060 Length= 970.0' Slope= 0.4227 '/'

#### Reach r22b:

Overland Flow Reach

Inflow Area = 15.520 ac, Inflow Depth = 0.49" for 2-yr event 1.34 cfs @ 13.65 hrs, Volume= Inflow = 0.631 af Outflow = 1.33 cfs @ 13.78 hrs, Volume= 0.631 af, Atten= 1%, Lag= 7.6 min Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 1.2 fps, Min. Travel Time= 8.8 min Avg. Velocity = 0.5 fps, Avg. Travel Time= 20.5 min Peak Depth= 0.11' @ 13.78 hrs Capacity at bank full= 175.17 cfs Inlet Invert= 560.00', Outlet Invert= 512.00'

50.00' x 1.00' deep Parabolic Channel, n= 0.060 Length= 620.0' Slope= 0.0774 '/'

#### Reach r25a:

Ditch Pipe inverts need to be surveyed

 Inflow Area =
 60.314 ac, Inflow Depth =
 0.73" for 2-yr event

 Inflow =
 19.01 cfs @
 12.47 hrs, Volume=
 3.683 af

 Outflow =
 18.93 cfs @
 12.49 hrs, Volume=
 3.682 af, Atten= 0%, Lag= 1.5 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 7.6 fps, Min. Travel Time= 2.4 min Avg. Velocity = 1.9 fps, Avg. Travel Time= 9.6 min Peak Depth= 0.65' @ 12.49 hrs Capacity at bank full= 205.50 cfs Inlet Invert= 570.00', Outlet Invert= 504.00' 10.00' x 2.00' deep Parabolic Channel, n= 0.027 Length= 1,090.0' Slope= 0.0606 '/'

## Reach r25b: Wetland Reach

Wetland Reach Has wetland vegetation within reach

Inflow Area = Inflow = Outflow =	9.435 ac, Inflow Depth =       0.66" for 2-yr event         1.10 cfs @       12.98 hrs, Volume=       0.522 af         1.04 cfs @       13.23 hrs, Volume=       0.521 af, Atten= 6%, Lag= 15.3 min	
Max. Velocity= 0.9	r-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs ps, Min. Travel Time= 14.6 min fps, Avg. Travel Time= 34.1 min	

#### Reach r25c: Amenia Stream/Cascade Brook

(Steam) parallel to Route 22 Base Flow estimated from field observation (see field note 21)

 Inflow Area =
 129.431 ac, Inflow Depth = 30.04"
 for 2-yr event

 Inflow =
 108.01 cfs @
 13.04 hrs, Volume=
 323.974 af, Incl. 40.00 cfs Base Flow

 Outflow =
 107.67 cfs @
 13.11 hrs, Volume=
 323.293 af, Atten= 0%, Lag= 4.6 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 2.8 fps, Min. Travel Time= 5.8 min Avg. Velocity = 2.6 fps, Avg. Travel Time= 6.2 min

Peak Depth= 5.20' @ 13.11 hrs Capacity at bank full= 67.14 cfs Inlet Invert= 504.00', Outlet Invert= 500.00' 10.00' x 4.00' deep Parabolic Channel, n= 0.060 Length= 975.0' Slope= 0.0041 '/

#### Pond 8P:

No field note. Water spills over cart path; no storage.

Inflow Area =	52.997 ac, Inflow Depth = $0.72$ "	for 2-yr event
Inflow =	18.58 cfs @ 12.46 hrs, Volume=	3.196 af
Outflow =	18.58 cfs @ 12.46 hrs, Volume=	3.196 af, Atten= 0%, Lag= 0.0 min
Primary =	18.58 cfs @ 12.46 hrs, Volume=	3.196 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 575.22' @ 12.46 hrs Flood Elev= 574.70' Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= (not calculated)

#	Routing	Invert	Outlet Devices
1	Primary	574.70'	177.0 deg Sharp-Crested Vee/Trap Weir C= 2.46

Primary OutFlow Max=18.58 cfs @ 12.46 hrs HW=575.22' TW=570.65' (Dynamic Tailwater) ←1=Sharp-Crested Vee/Trap Weir (Weir Controls 18.58 cfs @ 1.8 fps)

#### Pond p04:

Field Note #13

Water ponding behind a golf cart path. Overflow dimensions are assumed based on aerial topo, and should be upgraded once survey is available.

Inflow Area =	38.062 ac, Inflow Depth = 0.82"	for 2-yr event
Inflow =	14.49 cfs @ 12.48 hrs, Volume=	2.604 af
Outflow =	14.48 cfs @ 12.49 hrs, Volume=	2.538 af, Atten= 0%, Lag= 0.3 min
Primary =	14.48 cfs @ 12.49 hrs, Volume=	2.538 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 605.81' @ 12.49 hrs Surf.Area= 5,930 sf Storage= 3,558 cf Flood Elev= 605.50' Surf.Area= 4,803 sf Storage= 2,882 cf Plug-Flow detention time= 21.0 min calculated for 2.538 af (97% of inflow) Center-of-Mass det. time= 6.9 min (910.9 - 904.0)

#	Invert	Avail.Sto	orage Storage De	escription		
1	604.20'	26,8	397 cf Custom S	tage Data (Conic)	_isted below	
Elevati (fe		Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
604. 606. 608.	00	0 6,650 17,060	0 3,990 22,907	0 3,990 26,897	0 6,655 17,092	
#R	louting	Invert	Outlet Devices			
1 P	rimary	605.50'	179.0 deg Sharp-0	Crested Vee/Trap V	<b>Veir</b> C= 2.46	

Primary OutFlow Max=14.48 cfs @ 12.49 hrs HW=605.81' TW=575.22' (Dynamic Tailwater)

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### Pond p06:

Field Note #22 Geometry to be confirmed by survey.

Inflow Area =	9.435 ac, Inflow Depth = 0.70"	for 2-yr event
Inflow =	3.56 cfs @ 12.29 hrs, Volume=	0.549 af
Outflow =	1.10 cfs @ 12.98 hrs, Volume=	0.522 af, Atten= 69%, Lag= 41.2 min
Primary =	1.10 cfs @ 12.98 hrs, Volume=	0.522 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 506.80' Surf.Area= 18,600 sf Storage= 42,160 cf Peak Elev= 507.21' @ 12.98 hrs Surf.Area= 20,460 sf Storage= 50,896 cf (8,736 cf above start) Flood Elev= 507.10' Surf.Area= 19,958 sf Storage= 48,537 cf (6,377 cf above start) Plug-Flow detention time= (not calculated) Center-of-Mass det. time= (not calculated)

#	Invert	Avail.St	orage	Storage Des	scription		
1	500.00'	67,	669 cf	Custom Sta	i <b>ge Data (Conic)</b> Li	isted below	
	ation (feet)	Surf.Area (sq-ft)		Inc.Store ubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
50 50	)0.00 )6.80 )8.00	0 18,600	(0	0 42,160	0 42,160	0 18,672	
#	Routing	24,030 Invert	Outlet E	25,509 Devices	67,669	24,138	
1	Primary	506.80'		•	Culvert CMP, proj	•	-
2	Primary	507.10'	Outlet Invert= 506.00' S= 0.0400 '/' n= 0.024 Cc= 0.900 178.0 deg Sharp-Crested Vee/Trap Weir C= 2.46				

Primary OutFlow Max=1.10 cfs @ 12.98 hrs HW=507.21' TW=504.28' (Dynamic Tailwater) -1=Culvert (Inlet Controls 0.52 cfs @ 1.7 fps)

-2=Sharp-Crested Vee/Trap Weir (Weir Controls 0.58 cfs @ 0.8 fps)

## Pond p07:

Field Note # 29 Outlet geometry to be confirmed by survey.

Inflow Area	a =	7.317 ac, Inflow Depth = 0.84"	for 2-yr event
Inflow	=	3.81 cfs @ 12.22 hrs, Volume=	0.515 af
Outflow	=	0.65 cfs @ 13.67 hrs, Volume=	0.487 af, Atten= 83%, Lag= 87.1 min
Primary	=	0.65 cfs @ 13.67 hrs, Volume=	0.487 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 572.80' Surf.Area= 21,640 sf Storage= 56,264 cf Peak Elev= 573.20' @ 13.67 hrs Surf.Area= 23,537 sf Storage= 66,097 cf (9,833 cf above start) Flood Elev= 573.50' Surf.Area= 24,936 sf Storage= 73,351 cf (17,087 cf above start) Plug-Flow detention time= (not calculated) Center-of-Mass det. time= (not calculated)

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#	Invert	Avail.St	orage	Storage Des	scription		
1	565.00'	85,	557 cf	Custom Sta	ige Data (Conic)	Listed below	
Elevati (fe	ion et)	Surf.Area (sq-ft)	(c	Inc.Store ubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
565.	.00	0		0	0	0	
572.	.80	21,640		56,264	56,264	21,735	
574.	.00	27,290		29,293	85,557	27,424	
	Routing	Invert	Outlet [	Devices			
1 P	Primary	572.80'	18.0" x	( 20.0' long (	Culvert CMP, pr	ojecting, no hea	dwall, Ke= 0.900
2 P	Primary	573.50'			0' S= 0.0400 '/' rested Vee/Trap '		

Primary OutFlow Max=0.65 cfs @ 13.67 hrs HW=573.20' TW=570.39' (Dynamic Tailwater) -1=Culvert (Inlet Controls 0.65 cfs @ 1.7 fps)

-2=Sharp-Crested Vee/Trap Weir (Controls 0.00 cfs)

## Pond p09:

Field Note #31 Geometry to be confirmed by survey.

Inflow Area =	8.452 ac, 1	nflow Depth = 0.70"	for 2-yr event	
Inflow =	4.10 cfs @	12.27 hrs, Volume=	0.493 af	
Outflow =	2.90 cfs @	12.52 hrs, Volume=	0.454 af,	Atten= 29%, Lag= 15.2 min
Primary =	2.90 cfs @	12.52 hrs, Volume=	0.454 af	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 549.25' @ 12.52 hrs Surf.Area= 3,855 sf Storage= 3,951 cf Flood Elev= 551.20' Surf.Area= 8,534 sf Storage= 15,673 cf Plug-Flow detention time= 85.0 min calculated for 0.454 af (92% of inflow) Center-of-Mass det. time= 46.0 min (945.2 - 899.2)

#	Invert	Avail.St	torage Stor	rage Des	scription		
1	547.50'	21,	989 cf Cus	39 cf Custom Stage Data (Conic) Listed below			
Eleva (f	ation eet)	Surf.Area (sq-ft)	Inc. (cubic	Store -feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
54	7.50	0		0	0	0	
548	8.00	1,080		180	180	1,080	
550	0.00	5,510	(	6,020	6,200	5,527	
552	2.00	10,550	15	5,790	21,989	10,606	
#	Routing	Invert	Outlet Devid	ces			
1	Primary	548.50'	<b>30.0" x 70.0' long Culvert</b> CMP, projecting, no headwall, Ke= 0.900 Outlet Invert= 542.00' S= 0.0929 '/' n= 0.012 Cc= 0.900				
2	Primary	551.20'		8.0 deg Sharp-Crested Vee/Trap Weir C= 2.46			

Primary OutFlow Max=2.90 cfs @ 12.52 hrs HW=549.25' TW=542.32' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 2.90 cfs @ 2.3 fps) -2=Sharp-Crested Vee/Trap Weir (Controls 0.00 cfs)

## Pond p10:

Field Note #25 Need to get full story on how this pond works

Inflow Are	a =	45.146 ac, I	nflow Depth = 0.21"	for 2-yr event	
Inflow	=	4.34 cfs @	12.41 hrs, Volume=	0.773 af	
Outflow	=	0.00 cfs @	0.00 hrs, Volume=	0.000 af,	Atten= 100%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 498.40' Surf.Area= 36,110 sf Storage= 101,108 cf Peak Elev= 499.26' @ 25.62 hrs Surf.Area= 39,488 sf Storage= 134,799 cf (33,691 cf above start) Plug-Flow detention time= (not calculated)

Center-of-Mass det. time= (not calculated)

#	Invert	Avail.Stora	ige Storage Des	cription			
1	490.00'	581,029	ocf Custom Sta	Custom Stage Data (Conic) Listed below			
Elevatio		Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)		
490.0	00	0	0	0	0		
498.4	40	36,110	101,108	101,108	36,221		
500.0	00	42,400	62,741	163,849	42,610		
502.0	00	54,880	97,012	260,861	55,187		
504.0	00	78,730	132,895	393,755	79,107		
506.0	00	109,382	187,274	581,029	109,836		

## Pond p12:

No field note. Natural depression.

Inflow Area =		6.420 ac, Inflow Depth = 0.31"	for 2-yr event
Inflow	=	0.59 cfs @ 12.83 hrs, Volume=	0.167 af
Outflow	=	0.00 cfs @ 0.00 hrs, Volume=	0.000 af, Atten= 100%, Lag= 0.0 min
Primary	=	0.00 cfs @ 0.00 hrs, Volume=	0.000 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 546.11' @ 26.42 hrs Surf.Area= 5,917 sf Storage= 7,288 cf Flood Elev= 547.50' Surf.Area= 13,848 sf Storage= 21,762 cf Plug-Flow detention time= (not calculated) Center-of-Mass det. time= (not calculated)

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HydroCAD® 7.0	0 s/n 000927 © 198	6-2003 Applied M	icrocomputer Syste	ems	4/10/2006 2:33:20 PM	
# Inve	rt Avail.Storage	Storage Descr	iption			
1 543.5	0' 26,986 cf	Custom Stage	e Data (Conic) Lis	sted below		
		-				
Elevation	Surf.Area	Inc.Store	Cum.Store	Wet.Area		
(feet)	(sq-ft) (	(cubic-feet)	(cubic-feet)	(sq-ft)		
543.50	0	0	0	0		
544.00	1,140	190	190	1,140		
546.00	5,260	5,899	6,089	5,278		
548.00	16,710	20,897	26,986	16,750		
			,			
# Routing	Invert Outlet	Devices				
1 Primary	547.50' <b>173.0</b>	deg Sharp-Cres	ted Vee/Trap We	eir C= 2.46		
Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=543.50' TW=544.00' (Dynamic Tailwater) 1=Sharp-Crested Vee/Trap Weir (Controls 0.00 cfs)						
		Por	nd p13:			
No Field Note Natural depression.						
Inflow Area = Inflow = Outflow = Primary =	0.05 cfs @ 12 0.00 cfs @ 0	w Depth = 0.31 .34 hrs, Volume .00 hrs, Volume .00 hrs, Volume	= 0.000 a	af af, Atten= 100°	%, Lag= 0.0 min	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 512.39' @ 24.49 hrs Surf.Area= 577 sf Storage= 397 cf Flood Elev= 519.50' Surf.Area= 4,313 sf Storage= 16,523 cf Plug-Flow detention time= (not calculated) Center-of-Mass det. time= (not calculated)

#	Invert	Avail.Sto	orage Storage I	Description		
1	511.40'	18,4	90 cf Custom	Stage Data (Conic)	Listed below	
Eleva (f	ation feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	•••••••	Wet.Area (sq-ft)	
51	1.40	0	0	0	0	
512	2.00	390	78	78	391	
514	4.00	1,360	1,652	1,730	1,381	
510	6.00	2,180	3,508	5,238	2,253	
518	8.00	3,240	5,385	10,623	3,375	
520	0.00	4,670	7,867	18,490	4,872	
#	Routing	Invert	Outlet Devices			
1	Primary	519.50'	176.0 deg Sharp	-Crested Vee/Trap	<b>Weir</b> C= 2.46	

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=511.40' TW=497.40' (Dynamic Tailwater) ☐ 1=Sharp-Crested Vee/Trap Weir (Controls 0.00 cfs)

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# Pond p14:

Field Note #26 Need to figure out how this pond works

Inflow Area = Inflow = Outflow = Primary =	36.186 ac, Inflow Depth = 0.73"       for 2-yr event         11.16 cfs @ 12.03 hrs, Volume=       2.215 af         0.00 cfs @ 0.00 hrs, Volume=       0.000 af, Atten= 100%, Lag= 0.0 min         0.00 cfs @ 0.00 hrs, Volume=       0.000 af						
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 497.40' Surf.Area= 22,200 sf Storage= 54,760 cf Peak Elev= 500.09' @ 48.00 hrs Surf.Area= 53,754 sf Storage= 151,255 cf (96,495 cf above start) Plug-Flow detention time= (not calculated)							

Center-of-Mass det. time= (not calculated)

#	Invert	Avail.St	torage Storage De	scription		
1	490.00'	805,	062 cf Custom Sta	age Data (Conic) ∟	isted below	
Eleva (1	ation feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
49	0.00	0	0	0	0	
49	7.40	22,200	54,760	54,760	22,286	
49	8.00	25,330	14,249	69,009	25,433	
50	0.00	52,810	76,476	145,485	52,948	
50	2.00	73,360	125,608	271,093	73,574	
50	4.00	84,070	157,308	428,402	84,467	
50	6.00	92,130	176,139	604,540	92,797	
50	8.00	108,618	200,522	805,062	109,437	
#	Routing	Invert	Outlet Devices			
1	Primary	500.00'	<b>24.0" x 80.0' long</b> Outlet Invert= 502.0			ing to fill, Ke= 0.500 0.900

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=497.40' TW=498.40' (Dynamic Tailwater)

## Pond p15:

Field Note # 43 Infiltration basin

Inflow Area =	5.770 ac, Inflow Depth = $1.69$ "	for 2-yr event
Inflow =	8.69 cfs @ 12.07 hrs, Volume=	0.811 af
Outflow =	8.49 cfs @ 12.08 hrs, Volume=	0.555 af, Atten= 2%, Lag= 1.0 min
Primary =	8.49 cfs @ 12.08 hrs, Volume=	0.555 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 536.14' @ 12.08 hrs Surf.Area= 3,243 sf Storage= 11,657 cf Flood Elev= 536.00' Surf.Area= 3,160 sf Storage= 11,127 cf Plug-Flow detention time= 191.8 min calculated for 0.555 af (68% of inflow)

#	Invert	Avail.Sto	orage Storage I	Description		
1	526.80'	18,5	577 cf Custom	Stage Data (Conic)	Listed below	
Eleva (f	ation eet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
526	6.80	0	0	0	0	
528	8.00	310	124	124	312	
530	0.00	660	948	1,072	694	
532	2.00	1,180	1,815	2,887	1,256	
534	4.00	1,990	3,135	6,022	2,113	
536	6.00	3,160	5,105	11,127	3,337	
538	8.00	4,320	7,450	18,577	4,575	
	Routing		Outlet Devices			
1	Primary	536.00'	171.0 deg x 50.0	)' long Sharp-Cres	ted Vee/Trap Wei	<b>r</b> C= 2.46

Center-of-Mass det. time= 84.9 min (875.9 - 791.0)

Primary OutFlow Max=8.46 cfs @ 12.08 hrs HW=536.14' TW=507.32' (Dynamic Tailwater) ←1=Sharp-Crested Vee/Trap Weir (Weir Controls 8.46 cfs @ 1.2 fps)

### Pond p16:

Field Note # 49 Large pond with man-made island. Geometry to be verified by survey. In particular, we are making big guesses about the outlets. Also need to find out about valves...

Inflow Area =	220.861 ac, Inflow Depth = 0.86"	for 2-yr event
Inflow =	45.38 cfs @ 12.34 hrs, Volume=	15.878 af
Outflow =	3.26 cfs @ 24.36 hrs, Volume=	5.140 af, Atten= 93%, Lag= 721.2 min
Primary =	3.26 cfs @ 24.36 hrs, Volume=	5.140 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 507.00' Surf.Area= 199,799 sf Storage= 878,320 cf Peak Elev= 509.98' @ 24.36 hrs Surf.Area= 248,980 sf Storage= 1,472,141 cf (593,821 cf above start) Flood Elev= 510.50' Surf.Area= 271,550 sf Storage= 1,623,217 cf (744,897 cf above start) Plug-Flow detention time= (not calculated) Center-of-Mass det. time= (not calculated)

#	Invert	Avail.Storage	<ul> <li>Storage De</li> </ul>	scription			
1	500.00'	2,062,087 c	f Custom Sta	Custom Stage Data (Conic) Listed below			
Elevat (fe	ion et)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)		
500.	.00	0	0	0	0		
503.	.00	140,344	140,344	140,344	140,358		
509.	.20	232,500	1,143,862	1,284,206	232,994		
510	.00	249,400	192,720	1,476,927	249,951		
512	.00	338,000	585,160	2,062,087	338,634		

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#	Routing	Invert	Outlet Devices
1	Primary	509.00'	<b>18.0" x 110.0' long Culvert</b> CMP, projecting, no headwall, Ke= 0.900
	-		Outlet Invert= 505.70' S= 0.0300 '/' n= 0.024 Cc= 0.900
2	Primary	500.00'	8.0" x 100.0' long assumed equalization pipe w/ valve X 0.00
			CMP, projecting, no headwall, Ke= 0.900
			Outlet Invert= 500.00' S= 0.0000 '/' n= 0.013 Cc= 0.900
3	Primary	510.50'	175.0 deg Sharp-Crested Vee/Trap Weir X 2.00 C= 2.46

Primary OutFlow Max=3.26 cfs @ 24.36 hrs HW=509.98' TW=505.65' (Dynamic Tailwater) -1=Culvert (Inlet Controls 3.26 cfs @ 2.7 fps) -2=assumed equalization pipe w/ valve (Controls 0.00 cfs)

-3=Sharp-Crested Vee/Trap Weir (Controls 0.00 cfs)

# Pond p17:

Field Note #45 Golf pond Geometry to be confirmed by surveyed

Inflow Area =	115.991 ac, Inflow Depth = 0.75"	for 2-yr event
Inflow =	20.70 cfs @ 13.91 hrs, Volume=	7.253 af
Outflow =	20.68 cfs @ 13.93 hrs, Volume=	7.253 af, Atten= 0%, Lag= 1.1 min
Primary =	20.68 cfs @ 13.93 hrs, Volume=	7.253 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 523.80' Surf.Area= 7,290 sf Storage= 9,234 cf Peak Elev= 525.24' @ 13.93 hrs Surf.Area= 10,505 sf Storage= 22,826 cf (13,592 cf above start) Flood Elev= 524.30' Surf.Area= 8,074 sf Storage= 13,623 cf (4,389 cf above start) Plug-Flow detention time= 43.0 min calculated for 7.039 af (97% of inflow) Center-of-Mass det. time= 19.3 min (1,014.0 - 994.7)

#	Invert	Invert Avail.Storage		Storage Des	scription		
1	520.00'	30,	224 cf	Custom Stage Data (Conic) Listed below			
-	ation feet)	Surf.Area (sq-ft)		Inc.Store cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
-	20.00	0		0	0	0	
52	23.80	7,290		9,234	9,234	7,313	
52	24.00	7,300		1,459	10,693	7,374	
52	526.00 12,460			19,531	30,224	12,581	
#	Routing	Invert	Outlet I	Devices			
1	Primary	523.80'	Head (f	feet) 0.20 0.	adth Broad-Creste .40 0.60 0.80 1.00 0 2.92 3.08 3.30	0	Neir
2 3	Primary Primary	524.30' 525.20'	143.0 d	<b>3.0 deg Sharp-Crested Vee/Trap Weir</b> C= 2.47 <b>8.0 deg x 60.0' long Sharp-Crested Vee/Trap Weir</b> C= 2.46		C= 2.46	

Primary OutFlow Max=20.68 cfs @ 13.93 hrs HW=525.24' TW=515.55' (Dynamic Tailwater) 1=Broad-Crested Rectangular Weir (Weir Controls 12.65 cfs @ 4.0 fps) 2=Sharp-Crested Vee/Trap Weir (Weir Controls 6.36 cfs @ 2.4 fps) -3=Sharp-Crested Vee/Trap Weir (Weir Controls 1.66 cfs @ 0.6 fps)

## Pond p18:

Field Note #46 Golf pond Geometry to be confirmed by surveyed

Inflow Area =	139.117 ac, Inflow Depth	h = 0.77" for 2-yr event	
Inflow =	22.84 cfs @ 13.91 hrs,	Volume= 8.886 af	
Outflow =	22.53 cfs @ 14.08 hrs,	Volume= 8.882 af,	Atten= 1%, Lag= 10.2 min
Primary =	22.53 cfs @ 14.08 hrs,	Volume= 8.882 af	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 513.90' Surf.Area= 20,680 sf Storage= 26,884 cf Peak Elev= 515.55' @ 14.08 hrs Surf.Area= 26,590 sf Storage= 66,821 cf (39,937 cf above start) Flood Elev= 514.81' Surf.Area= 23,768 sf Storage= 48,709 cf (21,825 cf above start) Plug-Flow detention time= 113.7 min calculated for 8.264 af (93% of inflow) Center-of-Mass det. time= 55.4 min (1,045.6 - 990.2)

#	Invert	Avail.St	torage Storage E	Description		
1	510.00'	148,	288 cf Custom	Stage Data (Conic)	Listed below	
(	ation (feet)	Surf.Area (sq-ft)	(cubic-feet)	(cubic-feet)	Wet.Area (sq-ft)	
	0.00	0	0	0	0	
	3.90	20,680	,	,	20,704	
51	4.00	20,690	2,068	28,952	20,756	
51	6.00	28,290	48,782	77,735	28,436	
51	8.00	42,760	70,554	148,288	42,967	
#	Routing	Invert	Outlet Devices			
1	Primary	513.90'	2.0' long x 0.5' b	readth Broad-Cres	sted Rectangular	Weir
	ý			0.40 0.60 0.80 1		
			Coef. (English) 2.80 2.92 3.08 3.30 3.32			
2	2 Primary 514.		143.0 deg Sharp-Crested Vee/Trap Weir C= 2.47			
3	Primary	515.32'		l' long Sharp-Crest		r C = 2.46
Ŭ	ary	0.0.0L				
D		. Mar. 00 /			1 500 401 (Dune	

Primary OutFlow Max=22.53 cfs @ 14.08 hrs HW=515.55' TW=508.42' (Dynamic Tailwater)

-1=Broad-Crested Rectangular Weir (Weir Controls 14.11 cfs @ 4.3 fps) -2=Sharp-Crested Vee/Trap Weir (Weir Controls 3.51 cfs @ 2.1 fps)

-3=Sharp-Crested Vee/Trap Weir (Weir Controls 3.97 cfs @ 1.4 fps)

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# Pond p19:

Wetland Geometry to be confirmed by survey Based off aerial topo, and assumed topo contour

Inflow Area = Inflow = Outflow = Primary = Secondary =	3.05 cfs 1.34 cfs 1.34 cfs	c, Inflow Depth = 0.49' @ 12.70 hrs, Volume= @ 13.55 hrs, Volume= @ 13.55 hrs, Volume= @ 0.00 hrs, Volume=	= 0.633 af = 0.631 af, = 0.631 af	Atten= 56%, Lag= 50.5 min		
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 972.00' Surf.Area= 86,000 sf Storage= 57,333 cf Peak Elev= 972.06' @ 13.55 hrs Surf.Area= 87,584 sf Storage= 63,780 cf (6,447 cf above start) Plug-Flow detention time= (not calculated) Center-of-Mass det. time= (not calculated)						
# Invert	Avail.S	torage Storage Descri	ption			
1 970.00'	282,	329 cf Custom Stage	Data (Conic) Liste	ed below		
Elevation (feet)	Surf.Area (sq-ft)		Cum.Store (cubic-feet)	Wet.Area (sq-ft)		
970.00	0	0	0	0		
972.00	86,000	57,333	57,333	86,006		
974.00	141,270	224,996	282,329	141,327		
# Routing	Invert	Outlet Devices				
1 Secondary		178.0 deg x 51.0' long Sharp-Crested Vee/Trap Weir C= 2.46				
2 Primary	972.00'	<b>35.0' long x 0.5' bread</b> Head (feet) 0.20 0.40 Coef. (English) 2.80 2	0.60 0.80 1.00			
Primary OutFlow	May-1 3	$1 cfs @ 1355 brs HW_{-}$	972 06' TW-970 (	)7' (Dynamic Tailwater)		

Primary OutFlow Max=1.34 cfs @ 13.55 hrs HW=972.06' TW=970.07' (Dynamic Tailwater) -2=Broad-Crested Rectangular Weir (Weir Controls 1.34 cfs @ 0.7 fps)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=972.00' TW=973.60' (Dynamic Tailwater) 1=Sharp-Crested Vee/Trap Weir (Controls 0.00 cfs)

# Pond p20:

Field Note #50 Spring Fed Pond Geometry to be confirmed by surveyed

Inflow Area	=	241.484 ac, Inflow Depth = 0.35"	for 2-yr event
Inflow	=	10.20 cfs @ 12.47 hrs, Volume=	7.088 af
Outflow	=	3.25 cfs @ 24.98 hrs, Volume=	6.552 af, Atten= 68%, Lag= 750.6 min
Primary	=	3.25 cfs @ 24.98 hrs, Volume=	6.552 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

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Starting Elev= 505.10' Surf.Area= 89,370 sf Storage= 138,524 cf Peak Elev= 505.65' @ 24.98 hrs Surf.Area= 89,376 sf Storage= 187,864 cf (49,340 cf above start) Plug-Flow detention time= 1,128.6 min calculated for 3.371 af (48% of inflow) Center-of-Mass det. time= 178.3 min (1,701.5 - 1,523.2)

#	Invert	Avail.S	torage Storage Description
1	502.00'	615,	682 cf Custom Stage Data (Prismatic) Listed below
	ation (feet)	Surf.Area (sq-ft)	
	02.00	0	
	05.10	89,370	
	06.00	89,380	
	08.00	99,280	
5	10.00	108,781	208,061 615,682
#	Routing	Invert	Outlet Devices
1	Primary	505.10'	3.0' long x 1.5' breadth Broad-Crested Rectangular Weir
	<b>.</b>		Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 Coef. (English) 2.62 2.64 2.64 2.68 2.75 2.86 2.92 3.07 3.07 3.03 3.28 3.32
2	Primary	Primary 506.20'	6.5' long x 1.5' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00
			Coef. (English) 2.62 2.64 2.64 2.68 2.75 2.86 2.92 3.07 3.07 3.03 3.28 3.32
3	Primary	506.00'	176.0 deg x 97.0' long Sharp-Crested Vee/Trap Weir C= 2.46
Prim	ary OutFlow	Max=3.2	5 cfs @ 24.98 hrs HW=505.65' TW=504.44' (Dynamic Tailwater)

Primary OutFlow Max=3.25 cfs @ 24.98 hrs HW=505.65' TW=504.44' (Dynamic Tailwater) -1=Broad-Crested Rectangular Weir (Weir Controls 3.25 cfs @ 2.0 fps) -2=Broad-Crested Rectangular Weir (Controls 0.00 cfs) -3=Sharp-Crested Vee/Trap Weir (Controls 0.00 cfs)

#### Pond p21:

Inflow Area	a =	489.305 ac, Inflow Depth = 0.64"	for 2-yr event
Inflow	=	97.31 cfs @ 12.46 hrs, Volume=	26.013 af
Outflow	=	14.57 cfs @ 16.56 hrs, Volume=	24.673 af, Atten= 85%, Lag= 245.7 min
Primary	=	14.57 cfs @ 16.56 hrs, Volume=	24.673 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 483.07' @ 16.56 hrs Surf.Area= 353,785 sf Storage= 465,156 cf Plug-Flow detention time= 467.0 min calculated for 24.668 af (95% of inflow) Center-of-Mass det. time= 390.0 min (1,486.0 - 1,095.9)

#	Invert	Avail.Storage	Storage Description
1	480.40'	5,244,885 cf	Custom Stage Data (Conic) Listed below

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Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
480.40	0	0	0	0
482.00	202,230	107,856	107,856	202,234
484.00	485,198	667,114	774,970	485,231
486.00	1,275,481	1,698,237	2,473,208	1,275,541
488.00	1,499,208	2,771,678	5,244,885	1,499,423
# Routing	Invert O	utlet Devices	. ,	. ,

1 Primary 480.40' **30.0" x 70.0' long Culvert** CMP, projecting, no headwall, Ke= 0.900 Outlet Invert= 480.40' S= 0.0000 '/' n= 0.024 Cc= 0.900

Primary OutFlow Max=14.57 cfs @ 16.56 hrs HW=483.07' TW=0.00' (Dynamic Tailwater) ←1=Culvert (Barrel Controls 14.57 cfs @ 3.5 fps)

## Pond p22:

Field Note #54 Golf Pond Geometry to be confirmed by survey

Inflow Area	=	97.943 ac, Inflow Depth = 0.83" for 2-yr event
Inflow =	=	43.15 cfs @ 12.49 hrs, Volume= 6.804 af
Outflow =	=	42.65 cfs @ 12.53 hrs, Volume= 6.506 af, Atten= 1%, Lag= 2.7 min
Primary =	=	42.65 cfs @ 12.53 hrs, Volume= 6.506 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 498.10' Surf.Area= 6,520 sf Storage= 10,106 cf Peak Elev= 501.28' @ 12.53 hrs Surf.Area= 10,398 sf Storage= 37,007 cf (26,901 cf above start) Plug-Flow detention time= 83.5 min calculated for 6.274 af (92% of inflow) Center-of-Mass det. time= 34.2 min (947.6 - 913.5)

# Inver	t Avail.S	torage Storage D	escription	
1 495.00	)' 143,	770 cf Custom S	tage Data (Prism	atic) Listed below
Elevation	Surf.Area	Inc.Store	Cum.Store	
(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)	
495.00	0	0	0	
498.10	6,520	10,106	10,106	
500.00	8,390	14,164	24,270	
502.00	11,530	19,920	44,190	
504.00	14,530	26,060	70,250	
506.00	18,340	32,870	103,120	
508.00	22,310	40,650	143,770	
# Routing	Invert	Outlet Devices		
1 Primary	499.75'	18.0" x 21.0' long	g Culvert CMP, p	projecting, no headwall, Ke= 0.900
		Outlet Invert= 499	.75' S= 0.0000 '/	n= 0.024 Cc= 0.900
2 Primary	500.50'	1.0' long x 15.0' k	preadth Broad-Cr	ested Rectangular Weir
-		Head (feet) 0.20	0.40 0.60 0.80	1.00 1.20 1.40 1.60
		Coef. (English) 2.	68 2.70 2.70 2.6	64 2.63 2.64 2.64 2.63

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3 Primary 500.50' **20.0' long x 13.5' breadth Broad-Crested Rectangular Weir** Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.62 2.66 2.70 2.66 2.65 2.66 2.65 2.63

Primary OutFlow Max=42.64 cfs @ 12.53 hrs HW=501.28' TW=499.89' (Dynamic Tailwater) -1=Culvert (Barrel Controls 4.21 cfs @ 2.9 fps) -2=Broad-Crested Rectangular Weir (Weir Controls 1.82 cfs @ 2.3 fps)

-3=Broad-Crested Rectangular Weir (Weir Controls 36.61 cfs @ 2.4 fps)

# Pond p23:

Inflow Area	ι =	41.587 ac, Inflow Depth = 1.06"	for 2-yr event	
Inflow	=	21.80 cfs @ 12.69 hrs, Volume=	3.660 af	
Outflow	=	19.72 cfs @ 12.87 hrs, Volume=	2.742 af,	Atten= 10%, Lag= 10.8 min
Primary	=	19.72 cfs @ 12.87 hrs, Volume=	2.742 af	-

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 507.81' @ 12.87 hrs Surf.Area= 18,661 sf Storage= 41,639 cf Plug-Flow detention time= 148.7 min calculated for 2.742 af (75% of inflow) Center-of-Mass det. time= 53.8 min (956.6 - 902.7)

#	Invert	Avail.St	orage Storage I	Descriptic	'n		
1	503.50'	100,3	303 cf <b>Custom</b>	Stage Da	ta (Prismati	c) Listed below	
Elevatio	•••	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	•	um.Store lbic-feet)		
503.	50	0	0		0		
506.	00	11,170	13,963		13,963		
508.	00	19,460	30,630		44,593		
510.	00	36,250	55,710		100,303		
	outing	Invert	Outlet Devices				
1 P	rimary	507.70'	178.0 deg x 178	.0' long S	harp-Creste	ed Vee/Trap Weir	C= 2.46

Primary OutFlow Max=19.71 cfs @ 12.87 hrs HW=507.81' TW=507.02' (Dynamic Tailwater) -1=Sharp-Crested Vee/Trap Weir (Weir Controls 19.71 cfs @ 1.0 fps)

# Pond zDP1: Design Point 1

Field note #10. Culvert dimensions to be confirmed by survey.

Inflow Area =	26.659 ac, Inflow Depth = 0.87"	for 2-yr event
Inflow =	12.00 cfs @ 12.53 hrs, Volume=	1.939 af
Outflow =	12.00 cfs @ 12.53 hrs, Volume=	1.939 af, Atten= 0%, Lag= 0.0 min
Primary =	12.00 cfs @ 12.53 hrs, Volume=	1.939 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 721.36' @ 12.53 hrs Surf.Area= 60 sf Storage= 38 cf Flood Elev= 727.00' Surf.Area= 1,105 sf Storage= 2,619 cf Plug-Flow detention time= 0.2 min calculated for 1.939 af (100% of inflow)

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#	Invert	Avail.St	orage Storage De	escription		
1	720.10'	3,7	706 cf Custom S	tage Data (Conic)	Listed below	
Eleva	ation feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
72	0.10	0	0	0	0	
72	2.00	90	57	57	96	
72	4.00	340	403	460	364	
72	6.00	760	1,072	1,533	815	
72	8.00	1,450	2,173	3,706	1,543	
#	Routing	Invert	Outlet Devices			
1	Primary		<b>42.0" x 120.0' lon</b> Outlet Invert= 700.	-		
2	Primary		155.0 deg Sharp-0			

Center-of-Mass det. time= 0.1 min (905.2 - 905.0)

Primary OutFlow Max=12.00 cfs @ 12.53 hrs HW=721.36' TW=686.21' (Dynamic Tailwater) -1=Culvert (Inlet Controls 12.00 cfs @ 3.8 fps) -2=Sharp-Crested Vee/Trap Weir (Controls 0.00 cfs)

# Pond zDP2: Design Point 2

Field Note #15 Culvert dimensions to be confirmed by survey Overflow to ditch is currently discarded... We may have to model that area...

Inflow Area =	97.712 ac, Inflow Depth = 0.70"	for 2-yr event
Inflow =	25.54 cfs @ 12.95 hrs, Volume=	5.701 af
Outflow =	25.49 cfs @ 13.00 hrs, Volume=	5.701 af, Atten= 0%, Lag= 3.1 min
Discarded =	0.00 cfs @ 0.00 hrs, Volume=	0.000 af
Primary =	25.49 cfs @ 13.00 hrs, Volume=	5.701 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 623.44' @ 13.00 hrs Surf.Area= 620 sf Storage= 918 cf Flood Elev= 624.50' Surf.Area= 925 sf Storage= 1,728 cf Plug-Flow detention time= 0.4 min calculated for 5.701 af (100% of inflow) Center-of-Mass det. time= 0.3 min (941.2 - 940.9)

#	Invert	Avail.Storag	ge Storage Des	cription	
1	619.60'	7,280	cf Custom Sta	<b>ge Data (Conic)</b> Li	sted below
Elevati (fe	ion et)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
619.		0	0	0	0
620.		10	1	1	10
622.		260	214	215	269
624.	.00	760	976	1,192	793
626.	.00	1,420	2,146	3,338	1,492
628.	.00	2,580	3,943	7,280	2,694

#	Routing	Invert	Outlet Devices
1	Primary	619.60'	24.0" x 150.0' long Culvert RCP, end-section conforming to fill, Ke= 0.500
			Outlet Invert= 608.00' S= 0.0773 '/' n= 0.012 Cc= 0.900
2	Discarded	624.50'	166.0 deg Sharp-Crested Vee/Trap Weir C= 2.46

Discarded OutFlow Max=0.00 cfs @ 0.00 hrs HW=619.60' (Free Discharge) ←2=Sharp-Crested Vee/Trap Weir (Controls 0.00 cfs)

Primary OutFlow Max=25.49 cfs @ 13.00 hrs HW=623.44' TW=607.58' (Dynamic Tailwater) ☐ 1=Culvert (Inlet Controls 25.49 cfs @ 8.1 fps)

## Pond zDP3: Design Point 3

Inflow Area	a =	212.742 ac, Ir	nflow Depth = 18	.52" for	2-yr event		
Inflow	=	121.33 cfs @	12.95 hrs, Volui	me=	328.340 af		
Primary	=	121.33 cfs @	12.95 hrs, Volu	me=	328.340 af,	Atten= 0%,	Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

## Pond zDP4: Design Point 4

Inflow Area	a =	489.305 ac, Inf	flow Depth = 0.61"	for 2-yr event	
Inflow	=	14.57 cfs @ 1	16.56 hrs, Volume=	24.673 af	
Primary	=	14.57 cfs @ 1	16.56 hrs, Volume=	24.673 af,	Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

# Pond zDP5: Design Point 5

Inflow Area =	28.325  ac,  Inflow Depth = 0.95"	for 2-yr event	
Inflow =	16.05 cfs @ 12.48 hrs, Volume=	2.235 af	
Primary =	16.05 cfs @ 12.48 hrs, Volume=	2.235 af, Atten= 0%, Lag= 0.0 min	1

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

# Pre-Development Conditions 10 year 24 hour Storm Event Model Computations

Existing Conditions_10454-01Type III 24-hr 10-yrRainfall=5.00"Prepared by The Chazen CompaniesPage 67HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems4/10/2006 2:33:30 PM
Subcatchment s01:
Runoff = 11.66 cfs @ 12.61 hrs, Volume= 1.799 af, Depth= 1.88"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=5.00"
Area (ac) CN Description
11.485 68
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
42.8 Direct Entry,
Subcatchment s02:
Runoff = 69.51 cfs @ 12.87 hrs, Volume= 13.464 af, Depth= 1.65"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=5.00"
Area (ac) CN Description
97.712 65
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
61.3 Direct Entry,
Subcatchment s03:
Runoff = 19.53 cfs @ 12.42 hrs, Volume= 2.475 af, Depth= 1.96"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=5.00"
Area (ac) CN Description
15.174 69
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
28.8 Direct Entry,
Subcatchment s04:
Runoff = 20.66 cfs @ 12.10 hrs, Volume= 1.571 af, Depth= 1.65"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=5.00"

Existing Conditions_10454-01 Prepared by The Chazen Companies HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Syste	Type III 24-hr 10-yr Rainfall=5.00'           Page 68           ems         4/10/2006 2:33:30 PM
Area (ac) CN Description 11.403 65	
11.100 00	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
6.5 Direct Entry,	
Subcatchment s05:	
Runoff = 15.43 cfs @ 12.26 hrs, Volume= 1.704 a	f, Depth= 1.37"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs Type III 24-hr 10-yr Rainfall=5.00"	s, dt= 0.01 hrs
Area (ac) CN Description	
14.935 61	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
17.3 Direct Entry,	
Subcatchment s06:	
Runoff = 9.91 cfs @ 12.26 hrs, Volume= 1.080 a	f, Depth= 1.44"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs Type III 24-hr 10-yr Rainfall=5.00"	s, dt= 0.01 hrs
Area (ac) CN Description	
9.007 62	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
17.3 Direct Entry,	
Subcatchment s06(OW): s06 Ope	en Water
Runoff = 2.57 cfs @ 12.00 hrs, Volume= 0.178 a	f, Depth= 5.00"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs Type III 24-hr 10-yr Rainfall=5.00"	s, dt= 0.01 hrs
Area (ac) CN Description	
0.428 100	

Existing Conditions_10454-01Type III 24-hr 10-yr Rainfall=5.00"Prepared by The Chazen CompaniesPage 69HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems4/10/2006 2:33:30 PM
Subcatchment s07:
Runoff = 9.18 cfs @ 12.21 hrs, Volume= 0.897 af, Depth= 1.58"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=5.00"
Area (ac) CN Description
6.811 64
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
13.9 Direct Entry,
Subcatchment s07(OW): s07 Open Water
Runoff = 3.04 cfs @ 12.00 hrs, Volume= 0.211 af, Depth= 5.00"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=5.00"
Area (ac) CN Description
0.506 100
Subcatchment s08:
Runoff = 24.28 cfs @ 12.35 hrs, Volume= 3.089 af, Depth= 1.17"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=5.00"
Area (ac) CN Description
31.719 58
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
21.8 Direct Entry,
Subcatchment s09:
Runoff = 11.29 cfs @ 12.24 hrs, Volume= 1.165 af, Depth= 1.65"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=5.00"
Area (ac) CN Description 8.452 65

Existing Conditions_10454-01 Type III 24-hr 10-yr Rainfall=5.00"
Prepared by The Chazen CompaniesPage 70HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems4/10/2006 2:33:30 PM
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
16.4 <b>Direct Entry</b> ,
Subcatchment s10:
Runoff = 9.66 cfs @ 12.43 hrs, Volume= 1.222 af, Depth= 1.80"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=5.00"
Area (ac) CN Description
8.130 67
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
27.9Direct Entry,
Subcatchment s10(OW): s10 Open Water
Runoff = 4.98 cfs @ 12.00 hrs, Volume= 0.346 af, Depth= 5.00"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=5.00"
Area (ac) CN Description 0.830 100
Subcatchment s11:
Subcatchment STT.
Runoff = 3.52 cfs @ 12.30 hrs, Volume= 0.386 af, Depth= 1.96"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=5.00"
Area (ac) CN Description
2.364 69
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
20.5 Direct Entry,
Subcatchment s11(IC): s11 Imp. Cover
Runoff = 12.83 cfs @ 12.04 hrs, Volume= 0.928 af, Depth= 4.76"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=5.00"

Existing Conditions_10454-01	Type III 24-hr 10-yr Rainfall=5.00"
Prepared by The Chazen Companies HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Syste	Page 71 ems 4/10/2006 2:33:30 PM
Area (ac) CN Description 2.338 98	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
2.8 Direct Entry,	
Subcatchment s12:	
Runoff = 2.87 cfs @ 12.69 hrs, Volume= 0.524 af	f, Depth= 0.98"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs Type III 24-hr 10-yr Rainfall=5.00"	, dt= 0.01 hrs
Area (ac) CN Description	
6.420 55	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
42.0 Direct Entry,	
Subcatchment s13:	
Runoff = $0.29 \text{ cfs} @ 12.14 \text{ hrs}$ , Volume= $0.029 \text{ af}$	f, Depth= 0.98"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs Type III 24-hr 10-yr Rainfall=5.00"	, dt= 0.01 hrs
Area (ac) CN Description	
0.350 55	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
8.6 Direct Entry,	
Subcatchment s14:	
Runoff = 18.31 cfs @ 12.43 hrs, Volume= 2.380 af	f, Depth= 1.58"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs Type III 24-hr 10-yr Rainfall=5.00"	, dt= 0.01 hrs
Area (ac) CN Description	
18.066 64	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
28.1 Direct Entry,	

Existing Conditions_10454-01Type III 24-hr 10-yr Rainfall=5.00"Prepared by The Chazen CompaniesPage 72HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems4/10/2006 2:33:30 PM
Subcatchment s14(IC): s14 Imp. Cover
Runoff = 13.29 cfs @ 12.03 hrs, Volume= 0.945 af, Depth= 4.76"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=5.00"
Area (ac) CN Description 2.380 98
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
2.3 Direct Entry,
Subcatchment s14(OW): s14 Open Water
Runoff = 3.11 cfs @ 12.00 hrs, Volume= 0.216 af, Depth= 5.00"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=5.00"
Area (ac) CN Description
0.518 100
Subcatchment s15:
Runoff = 0.49 cfs @ 12.24 hrs, Volume= 0.066 af, Depth= 0.75"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=5.00"
Area (ac) CN Description
1.068 51
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
13.1 Direct Entry,
Subcatchment s16:
Runoff = 84.57 cfs @ 12.32 hrs, Volume= 9.789 af, Depth= 1.73"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=5.00"
Area (ac) CN Description
67.994 66

Existing Conditions_10454-01 Prepared by The Chazen Companies	Type III 24-hr 10-yr Rainfall=5.00" Page 73
HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Syste	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
22.3Direct Entry,	
Subcatchment s16(IC): s16 Imp.	Cover
Runoff = 14.37 cfs @ 12.04 hrs, Volume= 1.044 at	f, Depth= 4.76"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs Type III 24-hr 10-yr Rainfall=5.00"	, dt= 0.01 hrs
Area (ac) CN Description	
2.629 98	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
2.9Direct Entry,	
Subcatchment s16(OW): s16 Ope	n Water
Runoff = 32.12 cfs @ 12.00 hrs, Volume= 2.230 at	f, Depth= 5.00"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs Type III 24-hr 10-yr Rainfall=5.00"	, dt= 0.01 hrs
Area (ac) CN Description 5.351 100	
Subcatchment s17:	
Runoff = 53.87 cfs @ 13.77 hrs, Volume= 16.676 at	f, Depth= 1.73"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs Type III 24-hr 10-yr Rainfall=5.00"	, dt= 0.01 hrs
Area (ac) CN Description 115.827 66	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
125.2Direct Entry,	
Subcatchment s17(OW): s17 Ope	n Water
Runoff = 0.98 cfs @ 12.00 hrs, Volume= 0.068 at	f, Depth= 5.00"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs Type III 24-hr 10-yr Rainfall=5.00"	, dt= 0.01 hrs

Existing Conditions_10454-01Type III 24-hr 10-yr RainfalPrepared by The Chazen CompaniesF	// <i>=5.00"</i> Page 74
HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems 4/10/2006 2:33	•
Area (ac) CN Description 0.164 100	
Subcatchment s18:	
Runoff = 28.52 cfs @ 12.36 hrs, Volume= 3.404 af, Depth= 1.80"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=5.00"	
Area (ac) CN Description	
22.654 67	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
24.4Direct Entry,	
Subcatchment s18(OW): s18 Open Water	
Runoff = 2.83 cfs @ 12.00 hrs, Volume= 0.197 af, Depth= 5.00"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=5.00"	
Area (ac) CN Description	
0.472 100	
Subcatchment s19:	
Runoff = 10.40 cfs @ 12.62 hrs, Volume= 1.683 af, Depth= 1.30"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=5.00"	
Area (ac) CN Description	
15.520 60	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
40.4Direct Entry,	
Subcatchment s20:	
Runoff = 22.28 cfs @ 12.50 hrs, Volume= 3.043 af, Depth= 1.96"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs	

Type III 24-hr 10-yr Rainfall=5.00"

Existing Conditions_10454-01 Prepared by The Chazen Companies HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Syste	Type III 24-hr 10-yr Rainfall=5.00 Page 75 ems 4/10/2006 2:33:30 PM
Area (ac) CN Description 18.655 69	
18.655 69	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
33.9Direct Entry,	
Subcatchment s20(OW): s20 Ope	n Water
Runoff = 11.81 cfs @ 12.00 hrs, Volume= 0.820 a	f, Depth= 5.00"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs Type III 24-hr 10-yr Rainfall=5.00"	s, dt= 0.01 hrs
Area (ac) CN Description 1.968 100	
Subcatchment s21:	
Runoff = 122.06 cfs @ 12.40 hrs, Volume= 15.045 a	f, Depth= 1.88"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs Type III 24-hr 10-yr Rainfall=5.00"	s, dt= 0.01 hrs
Area (ac) CN Description	
96.056 68	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
26.7 Direct Entry,	
Subcatchment s21(OW):	
Runoff = 73.44 cfs @ 12.00 hrs, Volume= 5.098 a	f, Depth= 5.00"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs Type III 24-hr 10-yr Rainfall=5.00"	s, dt= 0.01 hrs
Area (ac) CN Description 12.235 100	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	

Existing Conditions_10454-01Type III 24-hr 10-yr Rainfall=5.00"Prepared by The Chazen CompaniesPage 76HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems4/10/2006 2:33:30 PM				
Subcatchment s22:				
Runoff = 101.91 cfs @ 12.46 hrs, Volume= 13.422 af, Depth= 1.96"				
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=5.00"				
Area (ac) CN Description 82.287 69				
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)				
31.3 Direct Entry,				
Subcatchment s22(OW): s22 Open Water				
Runoff = 0.82 cfs @ 12.00 hrs, Volume= 0.057 af, Depth= 5.00"				
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=5.00"				
Area (ac) CN Description				
0.136 100				
Subcatchment s23:				
Runoff = 47.82 cfs @ 12.69 hrs, Volume= 7.617 af, Depth= 2.20"				
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=5.00"				
Area (ac) CN Description				
41.587 72				
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)				
47.0 Direct Entry,				
Subcatchment s24:				
Runoff = 37.09 cfs @ 12.45 hrs, Volume= 4.807 af, Depth= 2.04"				
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=5.00"				
Area (ac) CN Description 28.325 70				

		itions_1						Type III 2-	4-hr 10-yr	Rai	
			ו Compai <u>7 © 1986-</u>	2003 Applie	d Micro	compute	er Systen	าร	4/10/20	006	Page <u>2:33:31 F</u>
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Desc	ription					
30.7			( ,		Direc	t Entry	,				
				Subca	atchm	nent s2	25:				
Runoff	=	17.56 cfs	s@ 12.3	0 hrs, Volu	me=		1.953 af,	Depth=	1.73"		
		R-20 meth yr Rainfa		SCS, Time S	Span=	0.00-48	3.00 hrs,	dt= 0.01 h	rs		
Area	(ac) C	N Desc	cription								
13.	562 6	66									
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Desc	ription					
20.2			· · · ·		Direc	t Entry	,				
				R	each	r03:					
Overlanc Requires Inflow Ar Inflow Outflow	s more su	11.485 a 11.66 cfs	s@ 12.6	Depth = 1 1 hrs, Volu 5 hrs, Volu	me=		yr event 1.799 af 1.799 af,	Atten= 09	%, Lag= 2.	.7 m	'n
Max. Vel	ocity= 4.	9 fps, Mir	n. Travel 7	ne Span= 0. Time= 2.6 m Time= 6.7 r	nin	00 hrs,	dt= 0.01	hrs			
Capacity Inlet Inve	at bank ert= 845.0		4 cfs et Invert= 7	728.00' nel, n= 0.0	060 L	ength=	785.0' \$	Slope= 0.1	490 '/'		
				R	each	r04:					
Channel											
Inflow Ar Inflow Outflow	=	29.17 cfs	s@ 12.5	Depth = 1 0 hrs, Volu 2 hrs, Volu	me=	4	yr event 4.274 af 4.274 af,	Atten= 0°	%, Lag= 0.	.9 mi	'n
Routing I	ov Dvn-S	stor-Ind m	othod Tim	o Span- 0	00.40	00 6		hro			

Existing Conditions 10454-01 Type III 24-hr 10-yr Rainfall=5.00" Prepared by The Chazen Companies HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems 4/10/2006 2:33:31 PM

Peak Depth= 1.08' @ 12.52 hrs Capacity at bank full= 446.15 cfs Inlet Invert= 685.50', Outlet Invert= 608.00' 12.00' x 4.00' deep Parabolic Channel, n= 0.060 Length= 675.0' Slope= 0.1148 '/'

## Reach r08a:

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Man Made Ditch Inverts of pipe to be surveyed

Inflow Are	a =	97.712 ac, Inflow	Depth = $1.39$ "	for 10-yr event	
Inflow	=	34.34 cfs @ 12.8	88 hrs, Volume=	11.317 af	
Outflow	=	34.33 cfs @ 12.8	39 hrs, Volume=	11.317 af,	Atten= 0%, Lag= 0.3 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 9.4 fps, Min. Travel Time= 0.4 min Avg. Velocity = 5.1 fps, Avg. Travel Time= 0.7 min

Peak Depth= 0.67' @ 12.89 hrs Capacity at bank full= 81.88 cfs Inlet Invert= 607.00', Outlet Invert= 587.00' 10.00' x 1.00' deep Parabolic Channel, n= 0.027 Length= 226.0' Slope= 0.0885 '/'

### Reach r08b:

24" HDPE Inverts to be surveyed

Inflow Area = 97.712 ac, Inflow Depth = 1.39" for 10-yr event Inflow = 34.33 cfs @ 12.89 hrs, Volume= 11.317 af Outflow 34.33 cfs @ 12.89 hrs, Volume= 11.317 af, Atten= 0%, Lag= 0.2 min =

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 23.8 fps, Min. Travel Time= 0.2 min Avg. Velocity = 13.3 fps, Avg. Travel Time= 0.4 min

Peak Depth= 0.93' @ 12.89 hrs Capacity at bank full= 77.17 cfs Inlet Invert= 587.00', Outlet Invert= 557.75' 24.0" Diameter Pipe n= 0.012 Length= 295.0' Slope= 0.0992 '/'

## Reach r08c:

Ditch Pipe inverts to be surveyed

Inflow Area = 97.712 ac, Inflow Depth = 1.39" for 10-yr event Inflow 34.33 cfs @ 12.89 hrs, Volume= 11.317 af = 34.33 cfs @ 12.90 hrs, Volume= Outflow 11.317 af, Atten= 0%, Lag= 0.8 min =

Existing Conditions\_10454-01 Type I Prepared by The Chazen Companies HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 9.0 fps, Min. Travel Time= 1.1 min Avg. Velocity = 4.6 fps, Avg. Travel Time= 2.1 min

Peak Depth= 0.69' @ 12.90 hrs Capacity at bank full= 76.65 cfs Inlet Invert= 557.75', Outlet Invert= 512.00' 10.00' x 1.00' deep Parabolic Channel, n= 0.027 Length= 590.0' Slope= 0.0775 '/'

## Reach r08d: Amenia Stream/Cascade Brook

(Steam) parallel to Route 22 Base Flow estimated from field observation (see field note 21)

Inflow Area = Inflow = Outflow =	97.712 ac, Inflow Depth = 20.88" 74.33 cfs @ 12.90 hrs, Volume= 74.31 cfs @ 12.96 hrs, Volume=	= 170.028 af, Incl. 40.00 cfs Base Flow
Max. Velocity= 3	Stor-Ind method, Time Span= 0.00-4 3.6 fps, Min. Travel Time= 3.7 min 3.1 fps, Avg. Travel Time= 4.4 min	
		Length= 805.0' Slope= 0.0099 '/'

#### Reach r14a:

Grass lined channel

Inflow Area =	8.452 ac, Inflow Depth = $1.60$ "	for 10-yr event
Inflow =	10.15 cfs @ 12.33 hrs, Volume=	1.126 af
Outflow =	10.14 cfs @ 12.35 hrs, Volume=	1.126 af, Atten= 0%, Lag= 0.9 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 6.0 fps, Min. Travel Time= 1.0 min Avg. Velocity = 1.5 fps, Avg. Travel Time= 4.0 min

Peak Depth= 0.58' @ 12.35 hrs Capacity at bank full= 325.42 cfs Inlet Invert= 542.00', Outlet Invert= 526.00' 10.00' x 3.00' deep Parabolic Channel, n= 0.027 Length= 360.0' Slope= 0.0444 '/'

## Reach r14b:

30" HDPE Under Main Entrance Road

 Inflow Area =
 8.452 ac, Inflow Depth =
 1.60" for 10-yr event

 Inflow =
 10.14 cfs @
 12.35 hrs, Volume=
 1.126 af

 Outflow =
 10.14 cfs @
 12.35 hrs, Volume=
 1.126 af, Atten= 0%, Lag= 0.4 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 12.6 fps, Min. Travel Time= 0.6 min Avg. Velocity = 3.2 fps, Avg. Travel Time= 2.4 min

Peak Depth= 0.55' @ 12.35 hrs Capacity at bank full= 94.91 cfs Inlet Invert= 526.00', Outlet Invert= 505.70' 30.0" Diameter Pipe n= 0.012 Length= 445.0' Slope= 0.0456 '/'

## Reach r14c:

### **Overland Flow**

Inflow Area = Inflow = Outflow =	6.420 ac, Inflow Depth = 0.04"for 10-yr event0.07 cfs @ 24.61 hrs, Volume=0.021 af0.06 cfs @ 24.87 hrs, Volume=0.021 af, Atten= 6%, Lag= 15.6 min
Max. Velocity= 0.5	tor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs 5 fps, Min. Travel Time= 20.2 min 3 fps, Avg. Travel Time= 34.4 min

## Reach r15:

Brush Overbanks with Rocky Bottom Needs to be surveyed

 Inflow Area =
 4.702 ac, Inflow Depth =
 3.35" for 10-yr event

 Inflow =
 13.72 cfs @
 12.06 hrs, Volume=
 1.314 af

 Outflow =
 13.67 cfs @
 12.06 hrs, Volume=
 1.314 af, Atten= 0%, Lag= 0.4 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 9.6 fps, Min. Travel Time= 0.5 min Avg. Velocity = 3.3 fps, Avg. Travel Time= 1.5 min

Peak Depth= 0.82' @ 12.06 hrs Capacity at bank full= 188.47 cfs Inlet Invert= 554.00', Outlet Invert= 528.00' 5.00' x 3.00' deep Parabolic Channel, n= 0.027 Length= 290.0' Slope= 0.0897 '/'

# Reach r16:

Pipe Reach

Inflow Area =       4.702 ac, Inflow Depth = 3.35" for 10-yr event         Inflow =       14.13 cfs @ 12.04 hrs, Volume=       1.314 af         Outflow =       13.72 cfs @ 12.06 hrs, Volume=       1.314 af, Atten= 3%, Lag= 1.0 min					
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 10.6 fps, Min. Travel Time= 1.3 min Avg. Velocity = 3.5 fps, Avg. Travel Time= 4.1 min					
Peak Depth= 0.77' @ 12.06 hrs Capacity at bank full= 66.05 cfs Inlet Invert= 573.00', Outlet Invert= 554.00' 30.0" Diameter Pipe n= 0.012 Length= 860.0' Slope= 0.0221 '/'					
Reach r18a:					
Overland Flow Reach					
Inflow         =         0.00 cfs @         0.00 hrs, Volume=         0.000 af           Outflow         =         0.00 cfs @         0.00 hrs, Volume=         0.000 af, Atten= 0%, Lag= 0.0 min					
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 0.0 fps, Min. Travel Time= 0.0 min Avg. Velocity = 0.0 fps, Avg. Travel Time= 0.0 min					
Peak Depth= 0.00' @ 0.00 hrs Capacity at bank full= 379.63 cfs Inlet Invert= 973.60', Outlet Invert= 530.00' 50.00' x 1.00' deep Parabolic Channel, n= 0.060 Length= 1,220.0' Slope= 0.3636 '/'					
Reach r18b:					
Overland Flow Reach					
Inflow         =         0.00 cfs @         0.00 hrs, Volume=         0.000 af           Outflow         =         0.00 cfs @         0.00 hrs, Volume=         0.000 af, Atten= 0%, Lag= 0.0 min					
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 0.0 fps, Min. Travel Time= 0.0 min Avg. Velocity = 0.0 fps, Avg. Travel Time= 0.0 min					
Peak Depth= 0.00' @ 0.00 hrs Capacity at bank full= 151.94 cfs Inlet Invert= 530.60', Outlet Invert= 514.00' 50.00' x 1.00' deep Parabolic Channel, n= 0.060 Length= 285.0' Slope= 0.0582 '/'					

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# Reach r21a:

Man Made Ditch

# Reach r21c:

**Overland Flow Reach** 

Inflow Area = 41.587 ac, Inflow Depth = 1.93" for 10-yr event Inflow = 47.78 cfs @ 12.69 hrs, Volume= 6.699 af Outflow = 47.76 cfs @ 12.70 hrs, Volume= 6.699 af, Atten= 0%, Lag= 0.4 min Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 4.2 fps, Min. Travel Time= 0.6 min Avg. Velocity = 1.6 fps, Avg. Travel Time= 1.6 min Existing Conditions 10454-01 Prepared by The Chazen Companies HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems

Peak Depth= 0.49' @ 12.70 hrs Capacity at bank full= 227.81 cfs Inlet Invert= 506.70', Outlet Invert= 485.75' 50.00' x 1.00' deep Parabolic Channel, n= 0.060 Length= 160.0' Slope= 0.1309 '/'

## Reach r22a:

**Overland Flow Reach** 

Inflow Area = Inflow = Outflow =	15.520 ac, Inflow Depth = 1.30" 5.85 cfs @ 13.10 hrs, Volume= 5.81 cfs @ 13.17 hrs, Volume=	1.681 af	
Max. Velocity= 3.	Stor-Ind method, Time Span= 0.00-48 .3 fps, Min. Travel Time= 4.9 min .2 fps, Avg. Travel Time= 14.0 min	8.00 hrs, dt= 0.01 hrs	
Peak Depth= 0.14 Capacity at bank Inlet Invert= 970.0			

50.00' x 1.00' deep Parabolic Channel, n= 0.060 Length= 970.0' Slope= 0.4227 '/'

# Reach r22b:

**Overland Flow Reach** 

Inflow Area = Inflow = Outflow =	15.520 ac, Inflow Depth =1.30"for10-yr event5.81 cfs @13.17 hrs, Volume=1.681 af5.77 cfs @13.24 hrs, Volume=1.681 af, Atten= 1%, Lag= 4.5 min	
Max. Velocity= 1.	tor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs 8 fps, Min. Travel Time= 5.6 min 6 fps, Avg. Travel Time= 16.8 min	

## Reach r25a:

Ditch Pipe inverts need to be surveyed

60.314 ac, Inflow Depth =  $1.70^{\circ}$  for 10-yr event Inflow Area = Inflow 50.11 cfs @ 12.40 hrs, Volume= 8.561 af = Outflow 50.03 cfs @ 12.42 hrs. Volume= 8.561 af, Atten= 0%, Lag= 1.1 min =

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 10.2 fps, Min. Travel Time= 1.8 min Avg. Velocity = 2.3 fps, Avg. Travel Time= 8.0 min

Peak Depth= 1.03' @ 12.42 hrs Capacity at bank full= 205.50 cfs Inlet Invert= 570.00', Outlet Invert= 504.00'

10.00' x 2.00' deep Parabolic Channel, n= 0.027 Length= 1,090.0' Slope= 0.0606 '/'

## Reach r25b: Wetland Reach

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Wetland Reach Has wetland vegetation within reach

Inflow Area = Inflow = Outflow =	9.435 ac, Inflow Depth = 8.28 cfs @ 12.43 hrs, Vol 7.38 cfs @ 12.56 hrs, Vol	ume= 1.229 a	
Max. Velocity= 1.6	tor-Ind method, Time Span= ( 6 fps, Min. Travel Time= 8.0 4 fps, Avg. Travel Time= 29.8	min	)1 hrs
		.045 Length= 750.0'	Slope= 0.0060 '/'

### Reach r25c: Amenia Stream/Cascade Brook

(Steam) parallel to Route 22 Base Flow estimated from field observation (see field note 21)

Inflow Area = 129.431 ac, Inflow Depth = 30.74" for 10-yr event Inflow 131.17 cfs @ 12.54 hrs, Volume= 331.584 af, Incl. 40.00 cfs Base Flow = 129.74 cfs @ 12.63 hrs, Volume= Outflow = 330.902 af. Atten= 1%. Lag= 5.5 min Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

Max. Velocity= 2.9 fps, Min. Travel Time= 5.6 min Avg. Velocity = 2.6 fps, Avg. Travel Time= 6.1 min

Peak Depth= 5.85' @ 12.63 hrs Capacity at bank full= 67.14 cfs Inlet Invert= 504.00', Outlet Invert= 500.00' 10.00' x 4.00' deep Parabolic Channel, n= 0.060 Length= 975.0' Slope= 0.0041 '/'

## Pond 8P:

No field note. Water spills over cart path; no storage.

Inflow Area =	52.997 ac, Inflow Depth = 1.69"	for 10-yr event
Inflow =	48.34 cfs @ 12.39 hrs, Volume=	7.483 af
Outflow =	48.34 cfs @ 12.39 hrs, Volume=	7.483 af, Atten= 0%, Lag= 0.0 min
Primary =	48.34 cfs @ 12.39 hrs, Volume=	7.483 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 575.47' @ 12.39 hrs Flood Elev= 574.70' Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= (not calculated)

#	Routing	Invert	Outlet Devices
1	Primary	574.70'	177.0 deg Sharp-Crested Vee/Trap Weir C= 2.46

Primary OutFlow Max=48.34 cfs @ 12.39 hrs HW=575.47' TW=571.03' (Dynamic Tailwater) **1=Sharp-Crested Vee/Trap Weir** (Weir Controls 48.34 cfs @ 2.2 fps)

## Pond p04:

Field Note #13

Water ponding behind a golf cart path. Overflow dimensions are assumed based on aerial topo, and should be upgraded once survey is available.

Inflow Area =	38.062 ac, Inflow Depth = 1.84"	for 10-yr event
Inflow =	35.71 cfs @ 12.45 hrs, Volume=	5.845 af
Outflow =	35.71 cfs @ 12.45 hrs, Volume=	5.779 af, Atten= 0%, Lag= 0.2 min
Primary =	35.71 cfs @ 12.45 hrs, Volume=	5.779 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 605.94' @ 12.45 hrs Surf.Area= 6,419 sf Storage= 3,852 cf Flood Elev= 605.50' Surf.Area= 4,803 sf Storage= 2,882 cf Plug-Flow detention time= 10.2 min calculated for 5.778 af (99% of inflow) Center-of-Mass det. time= 3.6 min (880.6 - 876.9)

# In	nvert Avail.S	torage Storage D	Description		
1 604	4.20' 26	,897 cf <b>Custom S</b>	Stage Data (Conic)	Listed below	
Elevation (feet)	Surf.Area (sq-ft)		Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
604.20	0	0	0	0	
606.00	6,650	3,990	3,990	6,655	
608.00	17,060	22,907	26,897	17,092	
# Routir	ng Invert	Outlet Devices			
1 Prima	ry 605.50'	179.0 deg Sharp-	Crested Vee/Trap	<b>Weir</b> C= 2.46	

Primary OutFlow Max=35.71 cfs @ 12.45 hrs HW=605.94' TW=575.46' (Dynamic Tailwater) **1=Sharp-Crested Vee/Trap Weir** (Weir Controls 35.71 cfs @ 1.6 fps) HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems

## Pond p06:

Field Note #22 Geometry to be confirmed by survey.

Inflow Area =	9.435 ac, Inflow Depth = $1.60$ "	for 10-yr event
Inflow =	10.64 cfs @ 12.25 hrs, Volume=	1.258 af
Outflow =	8.28 cfs @ 12.43 hrs, Volume=	1.229 af, Atten= 22%, Lag= 10.5 min
Primary =	8.28 cfs @ 12.43 hrs, Volume=	1.229 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 506.80' Surf.Area= 18,600 sf Storage= 42,160 cf Peak Elev= 507.40' @ 12.43 hrs Surf.Area= 21,338 sf Storage= 55,020 cf (12,860 cf above start) Flood Elev= 507.10' Surf.Area= 19,958 sf Storage= 48,537 cf (6,377 cf above start) Plug-Flow detention time= 875.3 min calculated for 0.262 af (21% of inflow) Center-of-Mass det. time= 143.4 min (1,000.6 - 857.2)

#	Invert	Avail.St	torage	Storage Description			
1	500.00'	67,	669 cf	Custom Sta	Custom Stage Data (Conic) Listed below		
Eleva (fe	tion eet)	Surf.Area (sq-ft)	(1	Inc.Store cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
500	0.00	0		0	0	0	
506	6.80	18,600		42,160	42,160	18,672	
508	3.00	24,030		25,509	67,669	24,138	
#	Routing	Invert	Outlet	Devices			
1	Primary	506.80'	12.0"	x 20.0' long C	ulvert CMP, proj	ecting, no headwal	l, Ke= 0.900
2	Primary	507.10'	Outlet Invert= 506.00' S= 0.0400 '/' n= 0.024 Cc= 0.900 178.0 deg Sharp-Crested Vee/Trap Weir C= 2.46				

Primary OutFlow Max=8.28 cfs @ 12.43 hrs HW=507.40' TW=504.66' (Dynamic Tailwater) -1=Culvert (Inlet Controls 1.04 cfs @ 2.1 fps)

-2=Sharp-Crested Vee/Trap Weir (Weir Controls 7.24 cfs @ 1.4 fps)

# Pond p07:

Field Note # 29 Outlet geometry to be confirmed by survey.

Inflow Are	ea =	7.317 ac, Inflow Depth = 1.82"	for 10-yr event
Inflow	=	10.15 cfs @ 12.20 hrs, Volume=	1.108 af
Outflow	=	2.83 cfs @ 12.72 hrs, Volume=	1.078 af, Atten= 72%, Lag= 30.9 min
Primary	=	2.83 cfs @ 12.72 hrs, Volume=	1.078 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 572.80' Surf.Area= 21,640 sf Storage= 56,264 cf Peak Elev= 573.60' @ 12.72 hrs Surf.Area= 25,389 sf Storage= 75,703 cf (19,439 cf above start) Flood Elev= 573.50' Surf.Area= 24,936 sf Storage= 73,351 cf (17,087 cf above start) Plug-Flow detention time= (not calculated) Center-of-Mass det. time= (not calculated)

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#	Invert	Avail.St	torage	Storage Des	cription		
1	565.00'	85,	557 cf	Custom Stage Data (Conic) Listed below			
-	ation (feet)	Surf.Area (sq-ft)	(c	Inc.Store ubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
56	65.00	0		0	0	0	
57	2.80	21,640		56,264	56,264	21,735	
57	74.00	27,290		29,293	85,557	27,424	
#	Routing	Invert	Outlet [	Devices			
1	Primary	572.80'	18.0" x	20.0' long C	ulvert CMP, pro	jecting, no headwa	II, Ke= 0.900
2	Outlet Invert= 572.00' S= 0.0400 '/' n= 0.024 Cc= 0.900						
D	Drimony OutFlow, May 2.02 etc. @ 42.72 http:////C72.00/ TM/ 570.04/ (Dynamia Tailuatar)						

**Primary OutFlow** Max=2.83 cfs @ 12.72 hrs HW=573.60' TW=570.91' (Dynamic Tailwater) -1=Culvert (Inlet Controls 2.29 cfs @ 2.4 fps)

-2=Sharp-Crested Vee/Trap Weir (Weir Controls 0.54 cfs @ 0.8 fps)

# Pond p09:

Field Note #31 Geometry to be confirmed by survey.

Inflow Area =	8.452 ac, Inflow Depth = 1.65"	for 10-yr event
Inflow =	11.29 cfs @ 12.24 hrs, Volume=	1.165 af
Outflow =	10.15 cfs @ 12.33 hrs, Volume=	1.126 af, Atten= 10%, Lag= 5.5 min
Primary =	10.15 cfs @ 12.33 hrs, Volume=	1.126 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 550.00' @ 12.33 hrs Surf.Area= 5,516 sf Storage= 6,219 cf Flood Elev= 551.20' Surf.Area= 8,534 sf Storage= 15,673 cf Plug-Flow detention time= 43.2 min calculated for 1.125 af (97% of inflow) Center-of-Mass det. time= 25.0 min (895.5 - 870.5)

#	Invert	Avail.St	torage	Storage Description			
1	547.50'	21,	989 cf	Custom Stage Data (Conic) Listed below			
Eleva	ation feet)	Surf.Area (sq-ft)	(c	Inc.Store ubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
54	7.50	0		0	0	0	
54	8.00	1,080		180	180	1,080	
55	0.00	5,510		6,020	6,200	5,527	
55	2.00	10,550		15,790	21,989	10,606	
#	Routing	Invert	Outlet [	Devices			
1	Primary	548.50' <b>30.0" x 70.0' long Culvert</b> CMP, projecting, no headwall, Ke= 0.900 Outlet Invert= 542.00' S= 0.0929 '/' n= 0.012 Cc= 0.900				-	
-				68.0 deg Sharp-Crested Vee/Trap Weir C= 2.46			

Primary OutFlow Max=10.15 cfs @ 12.33 hrs HW=550.00' TW=542.58' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 10.15 cfs @ 3.3 fps) -2=Sharp-Crested Vee/Trap Weir (Controls 0.00 cfs)

## Pond p10:

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Field Note #25 Need to get full story on how this pond works

Inflow Are	a =	45.146 ac, I	nflow Depth = $0.42$ "	for 10-yr event	
Inflow	=	10.52 cfs @	12.38 hrs, Volume=	1.567 af	
Outflow	=	0.00 cfs @	0.00 hrs, Volume=	0.000 af, Atten=	= 100%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 498.40' Surf.Area= 36,110 sf Storage= 101,108 cf Peak Elev= 500.11' @ 25.62 hrs Surf.Area= 43,112 sf Storage= 169,383 cf (68,275 cf above start) Plug-Flow detention time= (not calculated) Center-of-Mass det. time= (not calculated)

#	Invert	Avail.Stora	ge Storage Des	cription		
1	490.00'	581,029	cf Custom Sta	<b>ge Data (Conic)</b> Li	sted below	
Elevatio		Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft <u>)</u>	
490.0	00	0	0	0	0	
498.4	40	36,110	101,108	101,108	36,221	
500.0	00	42,400	62,741	163,849	42,610	
502.0	00	54,880	97,012	260,861	55,187	
504.0	00	78,730	132,895	393,755	79,107	
506.0	00	109,382	187,274	581,029	109,836	

## Pond p12:

No field note. Natural depression.

Inflow Area =	=	6.420 ac, Inflow Depth = 0.98" for	or 10-yr event
Inflow =	=	2.87 cfs @ 12.69 hrs, Volume=	0.524 af
Outflow =	=	0.07 cfs @ 24.61 hrs, Volume=	0.021 af, Atten= 98%, Lag= 715.0 min
Primary =	=	0.07 cfs @ 24.61 hrs, Volume=	0.021 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 547.58' @ 24.61 hrs Surf.Area= 14,293 sf Storage= 22,575 cf Flood Elev= 547.50' Surf.Area= 13,848 sf Storage= 21,762 cf Plug-Flow detention time= 948.2 min calculated for 0.021 af (4% of inflow) Center-of-Mass det. time= 759.1 min (1,685.1 - 926.0)

# Existing Conditions\_10454-01

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HydroCAD® 7.0	0_s/n 000927_©1	1986-2003 Applied	Microcomputer Sys	tems	4/10/2006	2:33:32 PM
# Inver		00				
1 543.50	)' 26,986	cf Custom Sta	ge Data (Conic) Li	sted below		
Elevation (feet) 543.50 544.00 546.00 548.00	Surf.Area (sq-ft) 0 1,140 5,260 16,710	Inc.Store (cubic-feet) 0 190 5,899 20,897	Cum.Store (cubic-feet) 0 190 6,089 26,986	Wet.Area (sq-ft) 0 1,140 5,278 16,750		
# Routing	Invert Ou	Itlet Devices				
1 Primary	547.50' <b>17</b> 3	3.0 deg Sharp-Cr	ested Vee/Trap W	<b>eir</b> C= 2.46		

Primary OutFlow Max=0.07 cfs @ 24.61 hrs HW=547.58' TW=544.02' (Dynamic Tailwater) ←1=Sharp-Crested Vee/Trap Weir (Weir Controls 0.07 cfs @ 0.7 fps)

## Pond p13:

No Field Note Natural depression.

Inflow Area	=	0.350 ac, Ir	nflow Depth = 0.98"	for 10-yr event	
Inflow =	=	0.29 cfs @	12.14 hrs, Volume=	0.029 af	
Outflow :	=	0.00 cfs @	0.00 hrs, Volume=	0.000 af, Atten= 10	00%, Lag= 0.0 min
Primary :	=	0.00 cfs @	0.00 hrs, Volume=	0.000 af	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 513.41' @ 24.49 hrs Surf.Area= 1,075 sf Storage= 1,245 cf Flood Elev= 519.50' Surf.Area= 4,313 sf Storage= 16,523 cf Plug-Flow detention time= (not calculated) Center-of-Mass det. time= (not calculated)

#	Invert	Avail.St	orage Sto	orage De	escription		
1	511.40'	18,4	490 cf <b>Cu</b>	stom St	age Data (Conic)	Listed below	
Elevat (fe	ion et)	Surf.Area (sq-ft)		:.Store c-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
511.	.40	0		0	0	0	
512.	.00	390		78	78	391	
514.	.00	1,360		1,652	1,730	1,381	
516	.00	2,180		3,508	5,238	2,253	
518.	.00	3,240		5,385	10,623	3,375	
520.	.00	4,670		7,867	18,490	4,872	
# F	Routing	Invert	Outlet Dev	ices			
1 F	Primary	519.50'	176.0 deg	Sharp-C	rested Vee/Trap	<b>Weir</b> C= 2.46	

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=511.40' TW=497.40' (Dynamic Tailwater) ↑ 1=Sharp-Crested Vee/Trap Weir (Controls 0.00 cfs) HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems

# Pond p14:

Field Note #26 Need to figure out how this pond works

Inflow Area = Inflow = Outflow = Primary =	36.186 ac, Inflow Depth = 1.55"       for 10-yr event         31.73 cfs @ 12.37 hrs, Volume=       4.687 af         0.00 cfs @ 0.00 hrs, Volume=       0.000 af, Atten= 100%, Lag= 0.0 min         0.00 cfs @ 0.00 hrs, Volume=       0.000 af						
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 497.40' Surf.Area= 22,200 sf Storage= 54,760 cf Peak Elev= 501.81' @ 48.00 hrs Surf.Area= 71,366 sf Storage= 258,908 cf (204,148 cf above start)							

Plug-Flow detention time= (not calculated) onter-of-Mass det time- (not calculated)

Center-or-mass det.	ume=	(not calcul	aleu)

#	Invert	Avail.Stor	age Storage Des	scription		
1	490.00'	805,06	2 cf Custom Sta	<b>ge Data (Conic)</b> Li	sted below	
Elevat (fe	tion eet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft <u>)</u>	
490	.00	0	0	0	0	
497	.40	22,200	54,760	54,760	22,286	
498	.00	25,330	14,249	69,009	25,433	
500	.00	52,810	76,476	145,485	52,948	
502	.00	73,360	125,608	271,093	73,574	
504	.00	84,070	157,308	428,402	84,467	
506	.00	92,130	176,139	604,540	92,797	
508	.00	108,618	200,522	805,062	109,437	
#_ F	Routing	Invert C	Outlet Devices			
1	Primary		<b>4.0" x 80.0' long C</b> Outlet Invert= 502.0	,	section conformir n= 0.012 Cc= 0.	ng to fill, Ke= 0.500 900

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=497.40' TW=498.40' (Dynamic Tailwater) 1=Culvert (Controls 0.00 cfs)

# Pond p15:

Field Note # 43 Infiltration basin

Inflow Area	a =	5.770 ac, Infl	low Depth = $2.87$ "	for 10-yr event	
Inflow	=	13.80 cfs @ 12	2.07 hrs, Volume=	1.380 af	
Outflow	=	13.68 cfs @ 12	2.08 hrs, Volume=	1.125 af,	Atten= 1%, Lag= 0.6 min
Primary	=	13.68 cfs @ 12	2.08 hrs, Volume=	1.125 af	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 536.19' @ 12.08 hrs Surf.Area= 3,273 sf Storage= 11,851 cf Flood Elev= 536.00' Surf.Area= 3,160 sf Storage= 11,127 cf Plug-Flow detention time= 136.9 min calculated for 1.124 af (81% of inflow)

#	Invert	Avail.St	orage S	Storage De	escription		
1	526.80'	18,5	577 cf <b>C</b>	Custom St	age Data (Conic	) Listed below	
Elevat (fe	tion eet)	Surf.Area (sq-ft)		nc.Store bic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
526	6.80	0		0	0	0	
528	8.00	310		124	124	312	
530	0.00	660		948	1,072	694	
532	2.00	1,180		1,815	2,887	1,256	
534	.00	1,990		3,135	6,022	2,113	
536	6.00	3,160		5,105	11,127	3,337	
538	8.00	4,320		7,450	18,577	4,575	
	Routing		Outlet De				
1 1	Primary	536.00'	171.0 de	g x 50.0'	long Sharp-Cres	ted Vee/Trap We	ir C= 2.46

Center-of-Mass det. time= 58.7 min (848.7 - 790.0)

Primary OutFlow Max=13.65 cfs @ 12.08 hrs HW=536.19' TW=507.64' (Dynamic Tailwater) ←1=Sharp-Crested Vee/Trap Weir (Weir Controls 13.65 cfs @ 1.3 fps)

## Pond p16:

Field Note # 49 Large pond with man-made island. Geometry to be verified by survey. In particular, we are making big guesses about the outlets. Also need to find out about valves...

Inflow Area =	220.861 ac, Inflow Depth = 1.88"	for 10-yr event
Inflow =	112.78 cfs @ 12.32 hrs, Volume=	34.527 af
Outflow =	30.19 cfs @ 16.19 hrs, Volume=	22.733 af, Atten= 73%, Lag= 232.0 min
Primary =	30.19 cfs @ 16.19 hrs, Volume=	22.733 af
,	,	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 507.00' Surf.Area= 199,799 sf Storage= 878,320 cf Peak Elev= 511.03' @ 16.19 hrs Surf.Area= 294,848 sf Storage= 1,777,090 cf (898,770 cf above start) Flood Elev= 510.50' Surf.Area= 271,550 sf Storage= 1,623,217 cf (744,897 cf above start) Plug-Flow detention time= 1.829.6 min calculated for 2.569 af (7% of inflow) Center-of-Mass det. time= 489.7 min (1,418.7 - 929.0)

#	Invert	Avail.Storage	<ul> <li>Storage De</li> </ul>	scription	
1	500.00'	2,062,087 c	f Custom Sta	age Data (Conic) Li	sted below
Elevati (fe	ion et)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
500.	.00	0	0	Ó	0
503.	.00	140,344	140,344	140,344	140,358
509.	.20	232,500	1,143,862	1,284,206	232,994
510.	.00	249,400	192,720	1,476,927	249,951
512.	.00	338,000	585,160	2,062,087	338,634

Existing Conditions 10454-01

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#	Routing	Invert	Outlet Devices
1	Primary	509.00'	18.0" x 110.0' long Culvert CMP, projecting, no headwall, Ke= 0.900
	-		Outlet Invert= 505.70' S= 0.0300 '/' n= 0.024 Cc= 0.900
2	Primary	500.00'	8.0" x 100.0' long assumed equalization pipe w/ valve X 0.00
			CMP, projecting, no headwall, Ke= 0.900
			Outlet Invert= 500.00' S= 0.0000 '/' n= 0.013 Cc= 0.900
3	Primary	510.50'	175.0 deg Sharp-Crested Vee/Trap Weir X 2.00 C= 2.46

Primary OutFlow Max=30.19 cfs @ 16.19 hrs HW=511.03' TW=506.18' (Dynamic Tailwater) -1=Culvert (Inlet Controls 7.59 cfs @ 4.3 fps) -2=assumed equalization pipe w/ valve (Controls 0.00 cfs)

-3=Sharp-Crested Vee/Trap Weir (Weir Controls 22.60 cfs @ 1.8 fps)

## Pond p17:

Field Note #45 Golf pond Geometry to be confirmed by surveyed

Inflow Area	=	115.991 ac, Inflow Depth = 1.73" for 10-yr event	
Inflow :	=	53.92 cfs @ 13.77 hrs, Volume= 16.744 af	
Outflow :	=	53.88 cfs @ 13.78 hrs, Volume= 16.744 af, Atten= 0%, Lag= 0.7 min	۱
Primary :	=	53.88 cfs @ 13.78 hrs, Volume= 16.744 af	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 523.80' Surf.Area= 7,290 sf Storage= 9,234 cf Peak Elev= 525.45' @ 13.78 hrs Surf.Area= 11,046 sf Storage= 24,874 cf (15,640 cf above start) Flood Elev= 524.30' Surf.Area= 8,074 sf Storage= 13,623 cf (4,389 cf above start) Plug-Flow detention time= 22.7 min calculated for 16.532 af (99% of inflow) Center-of-Mass det. time= 11.7 min (979.7 - 967.9)

#	Invert	Avail.Storage		Storage Des	scription				
1	520.00'	30,224 cf		Custom Stage Data (Conic) Listed below					
Elevation (feet)		Surf.Area (sq-ft) (d		Inc.Store cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)			
520.00		0		0	0	0			
523.80		7,290		9,234	9,234	7,313			
524.00		7,300		1,459	10,693	7,374			
526.00		12,460		19,531	30,224	12,581			
#	Routing	Invert	Outlet	Devices					
1	Primary	523.80'	<b>2.2' long x 0.5' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 Coef. (English) 2.80 2.92 3.08 3.30 3.32						
2 3	Primary Primary	524.30' 525.20'	<b>143.0 deg Sharp-Crested Vee/Trap Weir</b> C= 2.47 <b>178.0 deg x 60.0' long Sharp-Crested Vee/Trap Weir</b> C= 2.46						

Type III 24-hr 10-yr Rainfall=5.00" Page 92 4/10/2006 2:33:32 PM Primary OutFlow Max=53.87 cfs @ 13.78 hrs HW=525.45' TW=515.92' (Dynamic Tailwater) 1=Broad-Crested Rectangular Weir (Weir Controls 15.51 cfs @ 4.3 fps) 2=Sharp-Crested Vee/Trap Weir (Weir Controls 10.52 cfs @ 2.7 fps) -3=Sharp-Crested Vee/Trap Weir (Weir Controls 27.85 cfs @ 1.5 fps)

# Pond p18:

Field Note #46 Golf pond Geometry to be confirmed by surveyed

Inflow Area =		139.117 ac, Inflow Depth = 1.75"	for 10-yr event
Inflow =		58.39 cfs @ 13.78 hrs, Volume=	20.345 af
Outflow =		58.22 cfs @ 13.82 hrs, Volume=	20.340 af, Atten= 0%, Lag= 2.1 min
Primary =		58.22 cfs @ 13.82 hrs, Volume=	20.340 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 513.90' Surf.Area= 20,680 sf Storage= 26,884 cf Peak Elev= 515.92' @ 13.82 hrs Surf.Area= 27,974 sf Storage= 75,706 cf (48,822 cf above start) Flood Elev= 514.81' Surf.Area= 23,768 sf Storage= 48,709 cf (21,825 cf above start) Plug-Flow detention time= 60.1 min calculated for 19.723 af (97% of inflow) Center-of-Mass det. time= 33.5 min (992.7 - 959.2 )

#	Invert	Avail.St	torage Storage Description					
1	510.00'	148,	288 cf Custom S	Custom Stage Data (Conic) Listed below				
	ation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)			
-	0.00	0	0	0	0			
51	3.90	20,680	26,884	26,884	20,704			
51	4.00	20,690	2,068	28,952	20,756			
51	6.00	28,290	48,782	77,735	28,436			
51	8.00	42,760	70,554	148,288	42,967			
#	Routing	Invert	Outlet Devices					
1	Primary	513.90'	2.0' long x 0.5' breadth Broad-Crested Rectangular Weir					
	Head (feet) 0.20 0.40 0.60 0.80 1.00 Coef. (English) 2.80 2.92 3.08 3.30 3.32							
2	Primary							
3	Primary	515.32'	5.32' 175.0 deg x 10.0' long Sharp-Crested Vee/Trap Weir C= 2.46					
						·		

Primary OutFlow Max=58.21 cfs @ 13.82 hrs HW=515.92' TW=510.03' (Dynamic Tailwater)

1=Broad-Crested Rectangular Weir (Weir Controls 19.02 cfs @ 4.7 fps)

-2=Sharp-Crested Vee/Trap Weir (Weir Controls 9.51 cfs @ 2.6 fps)

-3=Sharp-Crested Vee/Trap Weir (Weir Controls 29.68 cfs @ 2.1 fps)

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#### Pond p19:

Wetland Geometry to be confirmed by survey Based off aerial topo, and assumed topo contour

Inflow Area =	15.520 ac, Inflow Depth = $1.30$ "	for 10-yr event
Inflow =	10.40 cfs @ 12.62 hrs, Volume=	1.683 af
Outflow =	5.85 cfs @ 13.10 hrs, Volume=	1.681 af, Atten= 44%, Lag= 29.1 min
Primary =	5.85 cfs @ 13.10 hrs, Volume=	1.681 af
Secondary =	0.00 cfs @ 0.00 hrs, Volume=	0.000 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 972.00' Surf.Area= 86,000 sf Storage= 57,333 cf Peak Elev= 972.15' @ 13.10 hrs Surf.Area= 90,219 sf Storage= 74,507 cf (17,174 cf above start) Plug-Flow detention time= 607.9 min calculated for 0.365 af (22% of inflow) Center-of-Mass det. time= 74.4 min (981.7 - 907.3)

_	#	Invert	Avail.St	orage Storage	Description		
	1	970.00'	282,	329 cf Custom	Stage Data (Conic	) Listed below	
_	Eleva (f	ation eet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet		Wet.Area (sq-ft)	
	970	0.00	0	(	) 0	0	
	972	2.00	86,000	57,333	3 57,333	86,006	
	974	4.00	141,270	224,996	5 282,329	141,327	
_	#	Routing	Invert	Outlet Devices			
	1	Secondary	973.60'	178.0 deg x 51.	0' long Sharp-Cres	ted Vee/Trap Weir	C= 2.46
	~	<b>D</b> '					147 -

2 Primary 972.00' **35.0' long x 0.5' breadth Broad-Crested Rectangular Weir** Head (feet) 0.20 0.40 0.60 0.80 1.00 Coef. (English) 2.80 2.92 3.08 3.30 3.32

Primary OutFlow Max=5.85 cfs @ 13.10 hrs HW=972.15' TW=970.14' (Dynamic Tailwater) ←2=Broad-Crested Rectangular Weir (Weir Controls 5.85 cfs @ 1.1 fps)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=972.00' TW=973.60' (Dynamic Tailwater) -1=Sharp-Crested Vee/Trap Weir (Controls 0.00 cfs)

## Pond p20:

Field Note #50 Spring Fed Pond Geometry to be confirmed by surveyed

 Inflow Area =
 241.484 ac, Inflow Depth =
 1.32" for 10-yr event

 Inflow =
 32.37 cfs @
 16.15 hrs, Volume=
 26.596 af

 Outflow =
 32.22 cfs @
 16.29 hrs, Volume=
 25.771 af, Atten= 0%, Lag= 8.3 min

 Primary =
 32.22 cfs @
 16.29 hrs, Volume=
 25.771 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

Existing Conditions_10454-01	Type III 24-hr 10-yr Rainfall=5.00"
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Starting Elev= 505.10' Surf.Area= 89,370 sf Storage= 138,524 cf Peak Elev= 506.18' @ 16.29 hrs Surf.Area= 90,255 sf Storage= 235,644 cf (97,120 cf above start) Plug-Flow detention time= 333.6 min calculated for 22.586 af (85% of inflow) Center-of-Mass det. time= 85.1 min (1,420.2 - 1,335.1)

#	Invert	Avail.S	torage Storage Description				
1	502.00'	615,	682 cf Custom Stage Data (Prismatic) Listed below				
-	- C	0					
	ation	Surf.Area					
	(feet)	(sq-ft)	(cubic-feet) (cubic-feet)				
50	02.00	0	0 0				
50	05.10	89,370	138,524 138,524				
50	06.00	89,380	80,437 218,961				
50	08.00	99,280	188,660 407,621				
5	10.00	108,781	208,061 615,682				
#	Routing	Invert	Outlet Devices				
1	Primary	505.10'	3.0' long x 1.5' breadth Broad-Crested Rectangular Weir				
	-		Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50				
			3.00				
			Coef. (English) 2.62 2.64 2.64 2.68 2.75 2.86 2.92 3.07 3.07 3.03 3.28				
			3.32				
2	Primary	506.20'	6.5' long x 1.5' breadth Broad-Crested Rectangular Weir				
	j		Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50				
			3.00				
			Coef. (English) 2.62 2.64 2.64 2.68 2.75 2.86 2.92 3.07 3.07 3.03 3.28				
			3.32				
3	Primary	506.00'	<b>176.0 deg x 97.0' long Sharp-Crested Vee/Trap Weir</b> C= 2.46				
5	i iiiiai y	000.00					
Prim	ary OutFlow	Max-32	22 cfs @ 16.29 hrs HW=506.18' TW=505.28' (Dynamic Tailwater)				
	-1=Broad-Crested Rectangular Weir (Weir Controls 9.11 cfs @ 2.8 fps)						

-2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

-3=Sharp-Crested Vee/Trap Weir (Weir Controls 23.11 cfs @ 1.3 fps)

#### Pond p21:

Inflow Area =	489.305 ac, Inflow Depth = 1.65"	for 10-yr event
Inflow =	269.39 cfs @ 12.46 hrs, Volume=	67.462 af
Outflow =	26.91 cfs @ 20.81 hrs, Volume=	61.738 af, Atten= 90%, Lag= 500.5 min
Primary =	26.91 cfs @ 20.81 hrs, Volume=	61.738 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 484.82' @ 20.81 hrs Surf.Area= 808,155 sf Storage= 1,468,971 cf Plug-Flow detention time= 689.9 min calculated for 61.725 af (91% of inflow) Center-of-Mass det. time= 582.3 min (1,663.5 - 1,081.2)

#	Invert	Avail.Storage	Storage Description
1	480.40'	5,244,885 cf	Custom Stage Data (Conic) Listed below

## Existing Conditions\_10454-01

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Elevation	Surf.Area	Inc.Store	Cum.Store	Wet.Area
(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)	(sq-ft)
480.40	0	0	0	0
482.00	202,230	107,856	107,856	202,234
484.00	485,198	667,114	774,970	485,231
486.00	1,275,481	1,698,237	2,473,208	1,275,541
488.00	1,499,208	2,771,678	5,244,885	1,499,423
# Routing	Invert O	utlet Devices		
	400 401 04			

1 Primary 480.40' **30.0" x 70.0' long Culvert** CMP, projecting, no headwall, Ke= 0.900 Outlet Invert= 480.40' S= 0.0000 '/' n= 0.024 Cc= 0.900

Primary OutFlow Max=26.91 cfs @ 20.81 hrs HW=484.82' TW=0.00' (Dynamic Tailwater) ←1=Culvert (Barrel Controls 26.91 cfs @ 5.5 fps)

### Pond p22:

Field Note #54 Golf Pond Geometry to be confirmed by survey

Inflow Are	a =	97.943 ac, Inflow Depth = 1.86" for 10-yr event
Inflow	=	102.22 cfs @ 12.46 hrs, Volume= 15.160 af
Outflow	=	101.85 cfs @ 12.49 hrs, Volume= 14.861 af, Atten= 0%, Lag= 1.8 min
Primary	=	101.85 cfs @ 12.49 hrs, Volume= 14.861 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 498.10' Surf.Area= 6,520 sf Storage= 10,106 cf Peak Elev= 501.93' @ 12.49 hrs Surf.Area= 11,424 sf Storage= 43,521 cf (33,415 cf above start) Plug-Flow detention time= 41.1 min calculated for 14.626 af (96% of inflow) Center-of-Mass det. time= 17.7 min (904.5 - 886.8)

#Inv	vert Avail.S	torage Storage D	Description					
1 495	.00' 143	770 cf Custom S	Stage Data (Prism	atic) Listed below				
Elevation	Surf.Area		Cum.Store					
(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)					
495.00	0	0	0					
498.10	6,520	10,106	10,106					
500.00	8,390	14,164	24,270					
502.00	11,530	19,920	44,190					
504.00	14,530	26,060	70,250					
506.00	18,340	32,870	103,120					
508.00	22,310	40,650	143,770					
# Routin	g Invert	Outlet Devices						
1 Primar	y 499.75'	18.0" x 21.0' long	g Culvert CMP, p	projecting, no headwall, Ke= 0.900				
	-	Outlet Invert= 499.75' S= 0.0000 '/' n= 0.024 Cc= 0.900						
2 Primar	y 500.50'	1.0' long x 15.0' breadth Broad-Crested Rectangular Weir						
	-	Head (feet) 0.20	Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60					
		Coef. (English) 2	.68 2.70 2.70 2.6	64 2.63 2.64 2.64 2.63				

Existing Conditions_10454-01	Type III 24-hr 10-yr Rainfall=5.00"
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20.0' long x 13.5' breadth Broad-Crested Rectangular Weir 3 Primary 500.50' Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.62 2.66 2.70 2.66 2.65 2.66 2.65 2.63

Primary OutFlow Max=101.84 cfs @ 12.49 hrs HW=501.93' TW=500.34' (Dynamic Tailwater) -1=Culvert (Barrel Controls 6.54 cfs @ 3.7 fps) -2=Broad-Crested Rectangular Weir (Weir Controls 4.52 cfs @ 3.2 fps)

-3=Broad-Crested Rectangular Weir (Weir Controls 90.78 cfs @ 3.2 fps)

### Pond p23:

Inflow Are	ea =	41.587 ac, Inflow De	oth = 2.20"	for 10-yr event		
Inflow	=	47.82 cfs @ 12.69 hi	s, Volume=	7.617 af		
Outflow	=	47.78 cfs @ 12.69 hr	s, Volume=	6.699 af,	Atten= 0%,	Lag= 0.2 min
Primary	=	47.78 cfs @ 12.69 hi	s, Volume=	6.699 af		-

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 507.89' @ 12.69 hrs Surf.Area= 19,006 sf Storage= 42,917 cf Plug-Flow detention time= 80.3 min calculated for 6.697 af (88% of inflow) Center-of-Mass det. time= 24.0 min (904.6 - 880.6)

#	Invert	Avail.St	orage	Storage De	escription		
1	503.50'	100,	303 cf	<b>Custom St</b>	age Data (Prisma	atic) Listed below	
Elevat (fe	tion eet)	Surf.Area (sq-ft)	(c	Inc.Store ubic-feet)	Cum.Store (cubic-feet)		
503	.50	0		0	0		
506	.00	11,170		13,963	13,963		
508	.00	19,460		30,630	44,593		
510	.00	36,250		55,710	100,303		
	Routing	Invert	Outlet [				
1 F	Primary	507.70'	178.0 d	leg x 178.0	' long Sharp-Cre	sted Vee/Trap Weir	C= 2.46

Primary OutFlow Max=47.78 cfs @ 12.69 hrs HW=507.89' TW=507.19' (Dynamic Tailwater) **1=Sharp-Crested Vee/Trap Weir** (Weir Controls 47.78 cfs @ 1.3 fps)

## Pond zDP1: Design Point 1

Field note #10. Culvert dimensions to be confirmed by survey.

Inflow Area	=	26.659 ac, In	flow Depth = 1.92"	for 10-yr event	
Inflow =	=	29.17 cfs @	12.50 hrs, Volume=	4.274 af	
Outflow =	=	29.17 cfs @	12.50 hrs, Volume=	4.274 af,	Atten= 0%, Lag= 0.2 min
Primary =	=	29.17 cfs @	12.50 hrs, Volume=	4.274 af	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 722.18' @ 12.50 hrs Surf.Area= 112 sf Storage= 93 cf Flood Elev= 727.00' Surf.Area= 1,105 sf Storage= 2,619 cf Plug-Flow detention time= 0.1 min calculated for 4.273 af (100% of inflow)

Invert

#

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Avail Storage Storage Description

<u> </u>	Invent	Avail.00	olage ololage De	sonption		
1	720.10'	3,7	706 cf Custom St	age Data (Conic)	Listed below	
Elevat (fe	tion eet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
720	).10	0	0	0	0	
722	2.00	90	57	57	96	
724	1.00	340	403	460	364	
726	6.00	760	1,072	1,533	815	
728	3.00	1,450	2,173	3,706	1,543	
#	Routing	Invert	Outlet Devices			
1	Primary		<b>42.0" x 120.0' lon</b> Outlet Invert= 700.			-
2	Primary	727.00'	155.0 deg Sharp-C	Crested Vee/Trap	<b>Weir</b> C= 2.47	

Center-of-Mass det. time= 0.1 min (879.8 - 879.7)

Primary OutFlow Max=29.17 cfs @ 12.50 hrs HW=722.18' TW=686.58' (Dynamic Tailwater) -1=Culvert (Inlet Controls 29.17 cfs @ 4.9 fps) -2=Sharp-Crested Vee/Trap Weir (Controls 0.00 cfs)

Pond zDP2: Design Point 2

Field Note #15 Culvert dimensions to be confirmed by survey Overflow to ditch is currently discarded... We may have to model that area...

Inflow Area = Inflow = Outflow =	97.712 ac, Inflow Depth = 1.65" 69.51 cfs @ 12.87 hrs, Volume= 69.49 cfs @ 12.88 hrs, Volume= 35 15 cfs @ 12.88 hrs, Volume=	13.464 af 13.464 af, Atten= 0%, Lag= 0.4 min
Discarded = Primary =	35.15 cfs @ 12.88 hrs, Volume= 34.34 cfs @ 12.88 hrs, Volume=	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 625.75' @ 12.88 hrs Surf.Area= 1,338 sf Storage= 3,072 cf Flood Elev= 624.50' Surf.Area= 925 sf Storage= 1,728 cf Plug-Flow detention time= 0.5 min calculated for 13.461 af (100% of inflow) Center-of-Mass det. time= 0.5 min (912.7 - 912.2)

#	Invert	Avail.Storage	e Storage Des	scription	
1	619.60'	7,280 c	f Custom Sta	ige Data (Conic) L	isted below
Elevati (fe	ion et)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft <u>)</u>
619.	.60	0	0	0	0
620.	.00	10	1	1	10
622.	.00	260	214	215	269
624.	.00	760	976	1,192	793
626.	.00	1,420	2,146	3,338	1,492
628.	.00	2,580	3,943	7,280	2,694

#	Routing	Invert	Outlet Devices
1	Primary	619.60'	24.0" x 150.0' long Culvert RCP, end-section conforming to fill, Ke= 0.500
			Outlet Invert= 608.00' S= 0.0773 '/' n= 0.012 Cc= 0.900
2	Discarded	624.50'	166.0 deg Sharp-Crested Vee/Trap Weir C= 2.46

Discarded OutFlow Max=35.15 cfs @ 12.88 hrs HW=625.75' (Free Discharge) ←2=Sharp-Crested Vee/Trap Weir (Weir Controls 35.15 cfs @ 2.8 fps)

**Primary OutFlow** Max=34.34 cfs @ 12.88 hrs HW=625.75' TW=607.67' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 34.34 cfs @ 10.9 fps)

## Pond zDP3: Design Point 3

Inflow Are	a =	212.742 ac, Inflo	ow Depth = 19.33"	for	10-yr event		
Inflow	=	195.74 cfs @ 12	2.51 hrs, Volume=		342.644 af		
Primary	=	195.74 cfs @ 12	2.51 hrs, Volume=		342.644 af,	Atten= 0%, Lag= 0.0	0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

### Pond zDP4: Design Point 4

Inflow Area	=	489.305 ac, Inflow Depth = 1.51"	for 10-yr event
Inflow	=	26.91 cfs @ 20.81 hrs, Volume=	61.738 af
Primary	=	26.91 cfs @ 20.81 hrs, Volume=	61.738 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

## Pond zDP5: Design Point 5

Inflow Area =	28.325  ac,  Inflow Depth = 2.04"	for 10-yr event
Inflow =	37.09 cfs @ 12.45 hrs, Volume=	4.807 af
Primary =	37.09 cfs @ 12.45 hrs, Volume=	4.807 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

# Pre-Development Conditions 25 year 24 hour Storm Event Model Computations

Existing Conditions_10454-01Type III 24-hr 25-yrRainfall=5.90"Prepared by The Chazen CompaniesPage 100HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems4/10/2006 2:33:41 PM
Subcatchment s01:
Runoff = 16.08 cfs @ 12.60 hrs, Volume= 2.435 af, Depth= 2.54"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90"
Area (ac) CN Description 11.485 68
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
42.8 Direct Entry,
Subcatchment s02:
Runoff = 98.45 cfs @ 12.87 hrs, Volume= 18.556 af, Depth= 2.28"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90"
Area (ac) CN Description 97.712 65
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
61.3 Direct Entry,
Subcatchment s03:
Runoff = 26.67 cfs @ 12.41 hrs, Volume= 3.332 af, Depth= 2.63"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90"
Area (ac) CN Description
15.174 69
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
28.8 Direct Entry,
Subcatchment s04:
Runoff = 29.21 cfs @ 12.10 hrs, Volume= 2.166 af, Depth= 2.28"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90"

5 =	e III 24-hr 25-yr Rainfall=5.90"
Prepared by The Chazen Companies HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems	Page 101 4/10/2006 2:33:41 PM
Area (ac) CN Description 11.403 65	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
6.5 Direct Entry,	
Subcatchment s05:	
Dunoff 22.90 of @ 12.25 bro Valumo 2.412 of Do	oth 1.04"
Runoff = 22.89 cfs @ 12.25 hrs, Volume= 2.413 af, De	pm= 1.94
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= Type III 24-hr 25-yr Rainfall=5.90"	0.01 hrs
Area (ac) CN Description	
14.935 61	
Tc Length Slope Velocity Capacity Description	
(min) (feet) (ft/ft) (ft/sec) (cfs)	
17.3 Direct Entry,	
Subcatchment s06:	
Runoff = 14.52 cfs @ 12.25 hrs, Volume= 1.518 af, De	pth= 2.02"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= Type III 24-hr 25-yr Rainfall=5.90"	0.01 hrs
Area (ac) CN Description	
9.007 62	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
17.3 Direct Entry,	
Subcatchment s06(OW): s06 Open Wa	ator
Runoff = 3.03 cfs @ 12.00 hrs, Volume= 0.210 af, De	pth= 5.90"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= Type III 24-hr 25-yr Rainfall=5.90"	0.01 hrs
Area (ac) CN Description	
0.428 100	

Existing Conditions_10454-01Type III 24-hr 25-yrRainfall=5.90"Prepared by The Chazen CompaniesPage 102HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems4/10/2006 2:33:41 PM
Subcatchment s07:
Runoff = 13.14 cfs @ 12.20 hrs, Volume= 1.244 af, Depth= 2.19"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90"
Area (ac) CN Description
6.811 64
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
13.9 Direct Entry,
Subcatchment s07(OW): s07 Open Water
Runoff = 3.58 cfs @ 12.00 hrs, Volume= 0.249 af, Depth= 5.90"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90"
Area (ac) CN Description
0.506 100
Subcatchment s08:
Runoff = 37.56 cfs @ 12.33 hrs, Volume= 4.480 af, Depth= 1.69"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90"
Area (ac) CN Description
31.719 58
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
21.8 Direct Entry,
Subcatchment s09:
Runoff = 16.02 cfs @ 12.23 hrs, Volume= 1.605 af, Depth= 2.28"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90"
Area (ac) CN Description 8.452 65

Existing Conditions_10454-01Type III 24-hr 25-yrPrepared by The Chazen CompaniesPage 11 - 25 - yr	
HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems 4/10/2006 2:33	age 103 <u>3:41 PM</u>
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
16.4 <b>Direct Entry</b> ,	
Subcatchment s10:	
Runoff = 13.41 cfs @ 12.40 hrs, Volume= 1.663 af, Depth= 2.45"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90"	
Area (ac) CN Description	
8.130 67	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
27.9Direct Entry,	
Subcatchment s10(OW): s10 Open Water	
Runoff = 5.88 cfs @ 12.00 hrs, Volume= 0.408 af, Depth= 5.90"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90"	
Area (ac) CN Description 0.830 100	
Subcatchment s11:	
Runoff = 4.81 cfs @ 12.28 hrs, Volume= 0.519 af, Depth= 2.63"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90"	
Area (ac) CN Description	
2.364 69	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
20.5 Direct Entry,	
Subcatchment s11(IC): s11 Imp. Cover	
Runoff = 15.16 cfs @ 12.04 hrs, Volume= 1.103 af, Depth= 5.66"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90"	

Existing Conditions_10454-01	Type III 24-hr 25-yr Rainfall=5.90"
Prepared by The Chazen Companies HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Syste	Page 104 ems 4/10/2006 2:33:41 PM
Area (ap) CN Department	
Area (ac) CN Description 2.338 98	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
2.8 Direct Entry,	
Subcatchment s12:	
Runoff = 4.67 cfs @ 12.65 hrs, Volume= 0.781 a	f, Depth= 1.46"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs Type III 24-hr 25-yr Rainfall=5.90"	s, dt= 0.01 hrs
Area (ac) CN Description	
6.420 55	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
42.0 Direct Entry,	
Subcatchment s13:	
Runoff = 0.48 cfs @ 12.14 hrs, Volume= 0.043 a	f, Depth= 1.46"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs Type III 24-hr 25-yr Rainfall=5.90"	s, dt= 0.01 hrs
Area (ac) CN Description	
0.350 55	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
8.6 Direct Entry,	
Subcatchment s14:	
Runoff = 26.14 cfs @ 12.40 hrs, Volume= 3.301 a	f, Depth= 2.19"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs Type III 24-hr 25-yr Rainfall=5.90"	s, dt= 0.01 hrs
Area (ac) CN Description 18.066 64	
Tc Length Slope Velocity Capacity Description	
(min) (feet) (ft/ft) (ft/sec) (cfs) 28.1 Direct Entry,	

Existing Conditions_10454-01Type III 24-hr 25-yrRainfall=5.90'Prepared by The Chazen CompaniesPage 105HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems4/10/2006 2:33:41 PM		
Subcatchment s14(IC): s14 Imp. Cover		
Runoff = 15.71 cfs @ 12.03 hrs, Volume= 1.123 af, Depth= 5.66"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90"		
Area (ac) CN Description 2.380 98		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
2.3Direct Entry,		
Subcatchment s14(OW): s14 Open Water		
Runoff = 3.67 cfs @ 12.00 hrs, Volume= 0.255 af, Depth= 5.90"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90"		
Area (ac) CN Description 0.518 100		
Subcatchment s15:		
Runoff = 0.92 cfs @ 12.21 hrs, Volume= 0.104 af, Depth= 1.16"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90"		
Area (ac) CN Description		
1.068 51		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
13.1Direct Entry,		
Subcatchment s16:		
Runoff = 118.67 cfs @ 12.32 hrs, Volume= 13.408 af, Depth= 2.37"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90"		
Area (ac) CN Description		
67.994 66		

Existing Conditions_10454-01 Type III 24-hr 25-yr Rainfall=5.	
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Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
(min)         (feet)         (ft/ft)         (ft/sec)         (cfs)           22.3         Direct Entry,	
Subcatchment s16(IC): s16 Imp.Cover	
Runoff = 16.98 cfs @ 12.04 hrs, Volume= 1.240 af, Depth= 5.66"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90"	
Area (ac) CN Description	
2.629 98	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
2.9 Direct Entry,	
Subcatchment s16(OW): s16 Open Water	
Runoff = 37.90 cfs @ 12.00 hrs, Volume= 2.631 af, Depth= 5.90"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90"	
Area (ac) CN Description	
5.351 100	
Subcatchment s17:	
Runoff = 75.65 cfs @ 13.77 hrs, Volume= 22.841 af, Depth= 2.37"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90"	
Area (ac) CN Description	
115.827 66	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
125.2 Direct Entry,	
Subcatchment s17(OW): s17 Open Water	
Runoff = 1.16 cfs @ 12.00 hrs, Volume= 0.081 af, Depth= 5.90"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs	

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90"

Existing Conditions_10454-01Type III 24-hr 25-yr Rainfall=5.90"Prepared by The Chazen CompaniesPage 107
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Area (ac) CN Description
Area (ac) CN Description 0.164 100
Subcatchment s18:
Runoff = 39.64 cfs @ 12.34 hrs, Volume= 4.634 af, Depth= 2.45"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90"
Area (ac) CN Description
22.654 67
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
24.4Direct Entry,
Subcatchment s18(OW): s18 Open Water
Runoff = 3.34 cfs @ 12.00 hrs, Volume= 0.232 af, Depth= 5.90"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90"
Area (ac) CN Description
0.472 100
Subcatchment s19:
Runoff = 15.56 cfs @ 12.61 hrs, Volume= 2.401 af, Depth= 1.86"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90"
Area (ac) CN Description
15.520 60
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
40.4Direct Entry,
Subcatchment s20:
Runoff = 30.46 cfs @ 12.47 hrs, Volume= 4.096 af, Depth= 2.63"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

Type III 24-hr 25-yr Rainfall=5.90"

HydroCAD® 7.00         s/n 000927         © 1986-2003 Applied Microcomputer Systems         4/10/2006         2:33:42 PM           Area (ac)         CN         Description         18.655         69           Tc         Length         Slope         Velocity         Capacity         Description           (min)         (feet)         (ft/ft)         (ft/sec)         (cfs)         Direct Entry,           Subcatchment s20(OW): s20 Open Water           Runoff         =         13.94 cfs         @         12.00 hrs, Volume=         0.968 af, Depth=         5.90"           Runoff         =         13.94 cfs         @         12.00 hrs, Volume=         0.968 af, Depth=         5.90"           Runoff         =         13.94 cfs         @         12.00 hrs, Volume=         0.968 af, Depth=         5.90"           Runoff         Subcatchment s20(OW): s20 Open Water         Subcatchment s21:         No         Subcatchment s21:           Runoff         =         168.20 cfs         @         12.38 hrs, Volume=         20.366 af, Depth=         2.54"           Runoff         =         168.20 cfs         @         12.38 hrs, Volume=         20.366 af, Depth=         2.54"           Runoff         =         168.20 cfs         D	Existing Conditions_10454-01	Type III 24-hr 25-yr Rainfall=5.90"
Area (ac)       CN       Description         18.655       69         Tc       Length       Slope       Velocity       Capacity       Description         (min)       (teet)       (tt/tt)       (tt/sec)       (cfs)       Direct Entry,         Subcatchment s20(OW):       s20 Open Water         Runoff       =       13.94 cfs @       12.00 hrs, Volume=       0.968 af, Depth=       5.90"         Runoff by SCS TR-20 method, UH=SCS, Time Span=       0.00-48.00 hrs, dt=       0.01 hrs         Type III 24-hr 25-yr Rainfall=5.90"       Area (ac)       CN       Description         1.968       100       Subcatchment s21:         Runoff       =       168.20 cfs @       12.38 hrs, Volume=       20.366 af, Depth=       2.54"         Runoff by SCS TR-20 method, UH=SCS, Time Span=       0.00-48.00 hrs, dt=       0.01 hrs       Type III 24-hr 25-yr       Rainfall=5.90"         Area (ac)       CN       Description       96.056       68       Tc       Length       Slope       Velocity       Capacity       Description         96.056       68       Tc       Length       Slope       Velocity       Capacity       Description         (min)       (feet)       (ft/ft)       (ft/sec)	Prepared by The Chazen Companies HvdroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microco	Page 108 2006 2:33:42 PM
18.655       69         Tc       Length       Slope       Velocity       Capacity       Description         (min)       (feet)       (ft/ft)       (ft/sec)       (cfs)         33.9       Direct Entry,         Subcatchment s20(OW): s20 Open Water         Runoff       =       13.94 cfs @       12.00 hrs, Volume=       0.968 af, Depth=       5.90"         Runoff by SCS TR-20 method, UH=SCS, Time Span=       0.00-48.00 hrs, dt=       0.01 hrs         Type III 24-hr 25-yr       Rainfall=5.90"         Area (ac)       CN       Description         1.968       100       Subcatchment s21:         Runoff       =       168.20 cfs @       12.38 hrs, Volume=       20.366 af, Depth=       2.54"         Runoff by SCS TR-20 method, UH=SCS, Time Span=       0.00-48.00 hrs, dt=       0.01 hrs       Type III 24-hr 25-yr       Rainfall=5.90"         Area (ac)       CN       Description		<u>, 10, 2000 21001 12 1 11</u>
$\begin{array}{rrrrrrrrrrrrrrrrrrrrrrrrrrrrrrrrrrrr$		
$\begin{array}{rcrc} (min) & (feet) & (ft/ft) & (ft/sec) & (cfs) \\ \hline 33.9 & Direct Entry, \\ \hline Subcatchment s20(OW): s20 Open Water \\ \hline Runoff = 13.94 cfs @ 12.00 hrs, Volume= 0.968 af, Depth= 5.90" \\ \hline Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs \\ \hline Type III 24-hr 25-yr Rainfall=5.90" \\ \hline Area (ac) & CN & Description \\ \hline 1.968 & 100 \\ \hline \\ Subcatchment s21: \\ \hline \\ Runoff = 168.20 cfs @ 12.38 hrs, Volume= 20.366 af, Depth= 2.54" \\ \hline \\ Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs \\ \hline \\ Type III 24-hr 25-yr Rainfall=5.90" \\ \hline \\ \hline \\ Area (ac) & CN & Description \\ \hline \\ 96.056 & 68 \\ \hline \\ Tc & Length & Slope & Velocity & Capacity & Description \\ \hline \\ (ftet) & (ft/ft) & (ft/sec) & (cfs) \\ \hline \\ 26.7 & Direct Entry, \\ \hline \\ Subcatchment s21(OW): \\ \hline \\ Runoff = 86.66 cfs @ 12.00 hrs, Volume= 6.016 af, Depth= 5.90" \\ \hline \\ Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs \\ \hline \\ Type III 24-hr 25-yr Rainfall=5.90" \\ \hline \\ \hline \\ Area (ac) & CN & Description \\ \hline \\ 96.056 & 68 \\ \hline \\ \hline \\ Tc & Length & Slope & Velocity & Capacity & Description \\ \hline \\ \hline \\ Runoff = 86.66 cfs @ 12.00 hrs, Volume= 6.016 af, Depth= 5.90" \\ \hline \\ Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs \\ \hline \\ Type III 24-hr 25-yr Rainfall=5.90" \\ \hline \\ \hline \\ \hline \\ \hline \\ Area (ac) & CN & Description \\ \hline \\ \hline \\ \hline \\ 12.235 & 100 \\ \hline \end{array}$	18.655 69	
Subcatchment s20(OW): s20 Open Water         Runoff       =       13.94 cfs @       12.00 hrs, Volume=       0.968 af, Depth= 5.90"         Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs         Type III 24-hr 25-yr Rainfall=5.90"         Area (ac) CN Description         1.968       100         Subcatchment s21:         Runoff       =       168.20 cfs @       12.38 hrs, Volume=       20.366 af, Depth= 2.54"         Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs         Type III 24-hr 25-yr Rainfall=5.90"         Area (ac) CN Description         (min) (feet)       Glope Velocity Capacity Description         (min) (feet)       Glope Velocity Capacity Description         (min) (feet)       Globes 12.00 hrs, Volume=       6.016 af, Depth= 5.90"         Runoff =       86.66 cfs @       12.00 hrs, Volume=       6.016 af, Depth= 5.90"         Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs         Type III 24-hr 25-yr Rainfall=5.90"         Area (ac) CN Description         Carea (ac) CN Description         Carea (ac) CN Description         Area (ac) CN Description		otion
$ \begin{array}{rcl} \operatorname{Runoff} &=& 13.94  \mathrm{cfs} @ 12.00  \mathrm{hrs},  \mathrm{Volume}= & 0.968  \mathrm{af},  \mathrm{Depth}= 5.90'' \\ \hline & \mathrm{Runoff}  \mathrm{by}  \mathrm{SCS}  \mathrm{TR-20}  \mathrm{method},  \mathrm{UH}=\mathrm{SCS},  \mathrm{Time}  \mathrm{Span}=  0.00-48.00  \mathrm{hrs},  \mathrm{dt}=  0.01  \mathrm{hrs} \\ \hline & \mathrm{Type}  \mathrm{III}  24 + \mathrm{hr}  25 + \mathrm{yr}  \mathrm{Rainfall}= 5.90'' \\ \hline & \underline{\mathrm{Area}}  (\mathrm{ac})  \mathrm{CN}   \mathrm{Description} \\ \hline & 1.968  100 \\ \hline & \mathbf{Subcatchment}  \mathbf{s21:} \\ \hline & \mathrm{Runoff}  =  168.20  \mathrm{cfs} @ 12.38  \mathrm{hrs},  \mathrm{Volume}=  20.366  \mathrm{af},  \mathrm{Depth}=  2.54'' \\ \hline & \mathrm{Runoff}  \mathrm{by}  \mathrm{SCS}  \mathrm{TR-20}  \mathrm{method},  \mathrm{UH}=\mathrm{SCS},  \mathrm{Time}  \mathrm{Span}=  0.00-48.00  \mathrm{hrs},  \mathrm{dt}=  0.01  \mathrm{hrs} \\ \hline & \mathrm{Type}  \mathrm{III}  24 + \mathrm{hr}  25 + \mathrm{yr}  \mathrm{Rainfall}= 5.90'' \\ \hline & \underline{\mathrm{Area}}  (\mathrm{ac})  \mathrm{CN}   \mathrm{Description} \\ \hline & 96.056  68 \\ \hline & \mathrm{Tc}   \mathrm{Length}   \mathrm{Slope}   \mathrm{Velocity}   \mathrm{Capacity}   \mathrm{Description} \\ \hline & \mathrm{glocatchment}  \mathbf{s21}(\mathrm{OW}): \\ \hline & \mathrm{CR}  \mathrm{unoff}  =  86.66  \mathrm{cfs}  @  12.00  \mathrm{hrs},  \mathrm{Volume}=  6.016  \mathrm{af},  \mathrm{Depth}=  5.90'' \\ \hline & \mathrm{Runoff}  \mathrm{by}  \mathrm{SCS}  \mathrm{TR-20}  \mathrm{method},  \mathrm{UH}=\mathrm{SCS},  \mathrm{Time}  \mathrm{Span}=  0.00-48.00  \mathrm{hrs},  \mathrm{dt}=  0.01  \mathrm{hrs} \\ \hline & \mathrm{Tppe}  \mathrm{III}  24 + \mathrm{hr}  25 + \mathrm{yr}   \mathrm{Rainfall}=  5.90'' \\ \hline & \mathrm{Runoff}  \mathrm{by}  \mathrm{SCS}  \mathrm{TR-20}  \mathrm{method},  \mathrm{UH}=\mathrm{SCS},  \mathrm{Time}  \mathrm{Span}=  0.00-48.00  \mathrm{hrs},  \mathrm{dt}=  0.01  \mathrm{hrs} \\ \ & \mathrm{Tppe}  \mathrm{III}  24 + \mathrm{hr}  25 + \mathrm{yr}   \mathrm{Rainfall}=  5.90'' \\ \hline & \mathrm{Runoff}  \mathrm{by}  \mathrm{SCS}  \mathrm{TR-20}  \mathrm{method},  \mathrm{UH}=\mathrm{SCS},  \mathrm{Time}  \mathrm{Span}=  0.00-48.00  \mathrm{hrs},  \mathrm{dt}=  0.01  \mathrm{hrs} \\ \ & \mathrm{Tppe}  \mathrm{III}  24 + \mathrm{hr}  25 + \mathrm{yr}   \mathrm{Rainfall}=  5.90'' \\ \hline & \mathrm{Area}  \mathrm{(ac)}  \ \mathrm{CN}   \mathrm{Description} \\ \hline & 12.235  100 \\ \hline \end{array} \right$	33.9 <b>Direct</b>	Entry,
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90" Area (ac) CN Description 1.968 100 Subcatchment s21: Runoff = 168.20 cfs @ 12.38 hrs, Volume= 20.366 af, Depth= 2.54" Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90" Area (ac) CN Description 96.056 68 Tc Length Slope Velocity Capacity Description (min) (feet) (tt/ft) (tt/sec) (cfs) 26.7 Direct Entry, Subcatchment s21(OW): Runoff = 86.66 cfs @ 12.00 hrs, Volume= 6.016 af, Depth= 5.90" Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90" Area (ac) CN Description 12.235 100	Subcatchment s20(OW	/): s20 Open Water
Type III 24-hr 25-yr Rainfall=5.90"         Area (ac)       CN       Description         1.968       100       Subcatchment s21:         Runoff       =       168.20 cfs @       12.38 hrs, Volume=       20.366 af, Depth= 2.54"         Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs       Type III 24-hr 25-yr Rainfall=5.90"         Area (ac)       CN       Description         96.056       68       (cfs)         Tc       Length       Slope       Velocity       Capacity       Description         26.7       Direct Entry,       Subcatchment s21(OW):       Subcatchment s21(OW):         Runoff       =       86.66 cfs @       12.00 hrs, Volume=       6.016 af, Depth= 5.90"         Runoff       =       86.66 cfs @       12.00 hrs, Volume=       6.016 af, Depth= 5.90"         Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs       Type III 24-hr 25-yr Rainfall=5.90"         Area (ac)       CN       Description         12.235       100	Runoff = 13.94 cfs @ 12.00 hrs, Volume=	0.968 af, Depth= 5.90"
1.968       100         Subcatchment s21:         Runoff       =       168.20 cfs @       12.38 hrs, Volume=       20.366 af, Depth=       2.54"         Runoff by SCS TR-20 method, UH=SCS, Time Span=       0.00-48.00 hrs, dt=       0.01 hrs         Type III 24-hr 25-yr       Rainfall=5.90"         Area (ac)       CN       Description         96.056       68         Tc       Length       Slope       Velocity       Capacity       Description         (min)       (feet)       (ft/ft)       (ft/sec)       (cfs)         26.7       Direct Entry,         Subcatchment s21(OW):         Runoff       =       86.66 cfs @       12.00 hrs, Volume=       6.016 af, Depth=       5.90"         Runoff by SCS TR-20 method, UH=SCS, Time Span=       0.00-48.00 hrs, dt=       0.01 hrs         Type III 24-hr 25-yr       Rainfall=5.90"         Area (ac)       CN       Description         12.235       100		.00-48.00 hrs, dt= 0.01 hrs
$\begin{array}{rcl} \operatorname{Runoff} &=& 168.20 \ \mathrm{cfs} @ 12.38 \ \mathrm{hrs}, \ \mathrm{Volume} &=& 20.366 \ \mathrm{af}, \ \mathrm{Depth} &=& 2.54" \\ \mbox{Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 \ \mathrm{hrs}, \ \mathrm{dt} &=& 0.01 \ \mathrm{hrs} \\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ $		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90" Area (ac) CN Description 96.056 68 Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs) 26.7 Direct Entry, Subcatchment s21(OW): Runoff = 86.66 cfs @ 12.00 hrs, Volume= 6.016 af, Depth= 5.90" Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90" Area (ac) CN Description 12.235 100	Subcatchme	ent s21:
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90" Area (ac) CN Description 96.056 68 Tc Length Slope Velocity Capacity Description (feet) (ft/ft) (ft/sec) (cfs) 26.7 Direct Entry, Subcatchment s21(OW): Runoff = 86.66 cfs @ 12.00 hrs, Volume= 6.016 af, Depth= 5.90" Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90" Area (ac) CN Description 12.235 100		
Type III 24-hr 25-yr Rainfall=5.90"         Area (ac) CN Description         96.056       68         Tc Length Slope Velocity Capacity Description         (min)       (feet)       (ft/ft)       (ft/sec)       (cfs)         26.7       Direct Entry,         Subcatchment s21(OW):         Runoff = 86.66 cfs @ 12.00 hrs, Volume= 6.016 af, Depth= 5.90"         Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs         Type III 24-hr 25-yr Rainfall=5.90"         Area (ac) CN Description         12.235       100	Runoff = $168.20 \text{ cfs} @ 12.38 \text{ hrs, Volume}=$	20.366 af, Depth= 2.54"
96.056       68         Tc       Length       Slope       Velocity       Capacity       Description         (min)       (feet)       (ft/ft)       (ft/sec)       (cfs)         26.7       Direct Entry,         Subcatchment s21(OW):         Runoff         86.66 cfs @       12.00 hrs, Volume=       6.016 af, Depth=       5.90"         Runoff by SCS TR-20 method, UH=SCS, Time Span=         0.00-48.00 hrs, dt=       0.01 hrs         Type III 24-hr 25-yr       Rainfall=       5.90"         Area (ac)       CN         Description       12.235       100	Type III 24-hr 25-yr Rainfall=5.90"	.00-48.00 hrs, dt= 0.01 hrs
Tc       Length       Slope       Velocity       Capacity       Description         26.7       Direct Entry,         Subcatchment s21(OW):         Runoff       =       86.66 cfs @       12.00 hrs, Volume=       6.016 af, Depth= 5.90"         Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs         Type III 24-hr 25-yr       Rainfall=5.90"         Area (ac)       CN       Description         12.235       100		
(min)(feet)(ft/ft)(ft/sec)(cfs)26.7Direct Entry,Subcatchment s21(OW):Runoff = 86.66 cfs @ 12.00 hrs, Volume= 6.016 af, Depth= 5.90"Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrsType III 24-hr 25-yr Rainfall=5.90"Area (ac)CNDescription12.235	96.056 68	
Subcatchment s21(OW):         Runoff = 86.66 cfs @ 12.00 hrs, Volume= 6.016 af, Depth= 5.90"         Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs         Type III 24-hr 25-yr Rainfall=5.90"         Area (ac) CN Description         12.235       100		otion
Runoff=86.66 cfs @12.00 hrs, Volume=6.016 af, Depth=5.90"Runoff by SCS TR-20 method, UH=SCS, Time Span=0.00-48.00 hrs, dt=0.01 hrsType III 24-hr 25-yr Rainfall=5.90"Area (ac)CNDescription12.235100	26.7 <b>Direct</b>	Entry,
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90" Area (ac) CN Description 12.235 100	Subcatchment	s21(OW):
Type III 24-hr 25-yr Rainfall=5.90" <u>Area (ac) CN Description</u> 12.235 100	Runoff = 86.66 cfs @ 12.00 hrs, Volume=	6.016 af, Depth= 5.90"
12.235 100		.00-48.00 hrs, dt= 0.01 hrs
12.235 100	Area (ac) CN Description	
To Longth Slope Velecity Capacity Description		
(min) (feet) (ft/ft) (ft/sec) (cfs)	Tc Length Slope Velocity Capacity Descrip (min) (feet) (ft/ft) (ft/sec) (cfs)	otion
0.0 Direct Entry,		Entry,

Existing Conditions_10454-01Type III 24-hr 25-yrRainfall=5.90"Prepared by The Chazen CompaniesPage 109HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems4/10/2006 2:33:42 PM
Subcatchment s22:
Runoff = 139.25 cfs @ 12.45 hrs, Volume= 18.067 af, Depth= 2.63"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90"
Area (ac) CN Description 82.287 69
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
31.3 Direct Entry,
Subcatchment s22(OW): s22 Open Water
Runoff = 0.96 cfs @ 12.00 hrs, Volume= 0.067 af, Depth= 5.90"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90"
Area (ac) CN Description
0.136 100
Subcatchment s23:
Runoff = 63.82 cfs @ 12.68 hrs, Volume= 10.091 af, Depth= 2.91"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90"
Area (ac) CN Description
41.587 72
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
47.0 Direct Entry,
Subcatchment s24:
Runoff = 50.24 cfs @ 12.45 hrs, Volume= 6.435 af, Depth= 2.73"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90"
Area (ac) CN Description 28.325 70

Existing Conditions_10454-01	Type III 24-hr 25-yr Rainfall=5.90"	
Prepared by The Chazen Companies HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Syste	Page 110 ems 4/10/2006 2:33:42 PM	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
30.7 Direct Entry,		
Subcatchment s25:		
Runoff = 24.65 cfs @ 12.28 hrs, Volume= 2.674 a	f, Depth= 2.37"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs Type III 24-hr 25-yr Rainfall=5.90"	, dt= 0.01 hrs	
Area (ac) CN Description		
13.562 66		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
20.2 Direct Entry,		
Reach r03:		
Overland Flow Reach Requires more survey		
Inflow Area =       11.485 ac, Inflow Depth = 2.54" for 25-yr ever         Inflow =       16.08 cfs @ 12.60 hrs, Volume=       2.435 a         Outflow =       16.00 cfs @ 12.64 hrs, Volume=       2.435 a		
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.0 Max. Velocity= 5.5 fps, Min. Travel Time= 2.4 min Avg. Velocity = 2.1 fps, Avg. Travel Time= 6.3 min	1 hrs	
Peak Depth= 0.66' @ 12.64 hrs Capacity at bank full= 92.14 cfs Inlet Invert= 845.00', Outlet Invert= 728.00' 10.00' x 1.50' deep Parabolic Channel, n= 0.060 Length= 785.0' Slope= 0.1490 '/'		
Reach r04:		
Channel		
Inflow Area =         26.659 ac, Inflow Depth =         2.60" for 25-yr ever           Inflow =         40.11 cfs @         12.48 hrs, Volume=         5.767 a           Outflow =         40.03 cfs @         12.51 hrs, Volume=         5.767 a		
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.0 Max. Velocity= 7.1 fps, Min. Travel Time= 1.6 min Avg. Velocity = 2.7 fps, Avg. Travel Time= 4.2 min	1 hrs	

Existing Conditions 10454-01 Type III 24-hr 25-yr Rainfall=5.90" Prepared by The Chazen Companies HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems 4/10/2006 2:33:42 PM

Peak Depth= 1.26' @ 12.51 hrs Capacity at bank full= 446.15 cfs Inlet Invert= 685.50', Outlet Invert= 608.00' 12.00' x 4.00' deep Parabolic Channel, n= 0.060 Length= 675.0' Slope= 0.1148 '/'

#### Reach r08a:

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Man Made Ditch Inverts of pipe to be surveyed

Inflow Are	a =	97.712 ac, Inflow Dept	h= 1.71"	for 25-yr event	
Inflow	=	35.41 cfs @ 12.88 hrs	, Volume=	13.933 af	
Outflow	=	35.41 cfs @ 12.88 hrs	Volume=	13.933 af,	Atten= 0%, Lag= 0.3 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 9.5 fps, Min. Travel Time= 0.4 min Avg. Velocity = 5.4 fps, Avg. Travel Time= 0.7 min

Peak Depth= 0.68' @ 12.88 hrs Capacity at bank full= 81.88 cfs Inlet Invert= 607.00', Outlet Invert= 587.00' 10.00' x 1.00' deep Parabolic Channel, n= 0.027 Length= 226.0' Slope= 0.0885 '/'

#### Reach r08b:

24" HDPE Inverts to be surveyed

Inflow Area = 97.712 ac, Inflow Depth = 1.71" for 25-yr event Inflow = 35.41 cfs @ 12.88 hrs, Volume= 13.933 af Outflow 35.41 cfs @ 12.89 hrs, Volume= 13.933 af, Atten= 0%, Lag= 0.2 min =

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 24.0 fps, Min. Travel Time= 0.2 min Avg. Velocity = 14.1 fps, Avg. Travel Time= 0.3 min

Peak Depth= 0.95' @ 12.89 hrs Capacity at bank full= 77.17 cfs Inlet Invert= 587.00', Outlet Invert= 557.75' 24.0" Diameter Pipe n= 0.012 Length= 295.0' Slope= 0.0992 '/'

#### Reach r08c:

Ditch Pipe inverts to be surveyed

Inflow Area = 97.712 ac. Inflow Depth =  $1.71^{\circ}$  for 25-vr event Inflow 35.41 cfs @ 12.89 hrs, Volume= 13.933 af = 35.41 cfs @ 12.90 hrs, Volume= Outflow 13.933 af, Atten= 0%, Lag= 0.6 min =

Existing Conditions\_10454-01 Type In Prepared by The Chazen Companies HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 9.1 fps, Min. Travel Time= 1.1 min Avg. Velocity = 4.9 fps, Avg. Travel Time= 2.0 min

Peak Depth= 0.70' @ 12.90 hrs Capacity at bank full= 76.65 cfs Inlet Invert= 557.75', Outlet Invert= 512.00' 10.00' x 1.00' deep Parabolic Channel, n= 0.027 Length= 590.0' Slope= 0.0775 '/'

#### Reach r08d: Amenia Stream/Cascade Brook

(Steam) parallel to Route 22 Base Flow estimated from field observation (see field note 21)

97.712 ac, Inflow Depth = 21.20" for 25-yr event Inflow Area = Inflow 75.41 cfs @ 12.90 hrs, Volume= 172.644 af, Incl. 40.00 cfs Base Flow = 75.39 cfs @ 12.94 hrs, Volume= Outflow 172.400 af, Atten= 0%, Lag= 2.6 min = Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 3.6 fps, Min. Travel Time= 3.7 min Avg. Velocity = 3.1 fps, Avg. Travel Time= 4.3 min Peak Depth= 3.41' @ 12.94 hrs Capacity at bank full= 104.49 cfs Inlet Invert= 512.00', Outlet Invert= 504.00' 10.00' x 4.00' deep Parabolic Channel, n= 0.060 Length= 805.0' Slope= 0.0099 '/'

#### Reach r14a:

Grass lined channel

 Inflow Area =
 8.452 ac, Inflow Depth =
 2.22" for 25-yr event

 Inflow =
 13.21 cfs @
 12.36 hrs, Volume=
 1.566 af

 Outflow =
 13.20 cfs @
 12.37 hrs, Volume=
 1.566 af, Atten= 0%, Lag= 0.7 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 6.5 fps, Min. Travel Time= 0.9 min Avg. Velocity = 1.6 fps, Avg. Travel Time= 3.7 min

Peak Depth= 0.66' @ 12.37 hrs Capacity at bank full= 325.42 cfs Inlet Invert= 542.00', Outlet Invert= 526.00' 10.00' x 3.00' deep Parabolic Channel, n= 0.027 Length= 360.0' Slope= 0.0444 '/'

#### Reach r14b:

30" HDPE Under Main Entrance Road

 Inflow Area =
 8.452 ac, Inflow Depth =
 2.22" for 25-yr event

 Inflow =
 13.20 cfs @
 12.37 hrs, Volume=
 1.566 af

 Outflow =
 13.19 cfs @
 12.38 hrs, Volume=
 1.566 af, Atten= 0%, Lag= 0.4 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 13.6 fps, Min. Travel Time= 0.5 min Avg. Velocity = 3.4 fps, Avg. Travel Time= 2.2 min

Peak Depth= 0.63' @ 12.38 hrs Capacity at bank full= 94.91 cfs Inlet Invert= 526.00', Outlet Invert= 505.70' 30.0" Diameter Pipe n= 0.012 Length= 445.0' Slope= 0.0456 '/'

#### Reach r14c:

#### **Overland Flow**

Inflow Area = Inflow = Outflow =	6.420 ac, Inflow Depth =0.52"for 25-yr event0.55 cfs @16.49 hrs, Volume=0.278 af0.55 cfs @16.65 hrs, Volume=0.278 af, Atten= 1%, Lag= 9.7 min
Max. Velocity= 0.9	tor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs 9 fps, Min. Travel Time= 10.6 min 4 fps, Avg. Travel Time= 22.8 min

#### Reach r15:

Brush Overbanks with Rocky Bottom Needs to be surveyed

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 10.1 fps, Min. Travel Time= 0.5 min Avg. Velocity = 3.5 fps, Avg. Travel Time= 1.4 min

Peak Depth= 0.90' @ 12.06 hrs Capacity at bank full= 188.47 cfs Inlet Invert= 554.00', Outlet Invert= 528.00' 5.00' x 3.00' deep Parabolic Channel, n= 0.027 Length= 290.0' Slope= 0.0897 '/'

# Reach r16:

#### Pipe Reach

Inflow Area =4.702 ac, Inflow Depth =4.14"for 25-yr eventInflow =17.06 cfs @12.04 hrs, Volume=1.622 afOutflow =16.62 cfs @12.06 hrs, Volume=1.622 af, Atten= 3%, Lag= 1.0 min			
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 11.2 fps, Min. Travel Time= 1.3 min Avg. Velocity = 3.8 fps, Avg. Travel Time= 3.8 min			
Peak Depth= 0.85' @ 12.06 hrs Capacity at bank full= 66.05 cfs Inlet Invert= 573.00', Outlet Invert= 554.00' 30.0" Diameter Pipe n= 0.012 Length= 860.0' Slope= 0.0221 '/'			
Reach r18a:			
Overland Flow Reach			
Inflow         =         0.00 cfs @         0.00 hrs, Volume=         0.000 af           Outflow         =         0.00 cfs @         0.00 hrs, Volume=         0.000 af, Atten= 0%, Lag= 0.0 min			
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 0.0 fps, Min. Travel Time= 0.0 min Avg. Velocity = 0.0 fps, Avg. Travel Time= 0.0 min			
Peak Depth= 0.00' @ 0.00 hrs Capacity at bank full= 379.63 cfs Inlet Invert= 973.60', Outlet Invert= 530.00' 50.00' x 1.00' deep Parabolic Channel, n= 0.060 Length= 1,220.0' Slope= 0.3636 '/'			
Reach r18b:			
Overland Flow Reach			
Inflow         =         0.00 cfs @         0.00 hrs, Volume=         0.000 af           Outflow         =         0.00 cfs @         0.00 hrs, Volume=         0.000 af, Atten= 0%, Lag= 0.0 min			
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 0.0 fps, Min. Travel Time= 0.0 min Avg. Velocity = 0.0 fps, Avg. Travel Time= 0.0 min			
Peak Depth= 0.00' @ 0.00 hrs Capacity at bank full= 151.94 cfs Inlet Invert= 530.60', Outlet Invert= 514.00' 50.00' x 1.00' deep Parabolic Channel, n= 0.060 Length= 285.0' Slope= 0.0582 '/'			

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## Reach r21a:

Man Made Ditch

Inflow Area =241.484 ac, Inflow Depth =1.93"for25-yr eventInflow =74.47 cfs @14.84 hrs, Volume=38.888 afOutflow =74.46 cfs @14.86 hrs, Volume=38.875 af, Atten= 0%, Lag= 1.1 min			
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 7.4 fps, Min. Travel Time= 1.5 min Avg. Velocity = 3.2 fps, Avg. Travel Time= 3.4 min			
Peak Depth= 1.90' @ 14.86 hrs Capacity at bank full= 191.76 cfs Inlet Invert= 504.00', Outlet Invert= 494.00' 10.00' x 3.00' deep Parabolic Channel, n= 0.027 Length= 648.0' Slope= 0.0154 '/'			
Reach r21b:			
Grass Ditch Geometry to be confirmed by survey (inverts at pipe)			
Inflow Area =97.943 ac, Inflow Depth =2.48"for25-yr eventInflow =139.51 cfs @12.49 hrs, Volume=20.234 afOutflow =139.49 cfs @12.50 hrs, Volume=20.234 af, Atten= 0%, Lag= 0.3 min			
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 10.2 fps, Min. Travel Time= 0.4 min Avg. Velocity = 2.4 fps, Avg. Travel Time= 1.6 min			
Peak Depth= 1.55' @ 12.50 hrs Capacity at bank full= 239.90 cfs Inlet Invert= 499.00', Outlet Invert= 491.10' 15.00' x 2.00' deep Parabolic Channel, n= 0.027 Length= 230.0' Slope= 0.0343 '/'			

## Reach r21c:

**Overland Flow Reach** 

Inflow Area = 41.587 ac, Inflow Depth =  $2.65^{\circ}$  for 25-yr event Inflow = 63.81 cfs @ 12.68 hrs, Volume= 9.172 afOutflow = 63.81 cfs @ 12.69 hrs, Volume= 9.172 af, Atten= 0%, Lag= 0.3 min Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrsMax. Velocity= 4.6 fps, Min. Travel Time= 0.6 minAvg. Velocity = 1.8 fps, Avg. Travel Time= 1.5 min Existing Conditions 10454-01 Type III 24-hr 25-yr Rainfall=5.90" Prepared by The Chazen Companies HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems 4/10/2006 2:33:42 PM

Peak Depth= 0.56' @ 12.69 hrs Capacity at bank full= 227.81 cfs Inlet Invert= 506.70', Outlet Invert= 485.75' 50.00' x 1.00' deep Parabolic Channel, n= 0.060 Length= 160.0' Slope= 0.1309 '/'

#### Reach r22a:

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**Overland Flow Reach** 

Inflow Area = Inflow = Outflow =	15.520 ac, Inflow Depth =1.86"for 25-yr event9.63 cfs @13.01 hrs, Volume=2.400 af9.58 cfs @13.07 hrs, Volume=2.400 af, Atten= 0%, Lag= 3.2	min					
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 3.9 fps, Min. Travel Time= 4.2 min Avg. Velocity = 1.2 fps, Avg. Travel Time= 13.2 min							
Peak Depth= 0.18' @ 13.07 hrs Capacity at bank full= 409.31 cfs Inlet Invert= 970.00', Outlet Invert= 560.00'							

50.00' x 1.00' deep Parabolic Channel, n= 0.060 Length= 970.0' Slope= 0.4227 '/'

## Reach r22b:

**Overland Flow Reach** 

Inflow Area = Inflow = Outflow =	15.520 ac, Inflow Depth =1.86"for 25-yr event9.58 cfs @13.07 hrs, Volume=2.400 af9.52 cfs @13.13 hrs, Volume=2.399 af, Atten= 1%, Lag= 3.8 min					
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 2.1 fps, Min. Travel Time= 4.8 min Avg. Velocity = 0.7 fps, Avg. Travel Time= 15.7 min						
Peak Depth= 0.26' @ 13.13 hrs Capacity at bank full= 175.17 cfs Inlet Invert= 560.00', Outlet Invert= 512.00' 50.00' x 1.00' deep Parabolic Channel, n= 0.060 Length= 620.0' Slope= 0.0774 '/'						

#### Reach r25a:

Ditch Pipe inverts need to be surveyed

60.314 ac, Inflow Depth = 2.34" for 25-yr event Inflow Area = Inflow 74.28 cfs @ 12.40 hrs, Volume= 11.742 af = Outflow 74.14 cfs @ 12.42 hrs. Volume= 11.741 af, Atten= 0%, Lag= 1.2 min =

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 11.4 fps, Min. Travel Time= 1.6 min Avg. Velocity = 2.4 fps, Avg. Travel Time= 7.5 min

Peak Depth= 1.24' @ 12.42 hrs Capacity at bank full= 205.50 cfs Inlet Invert= 570.00', Outlet Invert= 504.00' 10.00' x 2.00' deep Parabolic Channel, n= 0.027 Length= 1,090.0' Slope= 0.0606 '/'

#### Reach r25b: Wetland Reach

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Wetland Reach Has wetland vegetation within reach

Inflow Area = Inflow = Outflow =	13.69 cfs @	nflow Depth = 12.35 hrs,  V 12.45 hrs,  V	olume=	or 25-yr event 1.699 af 1.699 af,	Atten= 10%, Lag= 6.4 min		
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 1.8 fps, Min. Travel Time= 6.9 min Avg. Velocity = 0.4 fps, Avg. Travel Time= 27.9 min							
Peak Depth= 0.92' @ 12.45 hrs Capacity at bank full= 156.51 cfs Inlet Invert= 504.00', Outlet Invert= 499.50'							

20.00' x 3.00' deep Parabolic Channel, n= 0.045 Length= 750.0' Slope= 0.0060 '/'

#### Reach r25c: Amenia Stream/Cascade Brook

(Steam) parallel to Route 22 Base Flow estimated from field observation (see field note 21)

Inflow Area = 129.431 ac, Inflow Depth = 31.11" for 25-yr event Inflow 146.68 cfs @ 12.41 hrs, Volume= 335.591 af, Incl. 40.00 cfs Base Flow = 144.18 cfs @ 12.51 hrs, Volume= Outflow = 334.909 af. Atten= 2%. Lag= 5.9 min Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

Max. Velocity= 2.9 fps, Min. Travel Time= 5.6 min Avg. Velocity = 2.7 fps, Avg. Travel Time= 6.1 min

Peak Depth= 6.27' @ 12.51 hrs Capacity at bank full= 67.14 cfs Inlet Invert= 504.00', Outlet Invert= 500.00' 10.00' x 4.00' deep Parabolic Channel, n= 0.060 Length= 975.0' Slope= 0.0041 '/'

## Pond 8P:

No field note. Water spills over cart path; no storage.

Inflow Area =	=	52.997 ac, Inflow Depth = 2.33" for 25-yr event	
Inflow =	=	67.94 cfs @ 12.36 hrs, Volume= 10.279 af	
Outflow =	=	67.94 cfs @ 12.36 hrs, Volume= 10.279 af, Atten= 0%, Lag	g= 0.0 min
Primary =	=	67.94 cfs @ 12.36 hrs, Volume= 10.279 af	-

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 575.58' @ 12.36 hrs Flood Elev= 574.70' Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= (not calculated)

#	Routing	Invert	Outlet Devices
1	Primary	574.70'	177.0 deg Sharp-Crested Vee/Trap Weir C= 2.46

Primary OutFlow Max=67.94 cfs @ 12.36 hrs HW=575.58' TW=571.23' (Dynamic Tailwater) **1=Sharp-Crested Vee/Trap Weir** (Weir Controls 67.94 cfs @ 2.3 fps)

#### Pond p04:

Field Note #13

Water ponding behind a golf cart path. Overflow dimensions are assumed based on aerial topo, and should be upgraded once survey is available.

Inflow Area =	38.062 ac, Inflow Depth = 2.50"	for 25-yr event
Inflow =	49.26 cfs @ 12.43 hrs, Volume=	7.932 af
Outflow =	49.26 cfs @ 12.44 hrs, Volume=	7.866 af, Atten= 0%, Lag= 0.2 min
Primary =	49.26 cfs @ 12.44 hrs, Volume=	7.866 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 606.00' @ 12.44 hrs Surf.Area= 6,641 sf Storage= 3,985 cf Flood Elev= 605.50' Surf.Area= 4,803 sf Storage= 2,882 cf Plug-Flow detention time= 8.1 min calculated for 7.866 af (99% of inflow) Center-of-Mass det. time= 3.0 min (870.5 - 867.5)

#	Invert	Avail.Ste	orage	Storage Des	scription		
1	604.20'	26,8	397 cf	Custom Sta	ige Data (Conic) L	isted below	
Elevatio (fee		Surf.Area (sq-ft)	(c	Inc.Store ubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft <u>)</u>	
604.2 606.0 608.0	00	0 6,650 17,060		0 3,990 22,907	0 3,990 26,897	0 6,655 17,092	
#_ Ro	outing	Invert	Outlet [	Devices			
1 Pr	rimary	605.50'	179.0 d	leg Sharp-Ci	rested Vee/Trap W	<b>/eir</b> C= 2.46	

Primary OutFlow Max=49.25 cfs @ 12.44 hrs HW=606.00' TW=575.57' (Dynamic Tailwater) **1=Sharp-Crested Vee/Trap Weir** (Weir Controls 49.25 cfs @ 1.7 fps) HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems

## Pond p06:

Field Note #22 Geometry to be confirmed by survey.

Inflow Area =	9.435 ac, Inflow Depth = $2.20"$	for 25-yr event
Inflow =	15.39 cfs @ 12.25 hrs, Volume=	1.728 af
Outflow =	13.69 cfs @ 12.35 hrs, Volume=	1.699 af, Atten= 11%, Lag= 5.9 min
Primary =	13.69 cfs @ 12.35 hrs, Volume=	1.699 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 506.80' Surf.Area= 18,600 sf Storage= 42,160 cf Peak Elev= 507.48' @ 12.35 hrs Surf.Area= 21,671 sf Storage= 56,585 cf (14,425 cf above start) Flood Elev= 507.10' Surf.Area= 19,958 sf Storage= 48,537 cf (6,377 cf above start) Plug-Flow detention time= 507.9 min calculated for 0.732 af (42% of inflow) Center-of-Mass det. time= 111.5 min (962.4 - 851.0)

#	Invert	Avail.St	torage	Storage Des	cription		
1	500.00'	67,	669 cf	Custom Sta	ge Data (Conic) L	isted below	
500 506	tion eet) 0.00 6.80 3.00	Surf.Area (sq-ft) 0 18,600 24,030	(	Inc.Store cubic-feet) 0 42,160 25,509	Cum.Store (cubic-feet) 0 42,160 67,669	Wet.Area (sq-ft) 0 18,672 24,138	
1	<u>Routing</u> Primary Primary	Invert 506.80' 507.10'	Outlet Devices         12.0" x 20.0' long Culvert CMP, projecting, no headwall, Ke= 0.900         Outlet Invert= 506.00' S= 0.0400 '/' n= 0.024 Cc= 0.900         178.0 deg Sharp-Crested Vee/Trap Weir C= 2.46				

Primary OutFlow Max=13.68 cfs @ 12.35 hrs HW=507.48' TW=504.85' (Dynamic Tailwater) -1=Culvert (Inlet Controls 1.26 cfs @ 2.2 fps)

-2=Sharp-Crested Vee/Trap Weir (Weir Controls 12.43 cfs @ 1.5 fps)

## Pond p07:

Field Note # 29 Outlet geometry to be confirmed by survey.

Inflow Are	ea =	7.317 ac, Inflow Depth = 2.45'	for 25-yr event
Inflow	=	14.31 cfs @ 12.19 hrs, Volume=	= 1.493 af
Outflow	=	7.50 cfs @ 12.49 hrs, Volume=	= 1.463 af, Atten= 48%, Lag= 17.7 min
Primary	=	7.50 cfs @ 12.49 hrs, Volume=	= 1.463 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 572.80' Surf.Area= 21,640 sf Storage= 56,264 cf Peak Elev= 573.73' @ 12.49 hrs Surf.Area= 25,998 sf Storage= 78,858 cf (22,594 cf above start) Flood Elev= 573.50' Surf.Area= 24,936 sf Storage= 73,351 cf (17,087 cf above start) Plug-Flow detention time= 1,125.3 min calculated for 0.171 af (11% of inflow) Center-of-Mass det. time= 176.0 min (1,013.4 - 837.4)

## Existing Conditions\_10454-01

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#	Invert	Avail.St	orage	Storage Des	scription			
1	565.00'	85,	557 cf	cf Custom Stage Data (Conic) Listed below				
565 572	tion eet) 5.00 2.80 4.00	Surf.Area (sq-ft) 0 21,640 27,290	(c	Inc.Store ubic-feet) 0 56,264 29,293	Cum.Store (cubic-feet) 0 56,264 85,557	Wet.Area (sq-ft) 0 21,735 27,424		
#	Routing	Invert	Outlet [	Devices				
1	Primary	572.80'				jecting, no headwa		
2	Primary	573.50'				n= 0.024 Cc= 0.9 /eir X 2.00 C= 2.4		

Primary OutFlow Max=7.50 cfs @ 12.49 hrs HW=573.73' TW=571.22' (Dynamic Tailwater) -1=Culvert (Inlet Controls 2.96 cfs @ 2.6 fps)

-2=Sharp-Crested Vee/Trap Weir (Weir Controls 4.54 cfs @ 1.2 fps)

## Pond p09:

Field Note #31 Geometry to be confirmed by survey.

Inflow Area	a =	8.452 ac, Inflow Depth = 2.28"	for 25-yr event
Inflow	=	16.02 cfs @ 12.23 hrs, Volume=	1.605 af
Outflow	=	13.21 cfs @ 12.36 hrs, Volume=	1.566 af, Atten= 18%, Lag= 7.8 min
Primary	=	13.21 cfs @ 12.36 hrs, Volume=	1.566 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 550.26' @ 12.36 hrs Surf.Area= 6,173 sf Storage= 8,277 cf Flood Elev= 551.20' Surf.Area= 8,534 sf Storage= 15,673 cf Plug-Flow detention time= 34.8 min calculated for 1.566 af (98% of inflow) Center-of-Mass det. time= 21.2 min ( 882.0 - 860.7 )

#	Invert	Avail.St	torage Storage I	Storage Description				
1	547.50'	21,	989 cf Custom	cf Custom Stage Data (Conic) Listed below				
	ation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)		Wet.Area (sq-ft <u>)</u>			
54	17.50	0	0	0	0			
54	18.00	1,080	180	180	1,080			
55	50.00	5,510	6,020	6,200	5,527			
55	52.00	10,550	15,790	21,989	10,606			
#	Routing	Invert	Outlet Devices					
1	Primary	548.50'		<b>30.0" x 70.0' long Culvert</b> CMP, projecting, no headwall, Ke= 0.900 Outlet Invert= 542.00' S= 0.0929 '/' n= 0.012 Cc= 0.900				
2	Primary	551.20'		0 deg Sharp-Crested Vee/Trap Weir C= 2.46				

Primary OutFlow Max=13.21 cfs @ 12.36 hrs HW=550.26' TW=542.65' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 13.21 cfs @ 3.6 fps) -2=Sharp-Crested Vee/Trap Weir (Controls 0.00 cfs)

## Pond p10:

Field Note #25 Need to get full story on how this pond works

Inflow Are	a =	45.146 ac, I	nflow Depth = 0.91"	for 25-yr event	
Inflow	=	14.48 cfs @	12.37 hrs, Volume=	3.415 af	
Outflow	=	0.00 cfs @	0.00 hrs, Volume=	0.000 af,	Atten= 100%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 498.40' Surf.Area= 36,110 sf Storage= 101.108 cf Peak Elev= 501.77' @ 48.00 hrs Surf.Area= 53,467 sf Storage= 249,873 cf (148,765 cf above start) Plug-Flow detention time= (not calculated) Center-of-Mass det. time= (not calculated)

#	Invert	Avail.Stora	ige Storage Des	cription	
1	490.00'	581,029	ocf Custom Sta	<b>ge Data (Conic)</b> Li	sted below
Elevati (fe		Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
490.	00	0	0	0	0
498.	40	36,110	101,108	101,108	36,221
500.	00	42,400	62,741	163,849	42,610
502.	00	54,880	97,012	260,861	55,187
504.	00	78,730	132,895	393,755	79,107
506.	00	109,382	187,274	581,029	109,836

Pond p12:

No field note. Natural depression.

Inflow Area	=	6.420 ac, Inflow Depth = 1.46"	for 25-yr event
Inflow	=	4.67 cfs @ 12.65 hrs, Volume=	0.781 af
Outflow	=	0.55 cfs @ 16.49 hrs, Volume=	0.278 af, Atten= 88%, Lag= 230.1 min
Primary	=	0.55 cfs @ 16.49 hrs, Volume=	0.278 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 547.68' @ 16.49 hrs Surf.Area= 14,878 sf Storage= 23,643 cf Flood Elev= 547.50' Surf.Area= 13,848 sf Storage= 21,762 cf Plug-Flow detention time= 449.9 min calculated for 0.278 af (36% of inflow) Center-of-Mass det. time= 296.8 min (1,208.6 - 911.8)

## Existing Conditions\_10454-01

Prepared by The Chazen Companies

Hydro	HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems 4/10/2006 2:33:43 PI									
#	Invert	Avail.Ste	orage Storage De	age Storage Description						
1	543.50'	26,9	986 cf Custom Sta	age Data (Conic) L	isted below					
	ation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft <u>)</u>					
54	13.50	0	0	0	0					
54	14.00	1,140	190	190	1,140					
54	46.00	5,260	5,899	6,089	5,278					
54	18.00	16,710	20,897	26,986	16,750					
#	Routing	Invert	Outlet Devices							
1	Primary	547.50'	173.0 deg Sharp-C	rested Vee/Trap W	<b>/eir</b> C= 2.46					
Drim	<b>Primary OutElow</b> Max-0.55 cfc @ 16.40 hrs. $HW/=547.68'$ . $TW/=544.07'$ (Dynamic Tailwater)									

Primary OutFlow Max=0.55 cfs @ 16.49 hrs HW=547.68' TW=544.07' (Dynamic Tailwater) -1=Sharp-Crested Vee/Trap Weir (Weir Controls 0.55 cfs @ 1.0 fps)

### Pond p13:

No Field Note Natural depression.

Inflow Area	a =	0.350 ac, Ir	nflow Depth = 1.46"	for 25-yr event	
Inflow	=	0.48 cfs @	12.14 hrs, Volume=	0.043 af	
Outflow	=	0.00 cfs @	0.00 hrs, Volume=	0.000 af,	Atten= 100%, Lag= 0.0 min
Primary	=	0.00 cfs @	0.00 hrs, Volume=	0.000 af	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 514.07' @ 24.49 hrs Surf.Area= 1,389 sf Storage= 1,856 cf Flood Elev= 519.50' Surf.Area= 4,313 sf Storage= 16,523 cf Plug-Flow detention time= (not calculated) Center-of-Mass det. time= (not calculated)

#	Invert	Avail.St	orage	age Storage Description				
1	511.40'	18,4	490 cf	Custom Sta	age Data (Conic)	Listed below		
Elevatio (fee		Surf.Area (sq-ft)	(c	Inc.Store cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)		
511.4	40	0		0	0	0		
512.0	00	390		78	78	391		
514.0	00	1,360		1,652	1,730	1,381		
516.0	00	2,180		3,508	5,238	2,253		
518.0	00	3,240		5,385	10,623	3,375		
520.0	00	4,670		7,867	18,490	4,872		
_# R	outing	Invert	Outlet I	Devices				
1 P	rimary	519.50'	176.0 deg Sharp-Crested Vee/Trap Weir C= 2.46					

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=511.40' TW=497.40' (Dynamic Tailwater) ↑ 1=Sharp-Crested Vee/Trap Weir (Controls 0.00 cfs) HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems

### Pond p14:

Field Note #26 Need to figure out how this pond works

Inflow Area =	36.186 ac, Inflow Depth = 2.16"	for 25-yr event
Inflow =	43.40 cfs @ 12.39 hrs, Volume=	6.522 af
Outflow =	1.41 cfs @ 23.79 hrs, Volume=	1.344 af, Atten= 97%, Lag= 684.1 min
Primary =	1.41 cfs @ 23.79 hrs, Volume=	1.344 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 497.40' Surf.Area= 22,200 sf Storage= 54,760 cf Peak Elev= 502.49' @ 23.79 hrs Surf.Area= 75,975 sf Storage= 309,505 cf (254,745 cf above start) Plug-Flow detention time= 2,325.3 min calculated for 0.087 af (1% of inflow) Center-of-Mass det. time= 728.2 min (1,590.8 - 862.6)

#	Invert	Avail.Sto	rage Storage Des	Storage Description				
1	490.00'	805,06	62 cf Custom Sta	i <b>ge Data (Conic)</b> Li	sted below			
Eleva		Surf.Area	Inc.Store	Cum.Store	Wet.Area			
(f	eet)	(sq-ft)	(cubic-feet)	(cubic-feet)	(sq-ft)			
490	00.0	0	0	0	0			
497	7.40	22,200	54,760	54,760	22,286			
498	3.00	25,330	14,249	69,009	25,433			
500	0.00	52,810	76,476	145,485	52,948			
502	2.00	73,360	125,608	271,093	73,574			
504	4.00	84,070	157,308	428,402	84,467			
506	5.00	92,130	176,139	604,540	92,797			
508	3.00	108,618	200,522	805,062	109,437			
#	Routing	Invert C	Dutlet Devices					
1	Primary	500.00' 2	24.0" x 80.0' long (	Culvert CPP, end-	section conform	ing to fill, Ke= 0.500		
	-	C	Dutlet Invert= 502.0	0' S= -0.0250 '/'	n= 0.012 Cc= 0	.900		

Primary OutFlow Max=1.41 cfs @ 23.79 hrs HW=502.49' TW=501.05' (Dynamic Tailwater)

## Pond p15:

Field Note # 43 Infiltration basin

Inflow Area	a =	5.770 ac, Inflow Depth = $3.59$	" for 25-yr event
Inflow	=	16.94 cfs @ 12.07 hrs, Volume	= 1.726 af
Outflow	=	16.83 cfs @ 12.07 hrs, Volume	= 1.470 af, Atten= 1%, Lag= 0.6 min
Primary	=	16.83 cfs @ 12.07 hrs, Volume	= 1.470 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 536.22' @ 12.07 hrs Surf.Area= 3,289 sf Storage= 11,955 cf Flood Elev= 536.00' Surf.Area= 3,160 sf Storage= 11,127 cf Plug-Flow detention time= 119.0 min calculated for 1.470 af (85% of inflow)

#	Invert	Avail.Sto	orage St	Storage Description				
1	526.80'	18,5	77 cf <b>C</b>	ustom S	tage Data (Conic)	Listed below		
Eleva	ition eet)	Surf.Area (sq-ft)		c.Store ic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)		
526	6.80	0		0	0	0		
528	3.00	310		124	124	312		
530	0.00	660		948	1,072	694		
532	2.00	1,180		1,815	2,887	1,256		
534	4.00	1,990		3,135	6,022	2,113		
536	5.00	3,160		5,105	11,127	3,337		
538	3.00	4,320		7,450	18,577	4,575		
	Routing		Outlet De			(a.d.)/a.a/T.a.a.a.)0/a	- 0 0 40	
1	Primary	536.00' '	1/1.0 deg	J X 50.0'	long Sharp-Cres	ted vee/Trap We	II C = 2.46	

Center-of-Mass det. time= 51.4 min (840.3 - 788.9)

Primary OutFlow Max=16.79 cfs @ 12.07 hrs HW=536.22' TW=507.88' (Dynamic Tailwater) ←1=Sharp-Crested Vee/Trap Weir (Weir Controls 16.79 cfs @ 1.4 fps)

#### Pond p16:

Field Note # 49 Large pond with man-made island. Geometry to be verified by survey. In particular, we are making big guesses about the outlets. Also need to find out about valves...

Inflow Are	ea =	220.861 ac, I	nflow Depth = 2.53	B" for 25-yr event
Inflow	=	171.50 cfs @	12.37 hrs, Volume	e= 46.533 af
Outflow	=	70.70 cfs @	14.75 hrs, Volume	= 34.668 af, Atten= 59%, Lag= 142.8 min
Primary	=	70.70 cfs @	14.75 hrs, Volume	e= 34.668 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 507.00' Surf.Area= 199,799 sf Storage= 878,320 cf Peak Elev= 511.29' @ 14.75 hrs Surf.Area= 306,516 sf Storage= 1,854,149 cf (975,829 cf above start) Flood Elev= 510.50' Surf.Area= 271,550 sf Storage= 1,623,217 cf (744,897 cf above start) Plug-Flow detention time= 970.7 min calculated for 14.505 af (31% of inflow) Center-of-Mass det. time= 352.4 min (1,271.0 - 918.7)

#I	nvert	Avail.Storage	e Storage De	escription	
1 50	0.00'	2,062,087 c	of Custom St	age Data (Conic)	Listed below
Elevation		Surf.Area	Inc.Store	Cum.Store	Wet.Area
(feet)		(sq-ft)	(cubic-feet)	(cubic-feet)	(sq-ft)
500.00		0	0	0	0
503.00		140,344	140,344	140,344	140,358
509.20		232,500	1,143,862	1,284,206	232,994
510.00		249,400	192,720	1,476,927	249,951
512.00		338,000	585,160	2,062,087	338,634

Existing Conditions\_10454-01

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#	Routing	Invert	Outlet Devices
1	Primary	509.00'	18.0" x 110.0' long Culvert CMP, projecting, no headwall, Ke= 0.900
	-		Outlet Invert= 505.70' S= 0.0300 '/' n= 0.024 Cc= 0.900
2	Primary	500.00'	8.0" x 100.0' long assumed equalization pipe w/ valve X 0.00
			CMP, projecting, no headwall, Ke= 0.900
			Outlet Invert= 500.00' S= 0.0000 '/' n= 0.013 Cc= 0.900
3	Primary	510.50'	175.0 deg Sharp-Crested Vee/Trap Weir X 2.00 C= 2.46

Primary OutFlow Max=70.70 cfs @ 14.75 hrs HW=511.29' TW=506.34' (Dynamic Tailwater) -1=Culvert (Inlet Controls 8.33 cfs @ 4.7 fps) -2=assumed equalization pipe w/ valve (Controls 0.00 cfs)

-3=Sharp-Crested Vee/Trap Weir (Weir Controls 62.37 cfs @ 2.2 fps)

## Pond p17:

Field Note #45 Golf pond Geometry to be confirmed by surveyed

Inflow Area =	115.991 ac, Inflow Depth = 2.37"	for 25-yr event
Inflow =	75.70 cfs @ 13.77 hrs, Volume=	22.921 af
Outflow =	75.66 cfs @ 13.78 hrs, Volume=	22.921 af, Atten= 0%, Lag= 0.5 min
Primary =	75.66 cfs @ 13.78 hrs, Volume=	22.921 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 523.80' Surf.Area= 7,290 sf Storage= 9,234 cf Peak Elev= 525.54' @ 13.78 hrs Surf.Area= 11,275 sf Storage= 25,739 cf (16,505 cf above start) Flood Elev= 524.30' Surf.Area= 8,074 sf Storage= 13,623 cf (4,389 cf above start) Plug-Flow detention time= 18.0 min calculated for 22.709 af (99% of inflow) Center-of-Mass det. time= 9.6 min (968.2 - 958.6)

#	Invert	Avail.St	torage	Storage Des	scription		
1	520.00'	30,	224 cf	Custom Sta	i <b>ge Data (Conic)</b> Li	isted below	
Eleva	ation feet)	Surf.Area (sq-ft)	(c	Inc.Store cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
-	0.00	0		0	0	0	
52	3.80	7,290		9,234	9,234	7,313	
52	4.00	7,300		1,459	10,693	7,374	
52	6.00	12,460		19,531	30,224	12,581	
#	Routing	Invert	Outlet I	Devices			
1	Primary	523.80'	<b>2.2' long x 0.5' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 Coef. (English) 2.80 2.92 3.08 3.30 3.32				
2 3	Primary Primary	524.30' 525.20'	143.0 deg Sharp-Crested Vee/Trap Weir C= 2.47 178.0 deg x 60.0' long Sharp-Crested Vee/Trap Weir C= 2.46				

*Type III 24-hr 25-yr Rainfall=5.90"* Page 125 ns 4/10/2006 2:33:43 PM Primary OutFlow Max=75.65 cfs @ 13.78 hrs HW=525.54' TW=516.07' (Dynamic Tailwater) 1=Broad-Crested Rectangular Weir (Weir Controls 16.77 cfs @ 4.4 fps) 2=Sharp-Crested Vee/Trap Weir (Weir Controls 12.66 cfs @ 2.8 fps) -3=Sharp-Crested Vee/Trap Weir (Weir Controls 46.22 cfs @ 1.7 fps)

## Pond p18:

Field Note #46 Golf pond Geometry to be confirmed by surveyed

Inflow Area	=	139.117 ac, Inflow Depth = $2.40$ "	for 25-yr event
Inflow =	=	81.53 cfs @ 13.78 hrs, Volume=	27.788 af
Outflow =	=	81.31 cfs @ 13.80 hrs, Volume=	27.783 af, Atten= 0%, Lag= 1.7 min
Primary =	=	81.31 cfs @ 13.80 hrs, Volume=	27.783 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 513.90' Surf.Area= 20,680 sf Storage= 26,884 cf Peak Elev= 516.07' @ 13.80 hrs Surf.Area= 28,776 sf Storage= 80,103 cf (53,219 cf above start) Flood Elev= 514.81' Surf.Area= 23,768 sf Storage= 48,709 cf (21,825 cf above start) Plug-Flow detention time= 47.0 min calculated for 27.161 af (98% of inflow) Center-of-Mass det. time= 27.3 min (976.0 - 948.7)

#Inve	ert Avail.S	Avail.Storage Storage Description			
1 510.0	00' 148,	288 cf Custom S	tage Data (Conic)	Listed below	
Elevation (feet)	Surf.Area (sq-ft)	(cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
510.00 513.90	0 20,680	0 26,884	0 26,884	0 20,704	
513.90	20,680		28,952	20,756	
516.00	28,290	48,782	77,735	28,436	
518.00	42,760	70,554	148,288	42,967	
# Routing	Invert	Outlet Devices			
1 Primary	513.90'	<b>2.0' long x 0.5' br</b> Head (feet) 0.20 Coef. (English) 2.	0.40 0.60 0.80 1	.00	Weir
2 Primary 3 Primary		143.0 deg Sharp-Crested Vee/Trap Weir C= 2.47 175.0 deg x 10.0' long Sharp-Crested Vee/Trap Weir C= 2.46			

Primary OutFlow Max=81.30 cfs @ 13.80 hrs HW=516.07' TW=510.86' (Dynamic Tailwater)

-1=Broad-Crested Rectangular Weir (Weir Controls 21.18 cfs @ 4.9 fps) -2=Sharp-Crested Vee/Trap Weir (Weir Controls 13.08 cfs @ 2.8 fps)

-3=Sharp-Crested Vee/Trap Weir (Weir Controls 13.06 cfs @ 2.3 fps)

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### Pond p19:

Wetland Geometry to be confirmed by survey Based off aerial topo, and assumed topo contour

Inflow Area =	15.520 ac, Inflow Depth = 1.86"	for 25-yr event
Inflow =	15.56 cfs @ 12.61 hrs, Volume=	2.401 af
Outflow =	9.63 cfs @ 13.01 hrs, Volume=	2.400 af, Atten= 38%, Lag= 24.1 min
Primary =	9.63 cfs @ 13.01 hrs, Volume=	2.400 af
Secondary =	0.00 cfs @ 0.00 hrs, Volume=	0.000 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 972.00' Surf.Area= 86,000 sf Storage= 57,333 cf Peak Elev= 972.21' @ 13.01 hrs Surf.Area= 91,873 sf Storage= 81,241 cf (23,907 cf above start) Plug-Flow detention time= 386.6 min calculated for 1.083 af (45% of inflow) Center-of-Mass det. time= 65.1 min (960.9 - 895.8)

#	Invert	Avail.St	torage Storage De	escription		
1	970.00'	282,	329 cf Custom S	tage Data (Conic)	Listed below	
Elevat	tion eet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft <u>)</u>	
970		0	0	0	0	
972		86,000	57,333	57,333	86,006	
974	.00	141,270	224,996	282,329	141,327	
#	Routing	Invert	Outlet Devices			
1 5	Secondary	973.60'	178.0 deg x 51.0'	long Sharp-Creste	ed Vee/Trap Weir	C= 2.46

2 Primary 972.00' **35.0' long x 0.5' breadth Broad-Crested Rectangular Weir** Head (feet) 0.20 0.40 0.60 0.80 1.00 Coef. (English) 2.80 2.92 3.08 3.30 3.32

Primary OutFlow Max=9.63 cfs @ 13.01 hrs HW=972.21' TW=970.18' (Dynamic Tailwater) ←2=Broad-Crested Rectangular Weir (Weir Controls 9.63 cfs @ 1.3 fps)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=972.00' TW=973.60' (Dynamic Tailwater)

## Pond p20:

Field Note #50 Spring Fed Pond Geometry to be confirmed by surveyed

 Inflow Area =
 241.484 ac, Inflow Depth =
 1.97" for 25-yr event

 Inflow =
 74.91 cfs @
 14.74 hrs, Volume=
 39.732 af

 Outflow =
 74.47 cfs @
 14.84 hrs, Volume=
 38.888 af, Atten= 1%, Lag= 6.3 min

 Primary =
 74.47 cfs @
 14.84 hrs, Volume=
 38.888 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

Existing Conditions_10454-01	Type III 24-hr 25-yr Rainfall=5.90"
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Starting Elev= 505.10' Surf.Area= 89,370 sf Storage= 138,524 cf Peak Elev= 506.34' @ 14.84 hrs Surf.Area= 91,074 sf Storage= 251,251 cf (112,727 cf above start) Plug-Flow detention time= 222.9 min calculated for 35.700 af (90% of inflow) Center-of-Mass det. time= 57.6 min (1,273.6 - 1,216.0)

#	Invert	Avail.St	torage Storage D	escription	
1	502.00'	615,	682 cf Custom S	tage Data (Prism	atic) Listed below
		<b>•</b> • •			
	ration	Surf.Area		Cum.Store	
	(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)	
50	02.00	0	0	0	
50	05.10	89,370	138,524	138,524	
50	06.00	89,380	80,437	218,961	
50	08.00	99,280	188,660	407,621	
5	10.00	108,781	208,061	615,682	
#	Routing	Invert	Outlet Devices		
1	Primary	505.10'	3.0' long x 1.5' br	eadth Broad-Cre	sted Rectangular Weir
	-		Head (feet) 0.20	0.40 0.60 0.80	1.00 1.20 1.40 1.60 1.80 2.00 2.50
			3.00		
			Coef. (English) 2.	62 2.64 2.64 2.6	68 2.75 2.86 2.92 3.07 3.07 3.03 3.28
			3.32		
2	Primary	506.20'		eadth Broad-Cre	sted Rectangular Weir
	j		-		1.00 1.20 1.40 1.60 1.80 2.00 2.50
			3.00		
				62 2.64 2.64 2.6	68 2.75 2.86 2.92 3.07 3.07 3.03 3.28
			3.32		
3	Primary	506.00'		long Sharp-Cres	ted Vee/Trap Weir C= 2.46
	-		-		
			47 cfs @ 14.84 hrs		V=505.90' (Dynamic Tailwater)

-1=Broad-Crested Rectangular Weir (Weir Controls 8.99 cfs @ 2.4 fps)

-2=Broad-Crested Rectangular Weir (Weir Controls 0.91 cfs @ 1.0 fps)

-3=Sharp-Crested Vee/Trap Weir (Weir Controls 64.56 cfs @ 1.8 fps)

#### Pond p21:

Inflow Area =	489.305 ac, Inflow Depth = 2.32"	for 25-yr event
Inflow =	368.65 cfs @ 12.44 hrs, Volume=	94.664 af
Outflow =	33.57 cfs @ 20.68 hrs, Volume=	81.790 af, Atten= 91%, Lag= 494.5 min
Primary =	33.57 cfs @ 20.68 hrs, Volume=	81.790 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 485.88' @ 20.68 hrs Surf.Area= 1,229,812 sf Storage= 2,375,071 cf Plug-Flow detention time= 809.0 min calculated for 81.790 af (86% of inflow) Center-of-Mass det. time= 679.1 min (1,711.4 - 1,032.4)

#	Invert	Avail.Storage	Storage Description
1	480.40'	5,244,885 cf	Custom Stage Data (Conic) Listed below

## Existing Conditions\_10454-01

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Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
0	0	0	0
202,230	107,856	107,856	202,234
485,198	667,114	774,970	485,231
1,275,481	1,698,237	2,473,208	1,275,541
1,499,208	2,771,678	5,244,885	1,499,423
Invert Ou	utlet Devices		
	(sq-ft) 0 202,230 485,198 1,275,481 1,499,208	(sq-ft)         (cubic-feet)           0         0           202,230         107,856           485,198         667,114           1,275,481         1,698,237           1,499,208         2,771,678	(sq-ft)(cubic-feet)(cubic-feet)000202,230107,856107,856485,198667,114774,9701,275,4811,698,2372,473,2081,499,2082,771,6785,244,885

1 Primary 480.40' **30.0" x 70.0' long Culvert** CMP, projecting, no headwall, Ke= 0.900 Outlet Invert= 480.40' S= 0.0000 '/' n= 0.024 Cc= 0.900

Primary OutFlow Max=33.57 cfs @ 20.68 hrs HW=485.88' TW=0.00' (Dynamic Tailwater) ←1=Culvert (Barrel Controls 33.57 cfs @ 6.8 fps)

#### Pond p22:

Field Note #54 Golf Pond Geometry to be confirmed by survey

Inflow Are	a =	97.943 ac, Inflow Depth = 2.52" for 25-yr event
Inflow	=	140.35 cfs @ 12.45 hrs, Volume= 20.533 af
Outflow	=	139.51 cfs @ 12.49 hrs, Volume= 20.234 af, Atten= 1%, Lag= 2.3 min
Primary	=	139.51 cfs @ 12.49 hrs, Volume= 20.234 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 498.10' Surf.Area= 6,520 sf Storage= 10,106 cf Peak Elev= 502.28' @ 12.49 hrs Surf.Area= 11,957 sf Storage= 47,903 cf (37,797 cf above start) Plug-Flow detention time= 32.2 min calculated for 19.998 af (97% of inflow) Center-of-Mass det. time= 14.4 min (891.6 - 877.2)

#	Invert	Avail.St	orage Stor	age De	escription	
1	495.00'	143,	770 cf <b>Cus</b>	tom St	age Data (Prisma	atic) Listed below
Elevatio	on	Surf.Area	-	Store	Cum.Store	
(fee	et)	(sq-ft)	(cubic-	feet)	(cubic-feet)	
495.0	00	0		0	0	
498.1	0	6,520	10	),106	10,106	
500.0	00	8,390	14	I,164	24,270	
502.0	00	11,530	19	9,920	44,190	
504.0	00	14,530	26	6,060	70,250	
506.0	00	18,340	32	2,870	103,120	
508.0	00	22,310	40	),650	143,770	
# Ro	outing	Invert	Outlet Devic	es		
1 Pr	imary	499.75'	18.0" x 21.0	)' long	Culvert CMP, p	rojecting, no headwall, Ke= 0.900
			<b>Outlet Inver</b>	t= 499.	75' S= 0.0000 '/'	n= 0.024 Cc= 0.900
2 Pr	rimary	500.50'	1.0' long x	15.0' b	readth Broad-Cre	ested Rectangular Weir
			Head (feet)	0.20 (	0.40 0.60 0.80 1	.00 1.20 1.40 1.60
			Coef. (Engli	sh) 2.6	68 2.70 2.70 2.6	4 2.63 2.64 2.64 2.63

Existing Conditions_10454-01	Type III 24-hr 25-yr Rainfall=5.90"
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20.0' long x 13.5' breadth Broad-Crested Rectangular Weir 3 Primary 500.50' Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.62 2.66 2.70 2.66 2.65 2.66 2.65 2.63

Primary OutFlow Max=139.49 cfs @ 12.49 hrs HW=502.28' TW=500.55' (Dynamic Tailwater) -1=Culvert (Barrel Controls 8.05 cfs @ 4.6 fps) -2=Broad-Crested Rectangular Weir (Weir Controls 6.26 cfs @ 3.5 fps)

-3=Broad-Crested Rectangular Weir (Weir Controls 125.18 cfs @ 3.5 fps)

#### Pond p23:

Inflow Are	a =	41.587 ac, In	flow Depth = 2.91	for 25-yr event	
Inflow	=	63.82 cfs @	12.68 hrs, Volume=	= 10.091 af	
Outflow	=	63.81 cfs @	12.68 hrs, Volume=	= 9.172 af,	Atten= 0%, Lag= 0.1 min
Primary	=	63.81 cfs @	12.68 hrs, Volume=	= 9.172 af	-

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 507.93' @ 12.68 hrs Surf.Area= 19,168 sf Storage= 43,515 cf Plug-Flow detention time= 64.4 min calculated for 9.170 af (91% of inflow) Center-of-Mass det. time= 19.2 min (891.5 - 872.4)

#	Invert	Avail.St	orage Sto	rage De	escription	
1	503.50'	100,	303 cf Cus	stom St	age Data (Prisma	atic) Listed below
Elevat (fe	tion eet)	Surf.Area (sq-ft)		Store -feet)	Cum.Store (cubic-feet)	
503	8.50	0		0	0	
506	5.00	11,170	1:	3,963	13,963	
508	8.00	19,460	3	0,630	44,593	
510	0.00	36,250	5	5,710	100,303	
	Routing	Invert	Outlet Devi			
1 1	Primary	507.70'	178.0 deg	x 178.0'	' long Sharp-Cre	sted Vee/Trap Weir C= 2.46

Primary OutFlow Max=63.81 cfs @ 12.68 hrs HW=507.93' TW=507.26' (Dynamic Tailwater) **1=Sharp-Crested Vee/Trap Weir** (Weir Controls 63.81 cfs @ 1.5 fps)

## Pond zDP1: Design Point 1

Field note #10. Culvert dimensions to be confirmed by survey.

Inflow Area	ι =	26.659 ac, I	nflow Depth = 2.60"	for 25-yr event	
Inflow	=	40.11 cfs @	12.48 hrs, Volume=	5.767 af	
Outflow	=	40.11 cfs @	12.48 hrs, Volume=	5.767 af,	Atten= 0%, Lag= 0.1 min
Primary	=	40.11 cfs @	12.48 hrs, Volume=	5.767 af	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 722.62' @ 12.48 hrs Surf.Area= 168 sf Storage= 182 cf Flood Elev= 727.00' Surf.Area= 1,105 sf Storage= 2,619 cf Plug-Flow detention time= 0.1 min calculated for 5.766 af (100% of inflow)

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#	Invert	Avail.St	orage S <sup>.</sup>	torage Des	scription		
1	720.10'	3,	706 cf <b>C</b>	ustom Sta	ige Data (Conic)	Listed below	
Eleva (1	ation feet)	Surf.Area (sq-ft)		c.Store bic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft <u>)</u>	
72	0.10	0		0	0	0	
72	2.00	90		57	57	96	
72	4.00	340		403	460	364	
72	6.00	760		1,072	1,533	815	
72	8.00	1,450		2,173	3,706	1,543	
#	Routing	Invert	Outlet De	vices			
1	Primary	720.10'	42.0" x 1	20.0' long	Culvert CMP, s	quare edge head	wall, Ke= 0.500
2	Primary	727.00'	Outlet Invert= 700.00' S= 0.1675 '/' n= 0.024 Cc= 0.900 155.0 deg Sharp-Crested Vee/Trap Weir C= 2.47				

Center-of-Mass det. time= 0.1 min (870.8 - 870.7)

Primary OutFlow Max=40.10 cfs @ 12.48 hrs HW=722.62' TW=686.76' (Dynamic Tailwater) -1=Culvert (Inlet Controls 40.10 cfs @ 5.4 fps) -2=Sharp-Crested Vee/Trap Weir (Controls 0.00 cfs)

Pond zDP2: Design Point 2

Field Note #15 Culvert dimensions to be confirmed by survey Overflow to ditch is currently discarded... We may have to model that area...

Inflow Area =	97.712 ac, Inflow Depth = 2.28"	
Inflow =	98.45 cfs @ 12.87 hrs, Volume=	18.556 af
Outflow =	98.40 cfs @ 12.88 hrs, Volume=	18.556 af, Atten= 0%, Lag= 0.4 min
Discarded =	62.99 cfs @ 12.88 hrs, Volume=	4.623 af
Primary =	35.41 cfs @ 12.88 hrs, Volume=	13.933 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 626.08' @ 12.88 hrs Surf.Area= 1,467 sf Storage= 3,498 cf Flood Elev= 624.50' Surf.Area= 925 sf Storage= 1,728 cf Plug-Flow detention time= 0.5 min calculated for 18.552 af (100% of inflow) Center-of-Mass det. time= 0.5 min (902.9 - 902.4)

#	Invert	Avail.Storag	e Storage Des	scription	
1	619.60'	7,280 c	of Custom Sta	ge Data (Conic) Li	sted below
Elevati		Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft <u>)</u>
619.	60	0	0	0	0
620.	00	10	1	1	10
622.	00	260	214	215	269
624.	00	760	976	1,192	793
626.	00	1,420	2,146	3,338	1,492
628.	00	2,580	3,943	7,280	2,694

	Routing	Invert	Outlet Devices
1	Primary	619.60'	24.0" x 150.0' long Culvert RCP, end-section conforming to fill, Ke= 0.500
			Outlet Invert= 608.00' S= 0.0773 '/' n= 0.012 Cc= 0.900
2	Discarded	624.50'	166.0 deg Sharp-Crested Vee/Trap Weir C= 2.46

Discarded OutFlow Max=62.97 cfs @ 12.88 hrs HW=626.08' (Free Discharge) ←2=Sharp-Crested Vee/Trap Weir (Weir Controls 62.97 cfs @ 3.1 fps)

Primary OutFlow Max=35.41 cfs @ 12.88 hrs HW=626.08' TW=607.68' (Dynamic Tailwater) ☐ 1=Culvert (Inlet Controls 35.41 cfs @ 11.3 fps)

## Pond zDP3: Design Point 3

Inflow Are	ea =	212.742 ac, I	nflow Depth = 19.80"	for 25-yr event	
Inflow	=	249.61 cfs @	12.44 hrs, Volume=	351.023 af	
Primary	=	249.61 cfs @	12.44 hrs, Volume=	351.023 af,	Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

#### Pond zDP4: Design Point 4

Inflow Area	a =	489.305 ac, Inflow Depth = 2.01"	for 25-yr event
Inflow	=	33.57 cfs @ 20.68 hrs, Volume=	= 81.790 af
Primary	=	33.57 cfs @ 20.68 hrs, Volume=	= 81.790 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

## Pond zDP5: Design Point 5

Inflow Area	a =	28.325 ac, Inflow Depth = 2.73	for 25-yr event
Inflow	=	50.24 cfs @ 12.45 hrs, Volume	= 6.435 af
Primary	=	50.24 cfs @ 12.45 hrs, Volume	= 6.435 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

# Pre-Development Conditions 50 year 24 hour Storm Event Model Computations

Existing Conditions_10454-01Type III 24-hr 50-yrRainfall=6.70"Prepared by The Chazen CompaniesPage 133HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems4/10/2006 2:33:52 PM			
Subcatchment s01:			
Runoff = 20.20 cfs @ 12.60 hrs, Volume= 3.033 af, Depth= 3.17"			
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 50-yr Rainfall=6.70"			
Area (ac) CN Description 11.485 68			
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)			
42.8 Direct Entry,			
Subcatchment s02:			
Runoff = 125.74 cfs @ 12.87 hrs, Volume= 23.389 af, Depth= 2.87"			
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 50-yr Rainfall=6.70"			
Area (ac) CN Description 97.712 65			
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)			
61.3 Direct Entry,			
Subcatchment s03:			
Runoff = 33.32 cfs @ 12.41 hrs, Volume= 4.134 af, Depth= 3.27"			
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 50-yr Rainfall=6.70"			
Area (ac) CN Description			
15.174 69			
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)			
28.8 Direct Entry,			
Subcatchment s04:			
Runoff = 37.27 cfs @ 12.10 hrs, Volume= 2.730 af, Depth= 2.87"			
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 50-yr Rainfall=6.70"			

Existing Conditions_10454-01	Type III 24-hr 50-yr Rainfall=6.70"		
Prepared by The Chazen Companies HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Syster	Page 134 ms 4/10/2006 2:33:53 PM		
	10 10/2000 2.00.00 T M		
Area (ac) CN Description			
11.403 65			
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)			
6.5 Direct Entry,			
Subcatchment s05:			
Runoff = 30.05 cfs @ 12.25 hrs, Volume= 3.096 af	, Depth= 2.49"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, Type III 24-hr 50-yr Rainfall=6.70"	dt= 0.01 hrs		
Area (ac) CN Description			
14.935 61			
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)			
17.3 Direct Entry,			
Subcatchment s06:			
Subcatchment Soo.			
Runoff = 18.93 cfs @ 12.25 hrs, Volume= 1.938 af, Depth= 2.58"			
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 50-yr Rainfall=6.70"			
Area (ac) CN Description			
9.007 62			
Tc Length Slope Velocity Capacity Description			
(min) (feet) (ft/ft) (ft/sec) (cfs) 17.3 Direct Entry,			
Subcatchment s06(OW): s06 Open Water			
Runoff = 3.44 cfs @ 12.00 hrs, Volume= 0.239 af	, Depth= 6.70"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 50-yr Rainfall=6.70"			
Area (ac) CN Description			
0.428 100			

Existing Conditions_10454-01 Ty Prepared by The Chazen Companies HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems	pe III 24-hr 50-yr Rainfall=6.70" Page 135 4/10/2006 2:33:53 PM		
Subcatchment s07:			
Runoff = 16.90 cfs @ 12.20 hrs, Volume= 1.575 af, D	Depth= 2.78"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt Type III 24-hr 50-yr Rainfall=6.70"	= 0.01 hrs		
Area (ac) CN Description 6.811 64			
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)			
13.9 Direct Entry,			
Subcatchment s07(OW): s07 Open V	Vater		
Runoff = 4.07 cfs @ 12.00 hrs, Volume= 0.283 af, D	Depth= 6.70"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt Type III 24-hr 50-yr Rainfall=6.70"	= 0.01 hrs		
Area (ac) CN Description 0.506 100			
Subcatchment s08:			
Runoff = 50.48 cfs @ 12.33 hrs, Volume= 5.835 af, D	Depth= 2.21"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 50-yr Rainfall=6.70"			
Area (ac) CN Description			
31.719 58			
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)			
21.8 Direct Entry,			
Subcatchment s09:			
Runoff = 20.48 cfs @ 12.23 hrs, Volume= 2.023 af, D	Depth= 2.87"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 50-yr Rainfall=6.70"			
Area (ac) CN Description 8.452 65			

Existing Conditions_10454-01	Type III 24-hr 50-yr Rainfall=6.70"		
Prepared by The Chazen Companies       Page 1         HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems       4/10/2006 2:33:53 F			
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)			
16.4Direct Entry,			
Subcatchment s10:			
Runoff = 16.93 cfs @ 12.40 hrs, Volume= 2.080 a	f, Depth= 3.07"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs Type III 24-hr 50-yr Rainfall=6.70"	s, dt= 0.01 hrs		
Area (ac) CN Description 8.130 67			
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)			
27.9 Direct Entry,			
Subcatchment s10(OW): s10 Ope	n Water		
Runoff = 6.68 cfs @ 12.00 hrs, Volume= 0.463 a	f, Depth= 6.70"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs Type III 24-hr 50-yr Rainfall=6.70"	s, dt= 0.01 hrs		
Area (ac) CN Description 0.830 100			
Subcatchment s11:			
Runoff = 6.02 cfs @ 12.28 hrs, Volume= 0.644 a	f, Depth= 3.27"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 50-yr Rainfall=6.70"			
Area (ac) CN Description			
2.364 69			
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)			
20.5 Direct Entry,			
Subcatchment s11(IC): s11 Imp. Cover			
Runoff = 17.23 cfs @ 12.04 hrs, Volume= 1.259 a	f, Depth= 6.46"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 50-yr Rainfall=6.70"			

Existing Conditions_10454-01	Type III 24-hr 50-yr Rainfall=6.70"		
Prepared by The Chazen Companies HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Syste	Page 137 ems 4/10/2006 2:33:53 PM		
Area (ac) CN Description 2.338 98			
2.000 00			
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)			
2.8 Direct Entry,			
Subcatchment s12:			
Runoff = 6.45 cfs @ 12.65 hrs, Volume= 1.036 at	f, Depth= 1.94"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs Type III 24-hr 50-yr Rainfall=6.70"	, dt= 0.01 hrs		
Area (ac) CN Description			
6.420 55			
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)			
42.0 Direct Entry,			
Subcatchment s13:			
Runoff = 0.67 cfs @ 12.13 hrs, Volume= 0.056 af, Depth= 1.94"			
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 50-yr Rainfall=6.70"			
Area (ac) CN Description			
0.350 55			
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)			
8.6 Direct Entry,			
Subcatchment s14:			
Runoff = 33.60 cfs @ 12.40 hrs, Volume= 4.178 at	f, Depth= 2.78"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 50-yr Rainfall=6.70"			
Area (ac) CN Description			
18.066 64			
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)			
28.1 Direct Entry,			

Existing Conditions_10454-01Type III 24-hr 50-yrRainfall=6.70"Prepared by The Chazen CompaniesPage 138HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems4/10/2006 2:33:53 PM			
Subcatchment s14(IC): s14 Imp. Cover			
Runoff = 17.86 cfs @ 12.03 hrs, Volume= 1.281 af, Depth= 6.46"			
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 50-yr Rainfall=6.70"			
Area (ac) CN Description 2.380 98			
Tc Length Slope Velocity Capacity Description			
(min) (feet) (ft/ft) (ft/sec) (cfs)			
2.3 Direct Entry,			
Subcatchment s14(OW): s14 Open Water			
Runoff = 4.17 cfs @ 12.00 hrs, Volume= 0.289 af, Depth= 6.70"			
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 50-yr Rainfall=6.70"			
Area (ac) CN Description			
0.518 100			
Subcatchment s15:			
Runoff = 1.36 cfs @ 12.21 hrs, Volume= 0.141 af, Depth= 1.59"			
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 50-yr Rainfall=6.70"			
Area (ac) CN Description			
1.068 51			
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)			
13.1Direct Entry,			
Subcatchment s16:			
Runoff = 150.73 cfs @ 12.32 hrs, Volume= 16.832 af, Depth= 2.97"			
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 50-yr Rainfall=6.70"			
Area (ac) CN Description			
67.994 66			

Existing Conditions_10454-01	Type III 24-hr 50-yr Rainfall=6.70"			
Prepared by The Chazen CompaniesPageHydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems4/10/2006 2:33:53				
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)				
22.3 Direct Entry,				
Subcatchment s16(IC): s16 Imp	.Cover			
	f, Depth= 6.46"			
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs Type III 24-hr 50-yr Rainfall=6.70"	s, dt= 0.01 hrs			
Area (ac) CN Description				
2.629 98				
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)				
2.9Direct Entry,				
Subcatchment s16(OW): s16 Ope	n Water			
Runoff = 43.04 cfs @ 12.00 hrs, Volume= 2.988 a	f, Depth= 6.70"			
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 50-yr Rainfall=6.70"				
Area (ac) CN Description				
5.351 100				
Subcatchment s17:				
Runoff = 96.16 cfs @ 13.77 hrs, Volume= 28.673 a	f, Depth= 2.97"			
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 50-yr Rainfall=6.70"				
Area (ac) CN Description				
115.827 66				
Tc Length Slope Velocity Capacity Description				
(min) (feet) (ft/ft) (ft/sec) (cfs) 125.2 Direct Entry,				
Subcatchment s17(OW): s17 Ope	in water			
Runoff = 1.32 cfs @ 12.00 hrs, Volume= 0.092 a	f, Depth= 6.70"			
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs	s, dt= 0.01 hrs			

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 50-yr Rainfall=6.70"

Existing Conditions_10454-01 Prepared by The Chazen Companies	<i>Type III 24-hr 50-yr Rainfall=6.70"</i> Page 140		
HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Syste			
Area (ac) CN Description			
0.164 100			
Subastaburant a10			
Subcatchment s18:			
Runoff = 50.08 cfs @ 12.34 hrs, Volume= 5.795 at	, Depth= 3.07"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs Type III 24-hr 50-yr Rainfall=6.70"	, dt= 0.01 hrs		
Area (ac) CN Description			
22.654 67			
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)			
24.4Direct Entry,			
Subcatchment s18(OW): s18 Ope	n Water		
Runoff = 3.80 cfs @ 12.00 hrs, Volume= 0.264 at	, Depth= 6.70"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs Type III 24-hr 50-yr Rainfall=6.70"	, dt= 0.01 hrs		
Area (ac) CN Description			
0.472 100			
Subcatchment s19:			
Runoff = 20.51 cfs @ 12.61 hrs, Volume= 3.096 at	, Depth= 2.39"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 50-yr Rainfall=6.70"			
Area (ac) CN Description			
15.520 60			
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)			
40.4 Direct Entry,			
Subcatchment s20:			
Runoff = 38.07 cfs @ 12.47 hrs, Volume= 5.083 at	, Depth= 3.27"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs	, dt= 0.01 hrs		

Type III 24-hr 50-yr Rainfall=6.70"

Existing Conditions_10454-01	Type III 24-hr 50-yr Rainfall=6.70"			
Prepared by The Chazen Companies HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Sy	Page 141 stems 4/10/2006 2:33:53 PM			
Area (ac) CN Description				
Area (ac) CN Description 18.655 69				
To Longth Slope Valacity Conscity Description				
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)				
33.9 Direct Entry,				
Subcatchment s20(OW): s20 O	pen Water			
Runoff = 15.83 cfs @ 12.00 hrs, Volume= 1.099	9 af, Depth= 6.70"			
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 H Type III 24-hr 50-yr Rainfall=6.70"	nrs, dt= 0.01 hrs			
Area (ac) CN Description 1.968 100				
Subcatchment s21:				
Runoff = 211.45 cfs @ 12.37 hrs, Volume= 25.368	3 af, Depth= 3.17"			
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 H Type III 24-hr 50-yr Rainfall=6.70"	nrs, dt= 0.01 hrs			
Area (ac) CN Description 96.056 68				
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)				
26.7 Direct Entry,				
Subcatchment s21(OW)	):			
Runoff = 98.41 cfs @ 12.00 hrs, Volume= 6.831	l af, Depth= 6.70"			
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 H Type III 24-hr 50-yr Rainfall=6.70"	nrs, dt= 0.01 hrs			
Area (ac) CN Description				
12.235 100				
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)				
0.0 Direct Entry,				

Existing Conditions_10454-01Type III 24-hr 50-yrRainfall=6.70"Prepared by The Chazen CompaniesPage 142HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems4/10/2006 2:33:53 PM				
Subcatchment s22:				
Runoff = 173.94 cfs @ 12.45 hrs, Volume= 22.420 af, Depth= 3.27"				
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 50-yr Rainfall=6.70"				
Area (ac) CN Description 82.287 69				
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)				
31.3 Direct Entry,				
Subcatchment s22(OW): s22 Open Water				
Runoff = 1.09 cfs @ 12.00 hrs, Volume= 0.076 af, Depth= 6.70"				
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 50-yr Rainfall=6.70"				
Area (ac) CN Description				
0.136 100				
Subcatchment s23:				
Runoff = 78.66 cfs @ 12.64 hrs, Volume= 12.389 af, Depth= 3.57"				
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 50-yr Rainfall=6.70"				
Area (ac) CN Description				
41.587 72				
Tc         Length         Slope         Velocity         Capacity         Description           (min)         (feet)         (ft/ft)         (ft/sec)         (cfs)				
47.0 Direct Entry,				
Subcatchment s24:				
Runoff = 62.42 cfs @ 12.45 hrs, Volume= 7.956 af, Depth= 3.37"				
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 50-yr Rainfall=6.70"				
Area (ac) CN Description 28.325 70				

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Prepare HydroCA						ies 2003 Applie	d Micr	ocomp	outer Sy	stems		4/10/2	006	Page 14 2:33:53 Pl
Tc (min)	Leng (fee		Slope (ft/ft)		ocity sec)	Capacity (cfs)	Des	criptio	n					
30.7	(15)	-,	(1210)	(14		(0.0)	Dire	ct En	try,					
						Subc	atchr	nent	s25:					
Runoff	=	3	1.32 cfs	s @	12.28	3 hrs, Volu	ume=		3.357	af, De	pth= 2	.97"		
Runoff b Type III						CS, Time	Span=	= 0.00 <sup>.</sup>	-48.00 ŀ	nrs, dt= (	0.01 hr:	S		
Area	(ac)	CN	Dese	criptic	on									
13	.562	66												
Tc (min)	Leng (fee		Slope (ft/ft)		ocity sec)	Capacity (cfs)	Des	criptio	n					
20.2							Dire	ct En	t <b>ry</b> ,					
20.2									•					
20.2						F	Reach	n <b>r03</b> :						
Overland Requires						F		n r03:						
Overlan	s more	surv 1 20	ey 1.485 a 0.20 cfs	s @	12.60	F Depth = 3 ) hrs, Volu 3 hrs, Volu	<b>Reach</b> 3.17" ume=		50-yr ev 3.033	af	en= 0%	5, Lag= 1	.5 m	in
Overland Requires Inflow A Inflow Outflow Routing Max. Ve	rea = = = by Dyr locity=	surv 1 20 20 5.8 f	ey 1.485 a 0.20 cfs 0.10 cfs r-Ind m ps, Mi	s @ s @ ethoc n. Tra	12.60 12.63 d, Tim avel Ti	Depth = 3 ) hrs, Volu	Reach 3.17" ume= ume= 0.00-48 nin	for §	50-yr ev 3.033 3.033	af af, Atte		o, Lag= 1	.5 mi	in
Overland Requires Inflow A Inflow Outflow Routing Max. Ve Avg. Vel Peak De Capacity Inlet Inve	s more rea = = by Dyr locity= ocity = ocity = ocity = ocity = at bar ort = 84	surv 1 20 5.8 f 2.2 1 .74' ( nk ful 5.00'	ey 1.485 a 0.20 cfs 0.10 cfs r-Ind m ps, Min fps, Av @ 12.6 I= 92.1 , Outle	s @ s @ ethoc n. Tra rg. Tra g. Tra 3 hrs 4 cfs et Inve	12.60 12.63 d, Tim avel T avel T avel T	Depth = 3 ) hrs, Volu 3 hrs, Volu e Span= 0 ime= 2.2 r ime= 5.9	<b>Reach</b> 3.17" ume= ume= 0.00-48 nin min	for 8 3.00 h	50-yr ev 3.033 3.033 rs, dt= 0	af af, Atte			.5 mi	in
Overland Requires Inflow A Inflow Outflow Routing Max. Ve Avg. Vel Peak De Capacity Inlet Inve	s more rea = = by Dyr locity= ocity = ocity = ocity = ocity = at bar ort = 84	surv 1 20 5.8 f 2.2 1 .74' ( nk ful 5.00'	ey 1.485 a 0.20 cfs 0.10 cfs r-Ind m ps, Min fps, Av @ 12.6 I= 92.1 , Outle	s @ s @ ethoc n. Tra rg. Tra g. Tra 3 hrs 4 cfs et Inve	12.60 12.63 d, Tim avel T avel T avel T	Depth = 3 ) hrs, Volu 3 hrs, Volu e Span= 0 ime= 2.2 r ime= 5.9 f 28.00' nel, n= 0.	<b>Reach</b> 3.17" ume= ume= 0.00-48 nin min	for \$ 3.00 h	50-yr ev 3.033 3.033 rs, dt= 0	af af, Atte			.5 mi	in
Overland Requires Inflow A Inflow Outflow Routing Max. Ve Avg. Vel Peak De Capacity Inlet Inve	s more rea = = by Dyr locity= ocity = ocity = rat bai ert= 84 1.50'	surv 1 20 5.8 f 2.2 1 .74' ( nk ful 5.00'	ey 1.485 a 0.20 cfs 0.10 cfs r-Ind m ps, Min fps, Av @ 12.6 I= 92.1 , Outle	s @ s @ ethoc n. Tra rg. Tra g. Tra 3 hrs 4 cfs et Inve	12.60 12.63 d, Tim avel T avel T avel T	Depth = 3 ) hrs, Volu 3 hrs, Volu e Span= 0 ime= 2.2 r ime= 5.9 f 28.00' nel, n= 0.	Reach 3.17" ume= ume= 0.00-48 nin min	for \$ 3.00 h	50-yr ev 3.033 3.033 rs, dt= 0	af af, Atte			.5 mi	in

Avg. Velocity = 2.8 fps, Avg. Travel Time= 4.0 min

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Peak Depth= 1.40' @ 12.50 hrs Capacity at bank full= 446.15 cfs Inlet Invert= 685.50', Outlet Invert= 608.00' 12.00' x 4.00' deep Parabolic Channel, n= 0.060 Length= 675.0' Slope= 0.1148 '/'

#### Reach r08a:

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Man Made Ditch Inverts of pipe to be surveyed

Inflow Are	a =	97.712 ac, Inflo	w Depth = 1.98"	for 50-yr event	
Inflow	=	36.18 cfs @ 12	.87 hrs, Volume=	16.122 af	
Outflow	=	36.18 cfs @ 12	.88 hrs, Volume=	16.122 af,	Atten= 0%, Lag= 0.2 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 9.6 fps, Min. Travel Time= 0.4 min Avg. Velocity = 5.6 fps, Avg. Travel Time= 0.7 min

Peak Depth= 0.68' @ 12.88 hrs Capacity at bank full= 81.88 cfs Inlet Invert= 607.00', Outlet Invert= 587.00' 10.00' x 1.00' deep Parabolic Channel, n= 0.027 Length= 226.0' Slope= 0.0885 '/'

#### Reach r08b:

24" HDPE Inverts to be surveyed

Inflow Area = 97.712 ac, Inflow Depth = 1.98"for 50-yr event Inflow = 36.18 cfs @ 12.88 hrs, Volume= 16.122 af Outflow 36.18 cfs @ 12.88 hrs, Volume= 16.122 af, Atten= 0%, Lag= 0.1 min =

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 24.2 fps, Min. Travel Time= 0.2 min Avg. Velocity = 14.7 fps, Avg. Travel Time= 0.3 min

Peak Depth= 0.96' @ 12.88 hrs Capacity at bank full= 77.17 cfs Inlet Invert= 587.00', Outlet Invert= 557.75' 24.0" Diameter Pipe n= 0.012 Length= 295.0' Slope= 0.0992 '/'

#### Reach r08c:

Ditch Pipe inverts to be surveyed

Inflow Area = 97.712 ac, Inflow Depth = 1.98" for 50-yr event Inflow 36.18 cfs @ 12.88 hrs, Volume= 16.122 af = 36.17 cfs @ 12.89 hrs, Volume= Outflow 16.122 af, Atten= 0%, Lag= 0.6 min =

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 9.2 fps, Min. Travel Time= 1.1 min Avg. Velocity = 5.1 fps, Avg. Travel Time= 1.9 min

Peak Depth= 0.71' @ 12.89 hrs Capacity at bank full= 76.65 cfs Inlet Invert= 557.75', Outlet Invert= 512.00' 10.00' x 1.00' deep Parabolic Channel, n= 0.027 Length= 590.0' Slope= 0.0775 '/'

#### Reach r08d: Amenia Stream/Cascade Brook

(Steam) parallel to Route 22 Base Flow estimated from field observation (see field note 21)

97.712 ac, Inflow Depth = 21.47" for 50-yr event Inflow Area = Inflow 76.17 cfs @ 12.89 hrs, Volume= 174.833 af, Incl. 40.00 cfs Base Flow = 76.15 cfs @ 12.93 hrs, Volume= Outflow = 174.589 af, Atten= 0%, Lag= 2.3 min Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 3.6 fps, Min. Travel Time= 3.7 min Avg. Velocity = 3.1 fps, Avg. Travel Time= 4.3 min Peak Depth= 3.42' @ 12.93 hrs Capacity at bank full= 104.49 cfs Inlet Invert= 512.00', Outlet Invert= 504.00' 10.00' x 4.00' deep Parabolic Channel, n= 0.060 Length= 805.0' Slope= 0.0099 '/'

## Reach r14a:

Grass lined channel

 Inflow Area =
 8.452 ac, Inflow Depth =
 2.82" for 50-yr event

 Inflow =
 16.39 cfs @
 12.37 hrs, Volume=
 1.984 af

 Outflow =
 16.38 cfs @
 12.38 hrs, Volume=
 1.984 af, Atten= 0%, Lag= 0.6 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 6.9 fps, Min. Travel Time= 0.9 min Avg. Velocity = 1.7 fps, Avg. Travel Time= 3.6 min

Peak Depth= 0.72' @ 12.38 hrs Capacity at bank full= 325.42 cfs Inlet Invert= 542.00', Outlet Invert= 526.00' 10.00' x 3.00' deep Parabolic Channel, n= 0.027 Length= 360.0' Slope= 0.0444 '/'

#### Reach r14b:

30" HDPE Under Main Entrance Road

 Inflow Area =
 8.452 ac, Inflow Depth =
 2.82"
 for 50-yr event

 Inflow =
 16.38 cfs @
 12.38 hrs, Volume=
 1.984 af

 Outflow =
 16.38 cfs @
 12.39 hrs, Volume=
 1.984 af, Atten= 0%, Lag= 0.4 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 14.5 fps, Min. Travel Time= 0.5 min Avg. Velocity = 3.5 fps, Avg. Travel Time= 2.1 min

Peak Depth= 0.70' @ 12.39 hrs Capacity at bank full= 94.91 cfs Inlet Invert= 526.00', Outlet Invert= 505.70' 30.0" Diameter Pipe n= 0.012 Length= 445.0' Slope= 0.0456 '/'

#### Reach r14c:

#### **Overland Flow**

Inflow Area = Inflow = Outflow =	6.420 ac, Inflow Depth =0.99"for50-yr event1.30 cfs @14.35 hrs, Volume=0.532 af1.28 cfs @14.48 hrs, Volume=0.532 af, Atten= 1%, Lag= 7.8 min				
Max. Velocity= 1.2	tor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs 2 fps, Min. Travel Time= 8.2 min 5 fps, Avg. Travel Time= 20.0 min				
Peak Depth= $0.10'$ @ 14.48 hrs Capacity at bank full= 178.07 cfs Inlet Invert= 544.00', Outlet Invert= 498.00' 50.00' x 1.00' deep Parabolic Channel, n= 0.060 Length= 575.0' Slope= 0.0800 '/'					

## Reach r15:

Brush Overbanks with Rocky Bottom Needs to be surveyed

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 10.5 fps, Min. Travel Time= 0.5 min Avg. Velocity = 3.7 fps, Avg. Travel Time= 1.3 min

Peak Depth= 0.96' @ 12.06 hrs Capacity at bank full= 188.47 cfs Inlet Invert= 554.00', Outlet Invert= 528.00' 5.00' x 3.00' deep Parabolic Channel, n= 0.027 Length= 290.0' Slope= 0.0897 '/'

# Reach r16:

Pipe Reach

Inflow Area =       4.702 ac, Inflow Depth = 4.86" for 50-yr event         Inflow =       19.71 cfs @ 12.04 hrs, Volume=       1.903 af         Outflow =       19.23 cfs @ 12.06 hrs, Volume=       1.903 af, Atten= 2%, Lag= 0.9 min					
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 11.7 fps, Min. Travel Time= 1.2 min Avg. Velocity = 3.9 fps, Avg. Travel Time= 3.7 min					
Peak Depth= 0.92' @ 12.06 hrs Capacity at bank full= 66.05 cfs Inlet Invert= 573.00', Outlet Invert= 554.00' 30.0" Diameter Pipe n= 0.012 Length= 860.0' Slope= 0.0221 '/'					
Reach r18a:					
Overland Flow Reach					
Inflow         =         0.00 cfs @         0.00 hrs, Volume=         0.000 af           Outflow         =         0.00 cfs @         0.00 hrs, Volume=         0.000 af, Atten= 0%, Lag= 0.0 min					
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 0.0 fps, Min. Travel Time= 0.0 min Avg. Velocity = 0.0 fps, Avg. Travel Time= 0.0 min					
Peak Depth= 0.00' @ 0.00 hrs Capacity at bank full= 379.63 cfs Inlet Invert= 973.60', Outlet Invert= 530.00' 50.00' x 1.00' deep Parabolic Channel, n= 0.060 Length= 1,220.0' Slope= 0.3636 '/'					
Reach r18b:					
Overland Flow Reach					
Inflow         =         0.00 cfs @         0.00 hrs, Volume=         0.000 af           Outflow         =         0.00 cfs @         0.00 hrs, Volume=         0.000 af, Atten= 0%, Lag= 0.0 min					
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 0.0 fps, Min. Travel Time= 0.0 min Avg. Velocity = 0.0 fps, Avg. Travel Time= 0.0 min					
Peak Depth= $0.00'$ @ $0.00$ hrs Capacity at bank full= 151.94 cfs Inlet Invert= 530.60', Outlet Invert= 514.00' 50.00' x 1.00' deep Parabolic Channel, n= 0.060 Length= 285.0' Slope= 0.0582 '/'					

Existing Conditions\_10454-01

## Reach r21a:

Man Made Ditch

Inflow Area = Inflow = Outflow =	·
Max. Velocity=	-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs 8.3 fps, Min. Travel Time= 1.3 min 3.4 fps, Avg. Travel Time= 3.2 min
Capacity at ban Inlet Invert= 504	35' @ 14.42 hrs k full= 191.76 cfs ł.00', Outlet Invert= 494.00' deep Parabolic Channel, n= 0.027 Length= 648.0' Slope= 0.0154 '/'
	Reach r21b:
Grass Ditch Geometry to be	Reach r21b: confirmed by survey (inverts at pipe)
Geometry to be Inflow Area =	confirmed by survey (inverts at pipe) 97.943 ac, Inflow Depth = 3.10" for 50-yr event 175.37 cfs @ 12.49 hrs, Volume= 25.291 af

Peak Depth= 1.73' @ 12.49 hrs Capacity at bank full= 239.90 cfs Inlet Invert= 499.00', Outlet Invert= 491.10' 15.00' x 2.00' deep Parabolic Channel, n= 0.027 Length= 230.0' Slope= 0.0343 '/'

#### Reach r21c:

**Overland Flow Reach** 

Inflow Area = 41.587 ac, Inflow Depth =  $3.31^{"}$  for 50-yr event Inflow = 78.61 cfs @ 12.66 hrs, Volume= 11.471 afOutflow = 78.61 cfs @ 12.67 hrs, Volume= 11.471 af, Atten= 0%, Lag= 0.5 min Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrsMax. Velocity= 4.9 fps, Min. Travel Time= 0.5 minAvg. Velocity = 1.9 fps, Avg. Travel Time= 1.4 min

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Peak Depth= 0.61' @ 12.67 hrs Capacity at bank full= 227.81 cfs Inlet Invert= 506.70', Outlet Invert= 485.75' 50.00' x 1.00' deep Parabolic Channel, n= 0.060 Length= 160.0' Slope= 0.1309 '/'

#### Reach r22a:

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**Overland Flow Reach** 

Inflow Area = Inflow = Outflow =	15.520 ac, Inflow Depth =2.39"for 50-yr event13.60 cfs @12.96 hrs, Volume=3.094 af13.54 cfs @13.00 hrs, Volume=3.094 af, Atten= 0%, Lag= 2.8 min	
Max. Velocity= 4	Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs I.3 fps, Min. Travel Time= 3.8 min 1.3 fps, Avg. Travel Time= 12.5 min	
Peak Depth= 0.2 Capacity at bank Inlet Invert= 970.		

50.00' x 1.00' deep Parabolic Channel, n= 0.060 Length= 970.0' Slope= 0.4227 '/'

#### Reach r22b:

**Overland Flow Reach** 

Inflow Area = 15.520 ac, Inflow Depth = 2.39" for 50-yr event Inflow 13.54 cfs @ 13.00 hrs, Volume= = 3.094 af Outflow = 13.46 cfs @ 13.06 hrs, Volume= 3.094 af, Atten= 1%, Lag= 3.3 min Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 2.4 fps, Min. Travel Time= 4.3 min Avg. Velocity = 0.7 fps, Avg. Travel Time= 14.9 min Peak Depth= 0.31' @ 13.06 hrs Capacity at bank full= 175.17 cfs Inlet Invert= 560.00', Outlet Invert= 512.00'

50.00' x 1.00' deep Parabolic Channel, n= 0.060 Length= 620.0' Slope= 0.0774 '/'

#### Reach r25a:

Ditch Pipe inverts need to be surveyed

Inflow Area = 60.314 ac, Inflow Depth =  $2.94^{"}$  for 50-yr event Inflow 98.53 cfs @ 12.36 hrs, Volume= 14.753 af = Outflow 98.37 cfs @ 12.38 hrs. Volume= 14.753 af, Atten= 0%, Lag= 1.1 min =

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 12.4 fps, Min. Travel Time= 1.5 min Avg. Velocity = 2.6 fps, Avg. Travel Time= 7.0 min

Peak Depth= 1.41' @ 12.38 hrs Capacity at bank full= 205.50 cfs Inlet Invert= 570.00', Outlet Invert= 504.00' 10.00' x 2.00' deep Parabolic Channel, n= 0.027 Length= 1,090.0' Slope= 0.0606 '/'

#### Reach r25b: Wetland Reach

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Wetland Reach Has wetland vegetation within reach

Inflow Area = Inflow = Outflow =	9.435 ac, Inflow Depth = 2.73 18.58 cfs @ 12.32 hrs, Volume 17.11 cfs @ 12.40 hrs, Volume	e= 2.148 af				
Max. Velocity= 2	Stor-Ind method, Time Span= 0.00 2.0 fps, Min. Travel Time= 6.2 min 0.5 fps, Avg. Travel Time= 26.8 mi					
Peak Depth= 1.07' @ 12.40 hrs Capacity at bank full= 156.51 cfs Inlet Invert= 504.00', Outlet Invert= 499.50'						

20.00' x 3.00' deep Parabolic Channel, n= 0.045 Length= 750.0' Slope= 0.0060 '/'

#### Reach r25c: Amenia Stream/Cascade Brook

(Steam) parallel to Route 22 Base Flow estimated from field observation (see field note 21)

Inflow Area = 129.431 ac, Inflow Depth = 31.44" for 50-yr event Inflow 161.97 cfs @ 12.35 hrs, Volume= 339.135 af, Incl. 40.00 cfs Base Flow = 158.31 cfs @ 12.45 hrs, Volume= Outflow = 338,454 af. Atten= 2%. Lag= 5.8 min Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

Max. Velocity= 3.0 fps, Min. Travel Time= 5.5 min Avg. Velocity = 2.7 fps, Avg. Travel Time= 6.1 min

Peak Depth= 6.69' @ 12.45 hrs Capacity at bank full= 67.14 cfs Inlet Invert= 504.00', Outlet Invert= 500.00' 10.00' x 4.00' deep Parabolic Channel, n= 0.060 Length= 975.0' Slope= 0.0041 '/'

#### Pond 8P:

No field note. Water spills over cart path; no storage.

Inflow Area =	52.997 ac, Inflow Depth = 2.93'	for 50-yr event
Inflow =	86.03 cfs @ 12.35 hrs, Volume	= 12.927 af
Outflow =	86.03 cfs @ 12.35 hrs, Volume	= 12.927 af, Atten= 0%, Lag= 0.0 min
Primary =	86.03 cfs @ 12.35 hrs, Volume	= 12.927 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 575.67' @ 12.35 hrs Flood Elev= 574.70' Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= (not calculated)

#	Routing	Invert	Outlet Devices
1	Primary	574.70'	177.0 deg Sharp-Crested Vee/Trap Weir C= 2.46

Primary OutFlow Max=86.02 cfs @ 12.35 hrs HW=575.67' TW=571.41' (Dynamic Tailwater) **1=Sharp-Crested Vee/Trap Weir** (Weir Controls 86.02 cfs @ 2.4 fps)

#### Pond p04:

Field Note #13

Water ponding behind a golf cart path. Overflow dimensions are assumed based on aerial topo, and should be upgraded once survey is available.

Inflow Area =	38.062 ac, Inflow Depth = 3.12"	for 50-yr event
Inflow =	61.91 cfs @ 12.43 hrs, Volume=	9.897 af
Outflow =	61.87 cfs @ 12.44 hrs, Volume=	9.831 af, Atten= 0%, Lag= 0.7 min
Primary =	61.87 cfs @ 12.44 hrs, Volume=	9.831 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 606.05' @ 12.44 hrs Surf.Area= 6,885 sf Storage= 4,508 cf Flood Elev= 605.50' Surf.Area= 4,803 sf Storage= 2,882 cf Plug-Flow detention time= 6.9 min calculated for 9.831 af (99% of inflow) Center-of-Mass det. time= 2.7 min (863.4 - 860.7)

#	Invert	Avail.St	orage S	Storage Des	scription		
1	604.20'	26,8	397 cf	Custom Sta	ige Data (Conic) L	isted below	
Elevati (fe	on et)	Surf.Area (sq-ft)	-	Inc.Store ubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
604. 606. 608.	00	0 6,650 17,060		0 3,990 22,907	0 3,990 26,897	0 6,655 17,092	
_# R	louting		Outlet D				
1 P	rimary	605.50'	179.0 de	g Sharp-Cr	ested Vee/Trap V	<b>Veir</b> C= 2.46	

Primary OutFlow Max=61.86 cfs @ 12.44 hrs HW=606.05' TW=575.66' (Dynamic Tailwater) ☐ 1=Sharp-Crested Vee/Trap Weir (Weir Controls 61.86 cfs @ 1.8 fps) HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems

#### Pond p06:

Field Note #22 Geometry to be confirmed by survey.

Inflow Area =	9.435 ac, Inflow Depth = 2.77"	for 50-yr event
Inflow =	19.92 cfs @ 12.25 hrs, Volume=	2.177 af
Outflow =	18.58 cfs @ 12.32 hrs, Volume=	2.148 af, Atten= 7%, Lag= 4.3 min
Primary =	18.58 cfs @ 12.32 hrs, Volume=	2.148 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 506.80' Surf.Area= 18,600 sf Storage= 42,160 cf Peak Elev= 507.53' @ 12.32 hrs Surf.Area= 21,907 sf Storage= 57,694 cf (15,534 cf above start) Flood Elev= 507.10' Surf.Area= 19,958 sf Storage= 48,537 cf (6,377 cf above start) Plug-Flow detention time= 376.2 min calculated for 1.180 af (54% of inflow) Center-of-Mass det. time= 92.9 min ( 939.1 - 846.1 )

#	Invert	Avail.St	torage	ge Storage Description					
1	500.00'	67,	669 cf	69 cf Custom Stage Data (Conic) Listed below					
Eleva (f	ition eet)	Surf.Area (sq-ft)		Inc.Store ubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)			
	0.00	0	(00	0	0	0			
	6.80 8.00	18,600 24,030		42,160 25,509	42,160 67,669	18,672 24,138			
500	5.00	24,030		25,509	07,009	24,130			
#	Routing	Invert	Outlet D	evices					
1	Primary	506.80'		<b>12.0" x 20.0' long Culvert</b> CMP, projecting, no headwall, Ke= 0.900					
2	Primary	507.10'		Outlet Invert= 506.00' S= 0.0400 '/' n= 0.024 Cc= 0.900 178.0 deg Sharp-Crested Vee/Trap Weir C= 2.46					

Primary OutFlow Max=18.57 cfs @ 12.32 hrs HW=507.53' TW=505.01' (Dynamic Tailwater) -1=Culvert (Inlet Controls 1.41 cfs @ 2.3 fps)

-2=Sharp-Crested Vee/Trap Weir (Weir Controls 17.16 cfs @ 1.6 fps)

## Pond p07:

Field Note # 29 Outlet geometry to be confirmed by survey.

Inflow Are	ea =	7.317 ac, Inflow Depth = 3.05"	for 50-yr event
Inflow	=	18.23 cfs @ 12.19 hrs, Volume=	1.858 af
Outflow	=	12.56 cfs @ 12.37 hrs, Volume=	1.827 af, Atten= 31%, Lag= 10.9 min
Primary	=	12.56 cfs @ 12.37 hrs, Volume=	1.827 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 572.80' Surf.Area= 21,640 sf Storage= 56,264 cf Peak Elev= 573.80' @ 12.37 hrs Surf.Area= 26,344 sf Storage= 80,654 cf (24,390 cf above start) Flood Elev= 573.50' Surf.Area= 24,936 sf Storage= 73,351 cf (17,087 cf above start) Plug-Flow detention time= 702.8 min calculated for 0.535 af (29% of inflow) Center-of-Mass det. time= 152.4 min (985.9 - 833.5)

## Existing Conditions 10454-01

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#	Invert	Avail.St	orage	Storage Description				
1 5	65.00'	85,5	557 cf	f Custom Stage Data (Conic) Listed below				
Elevation (feet)		Surf.Area (sq-ft)	(c	Inc.Store cubic-feet)	Cum.Store (cubic-feet)	Wet.Are (sq-f		
565.00		0		0	0		0	
572.80		21,640		56,264	56,264	21,73	5	
574.00		27,290		29,293	85,557	27,42	24	
# Rou	ting	Invert	Outlet I	Devices				
1 Prim	nary	572.80'	18.0"	x 20.0' long (	Culvert CMP,	projecting, no he	adwall, Ke= 0.900	
2 Prim	nary		Outlet Invert= 572.00' S= 0.0400 '/' n= 0.024 Cc= 0.900 177.0 deg Sharp-Crested Vee/Trap Weir X 2.00 C= 2.46					

Primary OutFlow Max=12.55 cfs @ 12.37 hrs HW=573.80' TW=571.41' (Dynamic Tailwater) -1=Culvert (Inlet Controls 3.36 cfs @ 2.7 fps)

-2=Sharp-Crested Vee/Trap Weir (Weir Controls 9.19 cfs @ 1.3 fps)

## Pond p09:

Field Note #31 Geometry to be confirmed by survey.

Inflow Area	a =	8.452 ac, Inflow Depth = 2.87"	for 50-yr event
Inflow	=	20.48 cfs @ 12.23 hrs, Volume=	2.023 af
Outflow	=	16.39 cfs @ 12.37 hrs, Volume=	1.984 af, Atten= 20%, Lag= 8.3 min
Primary	=	16.39 cfs @ 12.37 hrs, Volume=	= 1.984 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 550.53' @ 12.37 hrs Surf.Area= 6,855 sf Storage= 10,413 cf Flood Elev= 551.20' Surf.Area= 8,534 sf Storage= 15,673 cf Plug-Flow detention time= 30.2 min calculated for 1.984 af (98% of inflow) Center-of-Mass det. time= 19.3 min (873.2 - 853.9)

#	Invert	Avail.St	torage Storage D	Storage Description				
1	547.50'	21,	989 cf Custom \$	Custom Stage Data (Conic) Listed below				
Eleva (f	tion eet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft <u>)</u>			
547	7.50	0	0	0	0			
548	8.00	1,080	180	180	1,080			
550	0.00	5,510	6,020	6,200	5,527			
552	2.00	10,550	15,790	21,989	10,606			
#	Routing	Invert	Outlet Devices					
1	Primary	548.50'	<b>30.0" x 70.0' long Culvert</b> CMP, projecting, no headwall, Ke= 0.900 Outlet Invert= 542.00' S= 0.0929 '/' n= 0.012 Cc= 0.900					
2	Primary	551.20'		deg Sharp-Crested Vee/Trap Weir C= 2.46				

Primary OutFlow Max=16.39 cfs @ 12.37 hrs HW=550.53' TW=542.72' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 16.39 cfs @ 3.8 fps) -2=Sharp-Crested Vee/Trap Weir (Controls 0.00 cfs)

## Pond p10:

Field Note #25 Need to get full story on how this pond works

Inflow Are	a =	45.146 ac, I	nflow Depth = 1.24	for 50-yr event	
Inflow	=	18.17 cfs @	12.37 hrs, Volume	= 4.664 af	
Outflow	=	0.00 cfs @	0.00 hrs, Volume	= 0.000 af, Atten= 100%, Lag= 0.0 min	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 498.40' Surf.Area= 36,110 sf Storage= 101.108 cf Peak Elev= 502.65' @ 48.00 hrs Surf.Area= 62,668 sf Storage= 304,259 cf (203,151 cf above start) Plug-Flow detention time= (not calculated) Center-of-Mass det. time= (not calculated)

#	Invert	Avail.Storag	ge Storage Des	cription		
1	490.00'	581,029	cf Custom Sta	<b>ge Data (Conic)</b> Li	sted below	
Elevatio (fee		Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft <u>)</u>	
490.0	)0	0	0	0	0	
498.4	10	36,110	101,108	101,108	36,221	
500.0	00	42,400	62,741	163,849	42,610	
502.0	00	54,880	97,012	260,861	55,187	
504.0	00	78,730	132,895	393,755	79,107	
506.0	00	109,382	187,274	581,029	109,836	

## Pond p12:

No field note. Natural depression.

Inflow Area	=	6.420  ac,  Inflow Depth = 1.94"	for 50-yr event
Inflow	=	6.45 cfs @ 12.65 hrs, Volume=	1.036 af
Outflow	=	1.30 cfs @ 14.35 hrs, Volume=	0.532 af, Atten= 80%, Lag= 102.5 min
Primary	=	1.30 cfs @ 14.35 hrs, Volume=	0.532 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 547.75' @ 14.35 hrs Surf.Area= 15,298 sf Storage= 24,410 cf Flood Elev= 547.50' Surf.Area= 13,848 sf Storage= 21,762 cf Plug-Flow detention time= 321.0 min calculated for 0.532 af (51% of inflow) Center-of-Mass det. time= 187.3 min (1,089.7 - 902.5)

## Existing Conditions\_10454-01

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HydroCAD® 7.00	HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems 4/10/2006 2:33:54 PM								
# Invert	Avail.Storag	brage Storage Description							
1 543.50	26,986	cf Custom Sta	ge Data (Conic) Li	sted below					
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)					
543.50 544.00 546.00 548.00 # Routing	0 1,140 5,260 16,710	0 190 5,899 20,897 let Devices	0 190 6,089 26,986	0 1,140 5,278 16,750					
1 Primary			ested Vee/Trap W	<b>eir</b> C= 2.46					

Primary OutFlow Max=1.30 cfs @ 14.35 hrs HW=547.75' TW=544.10' (Dynamic Tailwater) ←1=Sharp-Crested Vee/Trap Weir (Weir Controls 1.30 cfs @ 1.2 fps)

#### Pond p13:

No Field Note Natural depression.

Inflow Area	a =	0.350 ac, Ir	nflow Depth = 1.94"	for 50-yr event	
Inflow	=	0.67 cfs @	12.13 hrs, Volume=	0.056 af	
Outflow	=	0.00 cfs @	0.00 hrs, Volume=	0.000 af,	Atten= 100%, Lag= 0.0 min
Primary	=	0.00 cfs @	0.00 hrs, Volume=	0.000 af	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 514.42' @ 24.49 hrs Surf.Area= 1,530 sf Storage= 2,459 cf Flood Elev= 519.50' Surf.Area= 4,313 sf Storage= 16,523 cf Plug-Flow detention time= (not calculated) Center-of-Mass det. time= (not calculated)

#	Invert	Avail.St	orage Storage [	Description		
1	511.40'	18,4	190 cf Custom	Stage Data (Conic)	Listed below	
Elevat (fe	ion eet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
511	.40	0	0	0	0	
512	.00	390	78	78	391	
514	.00	1,360	1,652	1,730	1,381	
516	.00	2,180	3,508	5,238	2,253	
518	.00	3,240	5,385	10,623	3,375	
520	.00	4,670	7,867	18,490	4,872	
# F	Routing	Invert	Outlet Devices			
1 F	Primary	519.50'	176.0 deg Sharp	-Crested Vee/Trap	<b>Weir</b> C= 2.46	

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=511.40' TW=497.40' (Dynamic Tailwater) ↑ 1=Sharp-Crested Vee/Trap Weir (Controls 0.00 cfs) HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems

#### Pond p14:

Field Note #26 Need to figure out how this pond works

Inflow Area =	36.186 ac, Inflow Depth = 2.74"	for 50-yr event
Inflow =	54.63 cfs @ 12.39 hrs, Volume=	8.265 af
Outflow =	3.24 cfs @ 18.00 hrs, Volume=	2.121 af, Atten= 94%, Lag= 337.1 min
Primary =	3.24 cfs @ 18.00 hrs, Volume=	2.121 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 497.40' Surf.Area= 22,200 sf Storage= 54,760 cf Peak Elev= 502.76' @ 18.00 hrs Surf.Area= 77,420 sf Storage= 330,734 cf (275,974 cf above start) Plug-Flow detention time= 735.4 min calculated for 0.863 af (10% of inflow) Center-of-Mass det. time= 314.9 min (1,174.2 - 859.3)

#	Invert	Avail.Sto	rage Storage Des	Storage Description				
1	490.00'	805,06	62 cf Custom Sta	Custom Stage Data (Conic) Listed below				
Eleva		Surf.Area	Inc.Store	Cum.Store	Wet.Area			
(f	eet)	(sq-ft)	(cubic-feet)	(cubic-feet)	(sq-ft)			
490	0.00	0	0	0	0			
497	7.40	22,200	54,760	54,760	22,286			
498	3.00	25,330	14,249	69,009	25,433			
500	0.00	52,810	76,476	145,485	52,948			
502	2.00	73,360	125,608	271,093	73,574			
504	4.00	84,070	157,308	428,402	84,467			
506	5.00	92,130	176,139	604,540	92,797			
508	3.00	108,618	200,522	805,062	109,437			
#	Routing	Invert C	Dutlet Devices					
1	Primary	500.00' <b>2</b>	4.0" x 80.0' long C	Culvert CPP, end-	section conformi	ng to fill, Ke= 0.500		
	-	C	Dutlet Invert= 502.0	0' S= -0.0250 '/'	n= 0.012 Cc= 0	.900		

Primary OutFlow Max=3.24 cfs @ 18.00 hrs HW=502.76' TW=501.34' (Dynamic Tailwater)

## Pond p15:

Field Note # 43 Infiltration basin

Inflow Area	a =	5.770 ac, Inflow Depth = $4.2$	25" for 50-yr event
Inflow	=	19.82 cfs @ 12.07 hrs, Volum	ne= 2.044 af
Outflow	=	19.71 cfs @ 12.07 hrs, Volum	ne= 1.789 af, Atten= 1%, Lag= 0.5 min
Primary	=	19.71 cfs @ 12.07 hrs, Volum	ne= 1.789 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 536.25' @ 12.07 hrs Surf.Area= 3,303 sf Storage= 12,044 cf Flood Elev= 536.00' Surf.Area= 3,160 sf Storage= 11,127 cf Plug-Flow detention time= 106.4 min calculated for 1.789 af (88% of inflow)

#	Invert	Avail.Sto	orage Storage D	Description		
1	526.80'	18,5	77 cf Custom S	Custom Stage Data (Conic) Listed below		
Eleva	ation feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
52	6.80	0	0	0	0	
52	8.00	310	124	124	312	
53	0.00	660	948	1,072	694	
53	2.00	1,180	1,815	2,887	1,256	
53	4.00	1,990	3,135	6,022	2,113	
53	6.00	3,160	5,105	11,127	3,337	
53	8.00	4,320	7,450	18,577	4,575	
<u>#</u> 1	Routing Primary		Outlet Devices 171.0 deg x 50.0	long Sharp-Crest	ed Vee/Trap Wei	· C= 2.46
•	, initially	000.00				0-2.10

Center-of-Mass det. time= 46.4 min (834.3 - 787.9)

Primary OutFlow Max=19.66 cfs @ 12.07 hrs HW=536.25' TW=508.14' (Dynamic Tailwater) ←1=Sharp-Crested Vee/Trap Weir (Weir Controls 19.66 cfs @ 1.5 fps)

#### Pond p16:

Field Note # 49 Large pond with man-made island. Geometry to be verified by survey. In particular, we are making big guesses about the outlets. Also need to find out about valves...

Inflow Are	ea =	220.861 ac, In	flow Depth = 3.14	" for 50-yr event
Inflow	=	230.82 cfs @	12.34 hrs, Volume	= 57.842 af
Outflow	=	111.99 cfs @	14.25 hrs, Volume	= 45.930 af, Atten= 51%, Lag= 114.8 min
Primary	=	111.99 cfs @	14.25 hrs, Volume	= 45.930 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 507.00' Surf.Area= 199,799 sf Storage= 878,320 cf Peak Elev= 511.47' @ 14.25 hrs Surf.Area= 314,318 sf Storage= 1,905,677 cf (1,027,357 cf above start) Flood Elev= 510.50' Surf.Area= 271,550 sf Storage= 1,623,217 cf (744,897 cf above start) Plug-Flow detention time= 688.1 min calculated for 25.761 af (45% of inflow) Center-of-Mass det. time= 281.0 min (1,192.4 - 911.4)

#	Invert	Avail.Storage	Storage De	escription	
1	500.00'	2,062,087 cf	Custom St	tage Data (Conic) L	isted below
Elevatio		Surf.Area	Inc.Store	Cum.Store	Wet.Area
(fee	et)	(sq-ft)	(cubic-feet)	(cubic-feet)	(sq-ft)
500.0	00	0	0	0	0
503.0	00	140,344	140,344	140,344	140,358
509.2	20	232,500	1,143,862	1,284,206	232,994
510.0	00	249,400	192,720	1,476,927	249,951
512.0	00	338,000	585,160	2,062,087	338,634

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#	Routing	Invert	Outlet Devices
1	Primary	509.00'	18.0" x 110.0' long Culvert CMP, projecting, no headwall, Ke= 0.900
	-		Outlet Invert= 505.70' S= 0.0300 '/' n= 0.024 Cc= 0.900
2	Primary	500.00'	8.0" x 100.0' long assumed equalization pipe w/ valve X 0.00
			CMP, projecting, no headwall, Ke= 0.900
			Outlet Invert= 500.00' S= 0.0000 '/' n= 0.013 Cc= 0.900
3	Primary	510.50'	175.0 deg Sharp-Crested Vee/Trap Weir X 2.00 C= 2.46

Primary OutFlow Max=111.99 cfs @ 14.25 hrs HW=511.47' TW=506.51' (Dynamic Tailwater) -1=Culvert (Inlet Controls 8.80 cfs @ 5.0 fps) -2=assumed equalization pipe w/ valve (Controls 0.00 cfs)

-3=Sharp-Crested Vee/Trap Weir (Weir Controls 103.19 cfs @ 2.4 fps)

#### Pond p17:

Field Note #45 Golf pond Geometry to be confirmed by surveyed

Inflow Area	=	115.991 ac, Inflow Depth = 2.98" for 50-yr event	
Inflow :	=	96.22 cfs @ 13.77 hrs, Volume= 28.765 af	
Outflow :	=	96.17 cfs @ 13.77 hrs, Volume= 28.764 af, Atten= 0%, Lag= 0.4 min	1
Primary :	=	96.17 cfs @ 13.77 hrs, Volume= 28.764 af	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 523.80' Surf.Area= 7,290 sf Storage= 9,234 cf Peak Elev= 525.61' @ 13.77 hrs Surf.Area= 11,456 sf Storage= 26,425 cf (17,191 cf above start) Flood Elev= 524.30' Surf.Area= 8,074 sf Storage= 13,623 cf (4,389 cf above start) Plug-Flow detention time= 15.2 min calculated for 28.552 af (99% of inflow) Center-of-Mass det. time= 8.3 min (960.4 - 952.0)

#	Invert	Avail.St	torage	Storage Description				
1	520.00'	30,	224 cf	Custom Stage Data (Conic) Listed below				
Eleva (1	ation feet)	Surf.Area (sq-ft)	(c	Inc.Store cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)		
-	0.00	0		0	0	0		
52	3.80	7,290		9,234	9,234	7,313		
52	4.00	7,300		1,459	10,693	7,374		
52	6.00	12,460		19,531	30,224	12,581		
#	Routing	Invert	Outlet I	Devices				
1	Primary	523.80'	<b>2.2' long x 0.5' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 Coef. (English) 2.80 2.92 3.08 3.30 3.32					
2 3	Primary Primary	524.30' 525.20'	<b>143.0 deg Sharp-Crested Vee/Trap Weir</b> C= 2.47 <b>178.0 deg x 60.0' long Sharp-Crested Vee/Trap Weir</b> C= 2.46					

*Type III 24-hr 50-yr Rainfall=6.70"* Page 158 ns 4/10/2006 2:33:55 PM Primary OutFlow Max=96.17 cfs @ 13.77 hrs HW=525.61' TW=516.18' (Dynamic Tailwater) 1=Broad-Crested Rectangular Weir (Weir Controls 17.80 cfs @ 4.5 fps) 2=Sharp-Crested Vee/Trap Weir (Weir Controls 14.52 cfs @ 2.8 fps) -3=Sharp-Crested Vee/Trap Weir (Weir Controls 63.84 cfs @ 1.9 fps)

#### Pond p18:

Field Note #46 Golf pond Geometry to be confirmed by surveyed

Inflow Area	a =	139.117 ac, Inflow Dep	oth = 3.00"	for 50-yr event	
Inflow	=	103.28 cfs @ 13.77 hr	s, Volume=	34.823 af	
Outflow	=	103.13 cfs @ 13.79 hr	s, Volume=	34.818 af,	Atten= 0%, Lag= 1.2 min
Primary	=	103.13 cfs @ 13.79 hr	s, Volume=	34.818 af	-

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 513.90' Surf.Area= 20,680 sf Storage= 26,884 cf Peak Elev= 516.18' @ 13.79 hrs Surf.Area= 29,624 sf Storage= 84,240 cf (57,356 cf above start) Flood Elev= 514.81' Surf.Area= 23,768 sf Storage= 48,709 cf (21,825 cf above start) Plug-Flow detention time= 39.8 min calculated for 34.201 af (98% of inflow) Center-of-Mass det. time= 23.6 min (964.9 - 941.3)

#	Invert	Avail.S	torage Storage D	Description			
1	510.00'	148,	288 cf Custom \$	Stage Data (Conic)	Listed below		
51 51 51	ation feet) 0.00 3.90 4.00 6.00	Surf.Area (sq-ft) 0 20,680 20,690 28,290	(cubic-feet) 0 26,884 2,068	Cum.Store (cubic-feet) 0 26,884 28,952 77,735	Wet.Area (sq-ft) 0 20,704 20,756 28,436		
51	8.00	42,760	70,554	148,288	42,967		
#	Routing	Invert	Outlet Devices				
1	Primary	513.90'	<b>2.0' long x 0.5' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 Coef. (English) 2.80 2.92 3.08 3.30 3.32				
2 3	Primary Primary	514.81' 515.32'	<b>143.0 deg Sharp-Crested Vee/Trap Weir</b> C= 2.47 <b>175.0 deg x 10.0' long Sharp-Crested Vee/Trap Weir</b> C= 2.46				

**Primary OutFlow** Max=103.13 cfs @ 13.79 hrs HW=516.18' TW=511.37' (Dynamic Tailwater) **1=Broad-Crested Rectangular Weir** (Weir Controls 22.93 cfs @ 5.0 fps)

-2=Sharp-Crested Rectangular Weir (Weir Controls 22.93 cfs @ 5.0 lps)

-3=Sharp-Crested Vee/Trap Weir (Weir Controls 63.85 cfs @ 2.5 fps)

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#### Pond p19:

Wetland Geometry to be confirmed by survey Based off aerial topo, and assumed topo contour

Inflow Area =	15.520 ac, Inflow Depth = 2.39"	for 50-yr event
Inflow =	20.51 cfs @ 12.61 hrs, Volume=	3.096 af
Outflow =	13.60 cfs @ 12.96 hrs, Volume=	3.094 af, Atten= 34%, Lag= 20.9 min
Primary =	13.60 cfs @ 12.96 hrs, Volume=	3.094 af
Secondary =	0.00 cfs @ 0.00 hrs, Volume=	0.000 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 972.00' Surf.Area= 86,000 sf Storage= 57,333 cf Peak Elev= 972.27' @ 12.96 hrs Surf.Area= 93,340 sf Storage= 87,215 cf (29,881 cf above start) Plug-Flow detention time= 300.9 min calculated for 1.778 af (57% of inflow) Center-of-Mass det. time= 59.3 min ( 947.2 - 888.0 )

#	Invert	Avail.Sto	orage Storage Des	scription		
1 9	70.00'	282,3	29 cf Custom Sta	ge Data (Conic) L	isted below	
Elevation (feet)		Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
970.00		0	0	0	0	
972.00		86,000	57,333	57,333	86,006	
974.00		141,270	224,996	282,329	141,327	
_# Rou	ıting	Invert	Outlet Devices			

 1
 Secondary
 973.60'
 178.0 deg x 51.0' long Sharp-Crested Vee/Trap Weir
 C= 2.46

 2
 Primary
 972.00'
 35.0' long x 0.5' breadth Broad-Crested Rectangular Weir
 Head (feet)
 0.20
 0.40
 0.60
 0.80
 1.00

 Coef. (English)
 2.80
 2.92
 3.08
 3.30
 3.32

Primary OutFlow Max=13.60 cfs @ 12.96 hrs HW=972.27' TW=970.21' (Dynamic Tailwater) ←2=Broad-Crested Rectangular Weir (Weir Controls 13.60 cfs @ 1.5 fps)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=972.00' TW=973.60' (Dynamic Tailwater) -1=Sharp-Crested Vee/Trap Weir (Controls 0.00 cfs)

## Pond p20:

Field Note #50 Spring Fed Pond Geometry to be confirmed by surveyed

 Inflow Area =
 241.484 ac, Inflow Depth =
 2.59" for 50-yr event

 Inflow =
 117.86 cfs @
 14.24 hrs, Volume=
 52.112 af

 Outflow =
 115.73 cfs @
 14.40 hrs, Volume=
 51.255 af, Atten= 2%, Lag= 9.5 min

 Primary =
 115.73 cfs @
 14.40 hrs, Volume=
 51.255 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

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Starting Elev= 505.10' Surf.Area= 89,370 sf Storage= 138,524 cf Peak Elev= 506.52' @ 14.42 hrs Surf.Area= 91,962 sf Storage= 268,165 cf (129,641 cf above start) Plug-Flow detention time= 171.0 min calculated for 48.075 af (92% of inflow) Center-of-Mass det. time= 44.8 min (1,194.9 - 1,150.1)

#	Invert	Avail.S	orage Storage Desc	cription	
1	502.00'	615,	682 cf Custom Stag	je Data (Prisma	atic) Listed below
		- · ·			
	ration	Surf.Area	Inc.Store	Cum.Store	
	(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)	
50	02.00	0	0	0	
50	05.10	89,370	138,524	138,524	
50	06.00	89,380	80,437	218,961	
50	08.00	99,280	188,660	407,621	
5′	10.00	108,781	208,061	615,682	
#	Routing	Invert	Outlet Devices		
1	Primary	505.10'	3.0' long x 1.5' bread	dth Broad-Cres	sted Rectangular Weir
	-		Head (feet) 0.20 0.4	0 0.60 0.80 1	.00 1.20 1.40 1.60 1.80 2.00 2.50
			3.00		
			Coef. (English) 2.62	2.64 2.64 2.6	8 2.75 2.86 2.92 3.07 3.07 3.03 3.28
			3.32		
2	Primary	506.20'		dth Broad-Cres	sted Rectangular Weir
	,				.00 1.20 1.40 1.60 1.80 2.00 2.50
			3.00		
				2.64 2.64 2.6	8 2.75 2.86 2.92 3.07 3.07 3.03 3.28
			3.32		
3	Primary	506.00'		ng Sharp-Crest	ed Vee/Trap Weir C= 2.46
			-		
			.68 cfs @ 14.40 hrs H		

-1=Broad-Crested Rectangular Weir (Weir Controls 7.61 cfs @ 1.8 fps)

-2=Broad-Crested Rectangular Weir (Weir Controls 2.68 cfs @ 1.3 fps)

-3=Sharp-Crested Vee/Trap Weir (Weir Controls 105.39 cfs @ 1.8 fps)

#### Pond p21:

Inflow Area	=	489.305 ac, Inflow Depth = 2.95"	for 50-yr event
Inflow =	=	461.70 cfs @ 12.44 hrs, Volume=	120.203 af
Outflow =	=	37.28 cfs @ 21.11 hrs, Volume=	96.485 af, Atten= 92%, Lag= 520.3 min
Primary =	=	37.28 cfs @ 21.11 hrs, Volume=	96.485 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 486.58' @ 21.11 hrs Surf.Area= 1,340,291 sf Storage= 3,276,119 cf Plug-Flow detention time= 883.2 min calculated for 96.485 af (80% of inflow) Center-of-Mass det. time= 735.6 min (1,737.5 - 1,002.0)

#	Invert	Avail.Storage	Storage Description
1	480.40'	5,244,885 cf	Custom Stage Data (Conic) Listed below

## Existing Conditions\_10454-01

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Elevation	Surf.Area	Inc.Store	Cum.Store	Wet.Area	
(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)	(sq-ft)	
480.40	0	0	0	0	
482.00	202,230	107,856	107,856	202,234	
484.00	485,198	667,114	774,970	485,231	
486.00	1,275,481	1,698,237	2,473,208	1,275,541	
488.00	1,499,208	2,771,678	5,244,885	1,499,423	
# Routing	Invert	Outlet Devices			
1 Drimony	100 10'	20.0" x 70.0' long (	CMP proj	octing no boodwall	L

1 Primary 480.40' **30.0" x 70.0' long Culvert** CMP, projecting, no headwall, Ke= 0.900 Outlet Invert= 480.40' S= 0.0000 '/' n= 0.024 Cc= 0.900

Primary OutFlow Max=37.28 cfs @ 21.11 hrs HW=486.58' TW=0.00' (Dynamic Tailwater) ←1=Culvert (Barrel Controls 37.28 cfs @ 7.6 fps)

#### Pond p22:

Field Note #54 Golf Pond Geometry to be confirmed by survey

Inflow Are	a =	97.943 ac, Inflow Depth = 3.14" for 50-yr event
Inflow	=	176.41 cfs @ 12.45 hrs, Volume= 25.590 af
Outflow	=	175.37 cfs @ 12.49 hrs, Volume= 25.291 af, Atten= 1%, Lag= 2.2 min
Primary	=	175.37 cfs @ 12.49 hrs, Volume= 25.291 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 498.10' Surf.Area= 6,520 sf Storage= 10,106 cf Peak Elev= 502.60' @ 12.49 hrs Surf.Area= 12,435 sf Storage= 52,053 cf (41,947 cf above start) Plug-Flow detention time= 27.1 min calculated for 25.053 af (98% of inflow) Center-of-Mass det. time= 12.5 min (882.9 - 870.4)

#	Invert	Avail.St	orage Storag	ge Des	scription	
1	495.00'	143,	770 cf Custo	om Sta	ige Data (Prisma	atic) Listed below
Eleva		Surf.Area	Inc.St		Cum.Store	
(f	feet)	(sq-ft)	(cubic-fe	et)	(cubic-feet)	
49	5.00	0		0	0	
498	8.10	6,520	10,1	106	10,106	
500	0.00	8,390	14,1	164	24,270	
502	2.00	11,530	19,9	920	44,190	
504	4.00	14,530	26,0	060	70,250	
50	6.00	18,340	32,8	370	103,120	
508	8.00	22,310	40,6	650	143,770	
#	Routing	Invert	<b>Outlet Device</b>	s		
1	Primary	499.75'	18.0" x 21.0'	long (	Culvert CMP, p	rojecting, no headwall, Ke= 0.900
	-		Outlet Invert=	499.7	5' S= 0.0000 '/'	n= 0.024 Cc= 0.900
2	Primary	500.50'				ested Rectangular Weir
			Head (feet) C	.20 0.	.40 0.60 0.80 1	.00 1.20 1.40 1.60
			Coef. (English	n) 2.68	3 2.70 2.70 2.6	4 2.63 2.64 2.64 2.63

Existing Conditions_10454-01	Type III 24-hr 50-yr Rainfall=6.70"
Prepared by The Chazen Companies	Page 163
HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer System	tems 4/10/2006 2:33:55 PM

3 Primary 500.50' **20.0' long x 13.5' breadth Broad-Crested Rectangular Weir** Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.62 2.66 2.70 2.66 2.65 2.66 2.65 2.63

Primary OutFlow Max=175.33 cfs @ 12.49 hrs HW=502.60' TW=500.73' (Dynamic Tailwater) -1=Culvert (Inlet Controls 9.20 cfs @ 5.2 fps) -2=Broad-Crested Rectangular Weir (Weir Controls 7.91 cfs @ 3.8 fps)

-3=Broad-Crested Rectangular Weir (Weir Controls 158.22 cfs @ 3.8 fps)

#### Pond p23:

Inflow Area	a =	41.587 ac, I	nflow Depth =	3.57" for	r 50-yr event		
Inflow	=	78.66 cfs @	12.64 hrs, Vol	ume=	12.389 af		
Outflow	=	78.61 cfs @	12.66 hrs, Vol	ume=	11.471 af,	Atten= 0%,	Lag= 0.8 min
Primary	=	78.61 cfs @	12.66 hrs, Vol	ume=	11.471 af		-

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 507.96' @ 12.66 hrs Surf.Area= 19,305 sf Storage= 44,019 cf Plug-Flow detention time= 55.2 min calculated for 11.471 af (93% of inflow) Center-of-Mass det. time= 16.8 min (883.3 - 866.4)

#	Invert	Avail.Ste	orage	Storage De	escription		
1	503.50'	100,303 cf		Custom Stage Data (Prismatic) Listed below			
Elevatio (fee		Surf.Area (sq-ft)	(c	Inc.Store	Cum.Store (cubic-feet)		
503.5 506.0 508.0 510.0	00 00	0 11,170 19,460 36,250		0 13,963 30,630 55,710	0 13,963 44,593 100,303		
-	outing rimary			Devices leg x 178.0	long Sharp-Cre	sted Vee/Trap Weir	C= 2.46

Primary OutFlow Max=78.61 cfs @ 12.66 hrs HW=507.96' TW=507.31' (Dynamic Tailwater) ←1=Sharp-Crested Vee/Trap Weir (Weir Controls 78.61 cfs @ 1.6 fps)

## Pond zDP1: Design Point 1

Field note #10. Culvert dimensions to be confirmed by survey.

Inflow Area =		26.659 ac, I	nflow Depth = 3.23"	for 50-yr event	
Inflow	=	50.31 cfs @	12.48 hrs, Volume=	7.167 af	
Outflow	=	50.31 cfs @	12.48 hrs, Volume=	7.167 af,	Atten= 0%, Lag= 0.1 min
Primary	=	50.31 cfs @	12.48 hrs, Volume=	7.167 af	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 723.04' @ 12.48 hrs Surf.Area= 220 sf Storage= 266 cf Flood Elev= 727.00' Surf.Area= 1,105 sf Storage= 2,619 cf Plug-Flow detention time= 0.2 min calculated for 7.167 af (100% of inflow) Invert

#

5					-
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Avail.Storage Storage Description

<u><u></u> </u>		Collage Otolage De	55611211011		
1 720.1	0' 3,	706 cf Custom St	age Data (Conic)	Listed below	
Elevation (feet)	Surf.Area (sq-ft)		Cum.Store (cubic-feet)	Wet.Area (sq-ft <u>)</u>	
720.10	0	0	0	0	
722.00	90	57	57	96	
724.00	340	403	460	364	
726.00	760	1,072	1,533	815	
728.00	1,450	2,173	3,706	1,543	
# Routing	Invert	Outlet Devices			
1 Primary	720.10'	<b>42.0"</b> x <b>120.0'</b> long Outlet Invert= 700.			
2 Primary	727.00'	155.0 deg Sharp-C			

Center-of-Mass det. time= 0.1 min (864.4 - 864.3)

Primary OutFlow Max=50.31 cfs @ 12.48 hrs HW=723.04' TW=686.90' (Dynamic Tailwater) -1=Culvert (Inlet Controls 50.31 cfs @ 5.8 fps) -2=Sharp-Crested Vee/Trap Weir (Controls 0.00 cfs)

Pond zDP2: Design Point 2

Field Note #15 Culvert dimensions to be confirmed by survey Overflow to ditch is currently discarded... We may have to model that area...

Inflow Area = Inflow = Outflow = Discarded = Primary =	125.74 cfs @ 12.87 hrs, Volume= 125.68 cfs @ 12.87 hrs, Volume= 89.50 cfs @ 12.87 hrs, Volume=	23.389 af 23.389 af, Atten= 0%, Lag= 0.3 min 7.267 af
Primary =	36.18 cfs @ 12.87 hrs, Volume=	16.122 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 626.32' @ 12.87 hrs Surf.Area= 1,605 sf Storage= 3,968 cf Flood Elev= 624.50' Surf.Area= 925 sf Storage= 1,728 cf Plug-Flow detention time= 0.7 min calculated for 23.389 af (100% of inflow) Center-of-Mass det. time= 0.5 min (896.1 - 895.5)

#	Invert	Avail.Storag	e Storage Des	cription	
1	619.60'	7,280 0	cf Custom Sta	<b>ge Data (Conic)</b> Li	sted below
Elevati		Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
619.	60	0	0	0	0
620.	00	10	1	1	10
622.	00	260	214	215	269
624.	00	760	976	1,192	793
626. 628.		1,420 2,580	2,146 3,943	3,338 7,280	1,492 2,694

ī	#	Routing	Invert	Outlet Devices
	1	Primary	619.60'	24.0" x 150.0' long Culvert RCP, end-section conforming to fill, Ke= 0.500
				Outlet Invert= 608.00' S= 0.0773 '/' n= 0.012 Cc= 0.900
	2	Discarded	624.50'	166.0 deg Sharp-Crested Vee/Trap Weir C= 2.46

Discarded OutFlow Max=89.48 cfs @ 12.87 hrs HW=626.32' (Free Discharge) ←2=Sharp-Crested Vee/Trap Weir (Weir Controls 89.48 cfs @ 3.3 fps)

Primary OutFlow Max=36.18 cfs @ 12.87 hrs HW=626.32' TW=607.68' (Dynamic Tailwater) ☐ 1=Culvert (Inlet Controls 36.18 cfs @ 11.5 fps)

# Pond zDP3: Design Point 3

Inflow Area =		212.742 ac, I	nflow Depth = 20.23'	for	50-yr event	
Inflow	=	300.14 cfs @	12.39 hrs, Volume=		358.711 af	
Primary	=	300.14 cfs @	12.39 hrs, Volume=	•	358.711 af,	Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

## Pond zDP4: Design Point 4

Inflow Area =		489.305 ac, Inflow Depth = 2.37	for 50-yr event
Inflow	=	37.28 cfs @ 21.11 hrs, Volume	= 96.485 af
Primary	=	37.28 cfs @ 21.11 hrs, Volume	= 96.485 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

# Pond zDP5: Design Point 5

Inflow Area =		28.325 ac, Inflow Depth = 3.3	7" for 50-yr event
Inflow	=	62.42 cfs @ 12.45 hrs, Volum	e= 7.956 af
Primary	=	62.42 cfs @ 12.45 hrs, Volume	e= 7.956 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

# Pre-Development Conditions 100 year 24 hour Storm Event Model Computations

Existing Conditions_10454-01Type III 24-hr 100-yr Rainfall=7.00"Prepared by The Chazen CompaniesPage 166HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems4/10/2006 2:34:04 PM			
Subcatchment s01:			
Runoff = 21.78 cfs @ 12.60 hrs, Volume= 3.264 af, Depth= 3.41"			
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=7.00"			
Area (ac) CN Description 11.485 68			
Tc Length Slope Velocity Capacity Description			
(min) (feet) (ft/ft) (ft/sec) (cfs)			
Subcatchment s02:			
Runoff = 136.27 cfs @ 12.87 hrs, Volume= 25.263 af, Depth= 3.10"			
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=7.00"			
Area (ac) CN Description			
97.712 65			
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)			
61.3 Direct Entry,			
Subcatchment s03:			
Runoff = 35.87 cfs @ 12.41 hrs, Volume= 4.443 af, Depth= 3.51"			
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=7.00"			
Area (ac) CN Description			
15.174 69			
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)			
28.8 Direct Entry,			
Subcatchment s04:			
Runoff = 40.38 cfs @ 12.10 hrs, Volume= 2.948 af, Depth= 3.10"			
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=7.00"			

Existing Conditions_10454-01	Type III 24-hr 100-yr Rainfall=7.00"
Prepared by The Chazen Companies HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomput	Page 167 er Systems 4/10/2006 2:34:04 PM
	<u>, 10,2000 210 10 1 1 11</u>
Area (ac) CN Description 11.403 65	
11.403 65	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
6.5 Direct Entry	/,
Subcatchment s	05:
Runoff = 32.84 cfs @ 12.25 hrs, Volume=	3.363 af, Depth= 2.70"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48 Type III 24-hr 100-yr Rainfall=7.00"	3.00 hrs, dt= 0.01 hrs
Area (ac) CN Description	
14.935 61	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
17.3 Direct Entry	/,
Subcatchment s	ne.
Subcatchment S	
Runoff = 20.64 cfs @ 12.25 hrs, Volume=	2.102 af, Depth= 2.80"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48 Type III 24-hr 100-yr Rainfall=7.00"	3.00 hrs, dt= 0.01 hrs
Area (ac) CN Description	
9.007 62	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
17.3 Direct Entry	·,
Subastahmant a06/014/10	6 Open Weter
Subcatchment s06(OW): s0	6 Open water
Runoff = 3.60 cfs @ 12.00 hrs, Volume=	0.250 af, Depth= 7.00"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48 Type III 24-hr 100-yr Rainfall=7.00"	3.00 hrs, dt= 0.01 hrs
Area (ac) CN Description	
0.428 100	

Existing Conditions_10454-01Type III 24-hr 100-yrRainfall=7.00"Prepared by The Chazen CompaniesPage 168HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems4/10/2006 2:34:04 PM			
Subcatchment s07:			
Runoff = 18.36 cfs @ 12.19 hrs, Volume= 1.704 af, Depth= 3.00"			
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=7.00"			
Area (ac) CN Description			
6.811 64			
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)			
13.9 Direct Entry,			
Subcatchment s07(OW): s07 Open Water			
Runoff = 4.25 cfs @ 12.00 hrs, Volume= 0.295 af, Depth= 7.00"			
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=7.00"			
Area (ac) CN Description 0.506 100			
Subcatchment s08:			
Runoff = 55.54 cfs @ 12.33 hrs, Volume= 6.368 af, Depth= 2.41"			
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=7.00"			
Area (ac) CN Description			
31.719 58			
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)			
21.8 Direct Entry,			
Subcatchment s09:			
Runoff = 22.20 cfs @ 12.23 hrs, Volume= 2.185 af, Depth= 3.10"			
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=7.00"			
Area (ac) CN Description 8.452 65			

Existing Conditions_10454-01 Type III 24-hr 100-yr Rainfall=7.00"
Prepared by The Chazen CompaniesPage 169HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems4/10/2006 2:34:04 PM
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
16.4         Direct Entry,
Subcatchment s10:
Runoff = 18.28 cfs @ 12.40 hrs, Volume= 2.240 af, Depth= 3.31"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=7.00"
Area (ac) CN Description
8.130 67
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
27.9Direct Entry,
Subcatchment s10(OW): s10 Open Water
Runoff = 6.97 cfs @ 12.00 hrs, Volume= 0.484 af, Depth= 7.00"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=7.00"
Area (ac) CN Description
0.830 100
Subcatchment s11:
Runoff = 6.48 cfs @ 12.28 hrs, Volume= 0.692 af, Depth= 3.51"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=7.00"
Area (ac) CN Description
2.364 69
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
20.5 Direct Entry,
Subcatchment s11(IC): s11 Imp. Cover
Runoff = 18.01 cfs @ 12.04 hrs, Volume= 1.317 af, Depth= 6.76"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=7.00"

Existing Conditions_10454-01	Type III 24-hr 100-yr Rainfall=7.00"
Prepared by The Chazen Companies HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Sys	Page 170 tems 4/10/2006 2:34:04 PM
Area (ac) CN Description 2.338 98	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
2.8 Direct Entry,	
Subcatchment s12:	
Runoff = 7.15 cfs @ 12.64 hrs, Volume= 1.136	af, Depth= 2.12"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 h Type III 24-hr 100-yr Rainfall=7.00"	rs, dt= 0.01 hrs
Area (ac) CN Description	
6.420 55	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
42.0 Direct Entry,	
Subcatchment s13:	
Subcatchment 313.	
Runoff = 0.74 cfs @ 12.13 hrs, Volume= 0.062	af, Depth= 2.12"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 ht Type III 24-hr 100-yr Rainfall=7.00"	rs, dt= 0.01 hrs
Area (ac) CN Description	
0.350 55	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
8.6 Direct Entry,	
Subcatchment s14:	
Runoff = 36.49 cfs @ 12.40 hrs, Volume= 4.519	af, Depth= 3.00"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 h Type III 24-hr 100-yr Rainfall=7.00"	rs, dt= 0.01 hrs
Area (ac) CN Description	
18.066 64	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
28.1 Direct Entry,	

Existing Conditions_10454-01TypPrepared by The Chazen CompaniesHydroCAD® 7.00s/n 000927© 1986-2003 Applied Microcomputer Systems	e III 24-hr 100-yr Rainfall=7.00" Page 171 4/10/2006 2:34:05 PM
Subcatchment s14(IC): s14 Imp. Co	ver
Runoff = 18.66 cfs @ 12.03 hrs, Volume= 1.341 af, D	Depth= 6.76"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= Type III 24-hr 100-yr Rainfall=7.00"	= 0.01 hrs
Area (ac) CN Description 2.380 98	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
2.3 Direct Entry,	
Subcatchment s14(OW): s14 Open W	Vater
Runoff = 4.35 cfs @ 12.00 hrs, Volume= 0.302 af, D	Depth= 7.00"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= Type III 24-hr 100-yr Rainfall=7.00"	= 0.01 hrs
Area (ac) CN Description 0.518 100	
Subcatchment s15:	
Runoff = 1.53 cfs @ 12.20 hrs, Volume= 0.156 af, D	0epth= 1.76"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= Type III 24-hr 100-yr Rainfall=7.00"	= 0.01 hrs
Area (ac) CN Description 1.068 51	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
13.1Direct Entry,	
Subcatchment s16:	
Runoff = 163.08 cfs @ 12.32 hrs, Volume= 18.157 af, D	epth= 3.20"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= Type III 24-hr 100-yr Rainfall=7.00"	= 0.01 hrs
Area (ac) CN Description	
67.994 66	

Existing Conditions_10454-01Type III 24-hr 100-yrRainfall=7.00"Prepared by The Chazen CompaniesPage 172HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems4/10/2006 2:34:05 PM
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
22.3 Direct Entry,
Subcatchment s16(IC): s16 Imp.Cover
Runoff = 20.17 cfs @ 12.04 hrs, Volume= 1.481 af, Depth= 6.76"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=7.00"
Area (ac) CN Description
2.629 98
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
2.9 Direct Entry,
Subcatchment s16(OW): s16 Open Water
Runoff = 44.97 cfs @ 12.00 hrs, Volume= 3.121 af, Depth= 7.00"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=7.00"
Area (ac) CN Description
5.351 100
Subcatchment s17:
Runoff = 104.07 cfs @ 13.77 hrs, Volume= 30.930 af, Depth= 3.20"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=7.00"
Area (ac) CN Description
115.827 66
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
125.2 Direct Entry,
Subcatchment s17(OW): s17 Open Water
Runoff = 1.38 cfs @ 12.00 hrs, Volume= 0.096 af, Depth= 7.00"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=7.00"

Existing Conditions_10454-01Type III 24-hr 100-yr Rainfall=7.00"Prepared by The Chazen CompaniesPage 173HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems4/10/2006 2:34:05 PM
Area (ac) CN Description 0.164 100
Subcatchment s18:
Runoff = 54.10 cfs @ 12.34 hrs, Volume= 6.243 af, Depth= 3.31"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=7.00"
Area (ac) CN Description
22.654 67
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
24.4Direct Entry,
Subcatchment s18(OW): s18 Open Water
Runoff = 3.97 cfs @ 12.00 hrs, Volume= 0.275 af, Depth= 7.00"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=7.00"
Area (ac) CN Description
0.472 100
Subcatchment s19:
Runoff = 22.44 cfs @ 12.61 hrs, Volume= 3.367 af, Depth= 2.60"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=7.00"
Area (ac) CN Description
15.520 60
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
40.4 Direct Entry,
Subcatchment s20:
Runoff = 40.99 cfs @ 12.47 hrs, Volume= 5.463 af, Depth= 3.51"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

Type III 24-hr 100-yr Rainfall=7.00"

Existing Conditions_10454-01 Prepared by The Chazen Companies	Type III 24-hr 100-yr Rainfall=7.00 Page 174
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Area (ac) CN Description	
18.655 69	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
33.9 Direct Entry,	
Subcatchment s20(OW): s20 C	Open Water
Runoff = 16.54 cfs @ 12.00 hrs, Volume= 1.14	18 af, Depth= 7.00"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 Type III 24-hr 100-yr Rainfall=7.00"	) hrs, dt= 0.01 hrs
Area (ac) CN Description 1.968 100	
Subcatchment s21:	
Runoff = 228.02 cfs @ 12.37 hrs, Volume= 27.29	97 af, Depth= 3.41"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 Type III 24-hr 100-yr Rainfall=7.00"	) hrs, dt= 0.01 hrs
Area (ac) CN Description	
96.056 68	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
26.7 Direct Entry,	
Subcatchment s21(OV	V):
Runoff = 102.81 cfs @ 12.00 hrs, Volume= 7.13	37 af, Depth= 7.00"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 Type III 24-hr 100-yr Rainfall=7.00"	) hrs, dt= 0.01 hrs
Area (ac) CN Description 12.235 100	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	

Existing Conditions_10454-01Type III 24-hr 100-yr Rainfall=7.00"Prepared by The Chazen CompaniesPage 175HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems4/10/2006 2:34:05 PM
Subcatchment s22:
Runoff = 187.23 cfs @ 12.45 hrs, Volume= 24.096 af, Depth= 3.51"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=7.00"
Area (ac) CN Description
82.287 69
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
31.3 Direct Entry,
Subcatchment s22(OW): s22 Open Water
Runoff = 1.14 cfs @ 12.00 hrs, Volume= 0.079 af, Depth= 7.00"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=7.00"
Area (ac) CN Description
0.136 100
Subcatchment s23:
Runoff = 84.32 cfs @ 12.64 hrs, Volume= 13.270 af, Depth= 3.83"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=7.00"
Area (ac) CN Description
41.587 72
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
47.0 Direct Entry,
Subcatchment s24:
Runoff = 67.08 cfs @ 12.45 hrs, Volume= 8.541 af, Depth= 3.62"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=7.00"
Area (ac) CN Description
28.325 70

Existing Conditions_10454-01 Type III 24-hr 100-yr Rainfall=7.00
Prepared by The Chazen CompaniesPage 176HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems4/10/2006 2:34:05 PM
TcLengthSlopeVelocityCapacityDescription(min)(feet)(ft/ft)(ft/sec)(cfs)
30.7 Direct Entry,
Subcatchment s25:
Runoff = 33.89 cfs @ 12.28 hrs, Volume= 3.622 af, Depth= 3.20"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=7.00"
Area (ac) CN Description
13.562 66
TcLengthSlopeVelocityCapacityDescription(min)(feet)(ft/ft)(ft/sec)(cfs)
20.2 Direct Entry,
Reach r03:
Overland Flow Reach Requires more survey
Inflow Area = $11.485$ ac, Inflow Depth = $3.41$ " for 100-yr event Inflow = $21.78$ cfs @ $12.60$ hrs, Volume= $3.264$ af
Innow $=$ $21.76$ cfs @ $12.60$ hrs, Volume= $3.264$ afOutflow $=$ $21.68$ cfs @ $12.62$ hrs, Volume= $3.264$ af, Atten= 0%, Lag= 1.4 min
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 6.0 fps, Min. Travel Time= 2.2 min Avg. Velocity = 2.2 fps, Avg. Travel Time= 5.8 min
Peak Depth= 0.76' @ 12.62 hrs Capacity at bank full= 92.14 cfs Inlet Invert= 845.00', Outlet Invert= 728.00' 10.00' x 1.50' deep Parabolic Channel, n= 0.060 Length= 785.0' Slope= 0.1490 '/'
Reach r04:
Channel
Inflow Area =       26.659 ac, Inflow Depth =       3.47" for 100-yr event         Inflow =       54.22 cfs @       12.48 hrs, Volume=       7.707 af         Outflow =       54.14 cfs @       12.49 hrs, Volume=       7.707 af, Atten= 0%, Lag= 1.0 min
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 7.7 fps, Min. Travel Time= 1.5 min

Avg. Velocity = 2.9 fps, Avg. Travel Time= 3.9 min

Existing Conditions 10454-01 Type III 24-hr 100-yr Rainfall=7.00" Prepared by The Chazen Companies HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems 4/10/2006 2:34:05 PM

Peak Depth= 1.45' @ 12.49 hrs Capacity at bank full= 446.15 cfs Inlet Invert= 685.50', Outlet Invert= 608.00' 12.00' x 4.00' deep Parabolic Channel, n= 0.060 Length= 675.0' Slope= 0.1148 '/'

#### Reach r08a:

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Man Made Ditch Inverts of pipe to be surveyed

Inflow Are	a =	97.712 ac, Inflow	Depth = $2.08"$	for 100-yr event	
Inflow	=	36.43 cfs @ 12.8	7 hrs, Volume=	16.917 af	
Outflow	=	36.43 cfs @ 12.8	8 hrs, Volume=	16.917 af, Atter	n= 0%, Lag= 0.3 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 9.6 fps, Min. Travel Time= 0.4 min Avg. Velocity = 5.7 fps, Avg. Travel Time= 0.7 min

Peak Depth= 0.69' @ 12.88 hrs Capacity at bank full= 81.88 cfs Inlet Invert= 607.00', Outlet Invert= 587.00' 10.00' x 1.00' deep Parabolic Channel, n= 0.027 Length= 226.0' Slope= 0.0885 '/'

#### Reach r08b:

24" HDPE Inverts to be surveyed

Inflow Area = 97.712 ac, Inflow Depth = 2.08" for 100-yr event Inflow = 36.43 cfs @ 12.88 hrs, Volume= 16.917 af Outflow 36.43 cfs @ 12.88 hrs, Volume= 16.917 af, Atten= 0%, Lag= 0.1 min =

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 24.2 fps, Min. Travel Time= 0.2 min Avg. Velocity = 14.9 fps, Avg. Travel Time= 0.3 min

Peak Depth= 0.97' @ 12.88 hrs Capacity at bank full= 77.17 cfs Inlet Invert= 587.00', Outlet Invert= 557.75' 24.0" Diameter Pipe n= 0.012 Length= 295.0' Slope= 0.0992 '/'

#### Reach r08c:

Ditch Pipe inverts to be surveyed

Inflow Area = 97.712 ac, Inflow Depth = 2.08" for 100-vr event Inflow 36.43 cfs @ 12.88 hrs, Volume= 16.917 af = 36.43 cfs @ 12.89 hrs, Volume= Outflow 16.917 af, Atten= 0%, Lag= 0.6 min =

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Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 9.2 fps, Min. Travel Time= 1.1 min Avg. Velocity = 5.2 fps, Avg. Travel Time= 1.9 min

Peak Depth= 0.71' @ 12.89 hrs Capacity at bank full= 76.65 cfs Inlet Invert= 557.75', Outlet Invert= 512.00' 10.00' x 1.00' deep Parabolic Channel, n= 0.027 Length= 590.0' Slope= 0.0775 '/'

#### Reach r08d: Amenia Stream/Cascade Brook

(Steam) parallel to Route 22 Base Flow estimated from field observation (see field note 21)

97.712 ac, Inflow Depth = 21.57" for 100-yr event Inflow Area = Inflow 76.43 cfs @ 12.89 hrs, Volume= 175.627 af, Incl. 40.00 cfs Base Flow = 76.41 cfs @ 12.92 hrs, Volume= Outflow 175.384 af, Atten= 0%, Lag= 2.1 min = Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 3.6 fps, Min. Travel Time= 3.7 min Avg. Velocity = 3.1 fps, Avg. Travel Time= 4.3 min Peak Depth= 3.43' @ 12.92 hrs Capacity at bank full= 104.49 cfs Inlet Invert= 512.00', Outlet Invert= 504.00'

#### 10.00' x 4.00' deep Parabolic Channel, n= 0.060 Length= 805.0' Slope= 0.0099 '/'

#### Reach r14a:

Grass lined channel

Inflow Area	a =	8.452 ac, l	nflow Depth = 3.05"	for 100-yr event
Inflow	=	17.61 cfs @	12.37 hrs, Volume=	2.146 af
Outflow	=	17.60 cfs @	12.38 hrs, Volume=	2.146 af, Atten= 0%, Lag= 0.6 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 7.0 fps, Min. Travel Time= 0.9 min Avg. Velocity = 1.7 fps, Avg. Travel Time= 3.5 min

Peak Depth= 0.75' @ 12.38 hrs Capacity at bank full= 325.42 cfs Inlet Invert= 542.00', Outlet Invert= 526.00' 10.00' x 3.00' deep Parabolic Channel, n= 0.027 Length= 360.0' Slope= 0.0444 '/'

#### Reach r14b:

30" HDPE Under Main Entrance Road

 Inflow Area =
 8.452 ac, Inflow Depth =
 3.05" for 100-yr event

 Inflow =
 17.60 cfs @
 12.38 hrs, Volume=
 2.146 af

 Outflow =
 17.60 cfs @
 12.39 hrs, Volume=
 2.146 af, Atten= 0%, Lag= 0.4 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 14.8 fps, Min. Travel Time= 0.5 min Avg. Velocity = 3.6 fps, Avg. Travel Time= 2.1 min

Peak Depth= 0.73' @ 12.39 hrs Capacity at bank full= 94.91 cfs Inlet Invert= 526.00', Outlet Invert= 505.70' 30.0" Diameter Pipe n= 0.012 Length= 445.0' Slope= 0.0456 '/'

#### Reach r14c:

#### **Overland Flow**

Inflow Area = Inflow = Outflow =	6.420 ac, Inflow Depth =1.18"for100-yr event1.75 cfs @13.91 hrs, Volume=0.633 af1.72 cfs @14.03 hrs, Volume=0.633 af, Atten= 2%, Lag= 7.6 min
Max. Velocity= 1.3	tor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs 8 fps, Min. Travel Time= 7.5 min 5 fps, Avg. Travel Time= 19.4 min

# Reach r15:

Brush Overbanks with Rocky Bottom Needs to be surveyed

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 10.7 fps, Min. Travel Time= 0.5 min Avg. Velocity = 3.7 fps, Avg. Travel Time= 1.3 min

Peak Depth= 0.99' @ 12.06 hrs Capacity at bank full= 188.47 cfs Inlet Invert= 554.00', Outlet Invert= 528.00' 5.00' x 3.00' deep Parabolic Channel, n= 0.027 Length= 290.0' Slope= 0.0897 '/'

# Reach r16:

Pipe Reach

Inflow Area =       4.702 ac, Inflow Depth = 5.13" for 100-yr event         Inflow =       20.70 cfs @ 12.04 hrs, Volume=       2.010 af         Outflow =       20.21 cfs @ 12.06 hrs, Volume=       2.010 af, Atten= 2%, Lag= 0.9 min
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 11.8 fps, Min. Travel Time= 1.2 min Avg. Velocity = 4.0 fps, Avg. Travel Time= 3.6 min
Peak Depth= 0.95' @ 12.06 hrs Capacity at bank full= 66.05 cfs Inlet Invert= 573.00', Outlet Invert= 554.00' 30.0" Diameter Pipe n= 0.012 Length= 860.0' Slope= 0.0221 '/'
Reach r18a:
Overland Flow Reach
Inflow         =         0.00 cfs @         0.00 hrs, Volume=         0.000 af           Outflow         =         0.00 cfs @         0.00 hrs, Volume=         0.000 af, Atten= 0%, Lag= 0.0 min
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 0.0 fps, Min. Travel Time= 0.0 min Avg. Velocity = 0.0 fps, Avg. Travel Time= 0.0 min
Peak Depth= 0.00' @ 0.00 hrs Capacity at bank full= 379.63 cfs Inlet Invert= 973.60', Outlet Invert= 530.00' 50.00' x 1.00' deep Parabolic Channel, n= 0.060 Length= 1,220.0' Slope= 0.3636 '/'
Reach r18b:
Overland Flow Reach
Inflow         =         0.00 cfs @         0.00 hrs, Volume=         0.000 af           Outflow         =         0.00 cfs @         0.00 hrs, Volume=         0.000 af, Atten= 0%, Lag= 0.0 min
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 0.0 fps, Min. Travel Time= 0.0 min Avg. Velocity = 0.0 fps, Avg. Travel Time= 0.0 min
Peak Depth= 0.00' @ 0.00 hrs Capacity at bank full= 151.94 cfs Inlet Invert= 530.60', Outlet Invert= 514.00' 50.00' x 1.00' deep Parabolic Channel, n= 0.060 Length= 285.0' Slope= 0.0582 '/'

Existing Conditions\_10454-01

# Reach r21a:

Man Made Ditch

Inflow Area = Inflow = Outflow =	
Max. Velocity=	-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs 8.6 fps, Min. Travel Time= 1.3 min 3.4 fps, Avg. Travel Time= 3.2 min
Capacity at ban Inlet Invert= 504	49' @ 14.32 hrs k full= 191.76 cfs 4.00', Outlet Invert= 494.00' deep Parabolic Channel, n= 0.027 Length= 648.0' Slope= 0.0154 '/'
	Reach r21b:
Grass Ditch Geometry to be	Reach r21b: confirmed by survey (inverts at pipe)
Geometry to be	confirmed by survey (inverts at pipe) 97.943 ac, Inflow Depth = 3.34" for 100-yr event

Peak Depth= 1.79' @ 12.49 hrs Capacity at bank full= 239.90 cfs Inlet Invert= 499.00', Outlet Invert= 491.10' 15.00' x 2.00' deep Parabolic Channel, n= 0.027 Length= 230.0' Slope= 0.0343 '/'

## Reach r21c:

**Overland Flow Reach** 

Inflow Area = 41.587 ac, Inflow Depth =  $3.56^{"}$  for 100-yr event Inflow = 84.28 cfs @ 12.65 hrs, Volume= 12.352 af Outflow = 84.26 cfs @ 12.66 hrs, Volume= 12.352 af, Atten= 0%, Lag= 0.5 min Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 5.0 fps, Min. Travel Time= 0.5 min Avg. Velocity = 1.9 fps, Avg. Travel Time= 1.4 min

Existing Conditions 10454-01 Type III 24-hr 100-yr Rainfall=7.00" Prepared by The Chazen Companies HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems 4/10/2006 2:34:06 PM

Peak Depth= 0.63' @ 12.66 hrs Capacity at bank full= 227.81 cfs Inlet Invert= 506.70', Outlet Invert= 485.75' 50.00' x 1.00' deep Parabolic Channel, n= 0.060 Length= 160.0' Slope= 0.1309 '/'

#### Reach r22a:

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**Overland Flow Reach** 

Inflow Area = Inflow = Outflow =	15.520 ac, Inflow Depth =2.60"for100-yr event15.21 cfs @12.94 hrs, Volume=3.366 af15.14 cfs @12.98 hrs, Volume=3.366 af, Atten= 0%, Lag= 2.8 min	
Max. Velocity= 4	Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs I.5 fps, Min. Travel Time= 3.6 min 1.3 fps, Avg. Travel Time= 12.3 min	
1 2	22' @ 12.98 hrs k full= 409.31 cfs .00',  Outlet Invert= 560.00'	

50.00' x 1.00' deep Parabolic Channel, n= 0.060 Length= 970.0' Slope= 0.4227 '/'

#### Reach r22b:

**Overland Flow Reach** 

Inflow Area = 15.520 ac, Inflow Depth =  $2.60^{\circ}$  for 100-yr event Inflow 15.14 cfs @ 12.98 hrs, Volume= 3.366 af = Outflow = 15.05 cfs @ 13.04 hrs, Volume= 3.366 af, Atten= 1%, Lag= 3.2 min Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 2.5 fps, Min. Travel Time= 4.2 min Avg. Velocity = 0.7 fps, Avg. Travel Time= 14.7 min Peak Depth= 0.32' @ 13.04 hrs Capacity at bank full= 175.17 cfs Inlet Invert= 560.00', Outlet Invert= 512.00' 50.00' x 1.00' deep Parabolic Channel, n= 0.060 Length= 620.0' Slope= 0.0774 '/'

#### Reach r25a:

Ditch Pipe inverts need to be surveyed

60.314 ac, Inflow Depth = 3.17" Inflow Area = for 100-yr event Inflow 107.81 cfs @ 12.35 hrs, Volume= 15.920 af = Outflow 107.64 cfs @ 12.36 hrs. Volume= 15.919 af, Atten= 0%, Lag= 1.1 min =

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 12.8 fps, Min. Travel Time= 1.4 min Avg. Velocity = 2.6 fps, Avg. Travel Time= 6.9 min

Peak Depth= 1.47' @ 12.36 hrs Capacity at bank full= 205.50 cfs Inlet Invert= 570.00', Outlet Invert= 504.00' 10.00' x 2.00' deep Parabolic Channel, n= 0.027 Length= 1,090.0' Slope= 0.0606 '/'

#### Reach r25b: Wetland Reach

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Wetland Reach Has wetland vegetation within reach

<b>•</b> • • •	= 20.41 cfs @	Inflow Depth = 2.95" 12.31 hrs, Volume= 12.39 hrs, Volume=	2.323 af				
Max. Veloc	Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 2.1 fps, Min. Travel Time= 6.0 min Avg. Velocity = 0.5 fps, Avg. Travel Time= 26.4 min						
Capacity at	n= 1.12' @ 12.39 hr bank full= 156.51 ( 504.00', Outlet In	sfs					

20.00' x 3.00' deep Parabolic Channel, n= 0.045 Length= 750.0' Slope= 0.0060 '/'

#### Reach r25c: Amenia Stream/Cascade Brook

(Steam) parallel to Route 22 Base Flow estimated from field observation (see field note 21)

Inflow Area = 129.431 ac, Inflow Depth = 31.57" for 100-yr event Inflow 167.68 cfs @ 12.34 hrs, Volume= 340.463 af, Incl. 40.00 cfs Base Flow = 163.72 cfs @ 12.44 hrs, Volume= Outflow = 339,781 af. Atten= 2%. Lag= 5.9 min Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

Max. Velocity= 3.0 fps, Min. Travel Time= 5.5 min Avg. Velocity = 2.7 fps, Avg. Travel Time= 6.1 min

Peak Depth= 6.85' @ 12.44 hrs Capacity at bank full= 67.14 cfs Inlet Invert= 504.00', Outlet Invert= 500.00' 10.00' x 4.00' deep Parabolic Channel, n= 0.060 Length= 975.0' Slope= 0.0041 '/'

#### Pond 8P:

No field note. Water spills over cart path; no storage.

Inflow Area	=	52.997 ac, Inflow Depth = 3.16" for 100-yr event
Inflow =	=	93.20 cfs @ 12.35 hrs, Volume= 13.952 af
Outflow =	=	93.20 cfs @ 12.35 hrs, Volume= 13.952 af, Atten= 0%, Lag= 0.0 min
Primary =	=	93.20 cfs @ 12.35 hrs, Volume= 13.952 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 575.70' @ 12.35 hrs Flood Elev= 574.70' Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= (not calculated)

#	Routing	Invert	Outlet Devices
1	Primary	574.70'	177.0 deg Sharp-Crested Vee/Trap Weir C= 2.46

Primary OutFlow Max=93.20 cfs @ 12.35 hrs HW=575.70' TW=571.47' (Dynamic Tailwater) **1=Sharp-Crested Vee/Trap Weir** (Weir Controls 93.20 cfs @ 2.5 fps)

#### Pond p04:

Field Note #13

Water ponding behind a golf cart path. Overflow dimensions are assumed based on aerial topo, and should be upgraded once survey is available.

Inflow Area =	38.062 ac, Inflow Depth = 3.36"	for 100-yr event
Inflow =	66.76 cfs @ 12.43 hrs, Volume=	10.655 af
Outflow =	66.72 cfs @ 12.44 hrs, Volume=	10.589 af, Atten= 0%, Lag= 0.6 min
Primary =	66.72 cfs @ 12.44 hrs, Volume=	10.589 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 606.06' @ 12.44 hrs Surf.Area= 6,972 sf Storage= 4,699 cf Flood Elev= 605.50' Surf.Area= 4,803 sf Storage= 2,882 cf Plug-Flow detention time= 6.5 min calculated for 10.589 af (99% of inflow) Center-of-Mass det. time= 2.6 min (861.1 - 858.5)

#	Invert	Avail.St	orage	Storage Des	scription		
1	604.20'	26,8	397 cf	Custom Sta	ge Data (Conic) L	isted below	
Elevatio (fee		Surf.Area (sq-ft)	(c	Inc.Store cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
604.2 606.0 608.0	00	0 6,650 17,060		0 3,990 22,907	0 3,990 26,897	0 6,655 17,092	
#R0	outing	Invert	Outlet I	Devices			
1 Pi	rimary	605.50'	179.0 d	leg Sharp-Cr	ested Vee/Trap W	<b>/eir</b> C= 2.46	

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#### Pond p06:

Field Note #22 Geometry to be confirmed by survey.

Inflow Area =	9.435 ac, Inflow Depth = $2.99"$	for 100-yr event
Inflow =	21.68 cfs @ 12.24 hrs, Volume=	2.352 af
Outflow =	20.41 cfs @ 12.31 hrs, Volume=	2.323 af, Atten= 6%, Lag= 3.9 min
Primary =	20.41 cfs @ 12.31 hrs, Volume=	2.323 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 506.80' Surf.Area= 18,600 sf Storage= 42,160 cf Peak Elev= 507.55' @ 12.31 hrs Surf.Area= 21,985 sf Storage= 58,063 cf (15,903 cf above start) Flood Elev= 507.10' Surf.Area= 19,958 sf Storage= 48,537 cf (6,377 cf above start) Plug-Flow detention time= 344.2 min calculated for 1.355 af (58% of inflow) Center-of-Mass det. time= 87.5 min ( 932.0 - 844.5 )

#	Invert	Avail.St	torage	Storage Des	scription		
1	500.00'	67,	669 cf	Custom Stage Data (Conic) Listed below			
Eleva (f	ntion eet)	Surf.Area (sq-ft)	((	Inc.Store cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
500	).00	0		0	0	0	
	6.80 8.00	18,600 24,030		42,160 25,509	42,160 67,669	18,672 24,138	
000	5.00	21,000		20,000	07,000	24,100	
#	Routing	Invert	Outlet	Devices			
1	Primary	506.80'	<b>12.0" x 20.0' long Culvert</b> CMP, projecting, no headwall, Ke= 0.900 Outlet Invert= 506.00' S= 0.0400 '/' n= 0.024 Cc= 0.900				
2	Primary	507.10'			ested Vee/Trap W		-

Primary OutFlow Max=20.41 cfs @ 12.31 hrs HW=507.55' TW=505.07' (Dynamic Tailwater) -1=Culvert (Inlet Controls 1.47 cfs @ 2.3 fps)

-2=Sharp-Crested Vee/Trap Weir (Weir Controls 18.94 cfs @ 1.6 fps)

# Pond p07:

Field Note # 29 Outlet geometry to be confirmed by survey.

Inflow Area	a =	7.317 ac, Inflow Depth = 3.28 for 100-yr event	
Inflow	=	19.75 cfs @ 12.19 hrs, Volume= 1.999 af	
Outflow	=	14.61 cfs @ 12.34 hrs, Volume= 1.968 af, Atten= 26%, Lag= 9.0 min	
Primary	=	14.61 cfs @ 12.34 hrs, Volume= 1.968 af	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 572.80' Surf.Area= 21,640 sf Storage= 56,264 cf Peak Elev= 573.82' @ 12.34 hrs Surf.Area= 26,456 sf Storage= 81,230 cf (24,966 cf above start) Flood Elev= 573.50' Surf.Area= 24,936 sf Storage= 73,351 cf (17,087 cf above start) Plug-Flow detention time= 627.9 min calculated for 0.676 af (34% of inflow) Center-of-Mass det. time= 145.3 min (977.4 - 832.1)

# Existing Conditions 10454-01

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#	Invert	Avail.St	torage	Storage Description			
1	565.00'	85,	557 cf	Custom Sta	age Data (Conic) L	isted below	
Elevat (fe 565 572 574	eet) 5.00 2.80	Surf.Area (sq-ft) 0 21,640 27,290	(0	Inc.Store cubic-feet) 0 56,264 29,293	Cum.Store (cubic-feet) 0 56,264 85,557	Wet.Area (sq-ft) 0 21,735 27,424	
_	Routing	Invert	Outlet	Devices	00,007	27,727	
1 F	Primary Primary	572.80' 573.50'	<b>18.0" x 20.0' long Culvert</b> CMP, projecting, no headwall, Ke= 0.900 Outlet Invert= 572.00' S= 0.0400 '/' n= 0.024 Cc= 0.900 <b>177.0 deg Sharp-Crested Vee/Trap Weir X 2.00</b> C= 2.46				
					···· ···		

Primary OutFlow Max=14.60 cfs @ 12.34 hrs HW=573.82' TW=571.47' (Dynamic Tailwater) -1=Culvert (Inlet Controls 3.49 cfs @ 2.7 fps) -2=Sharp-Crested Vee/Trap Weir (Weir Controls 11.12 cfs @ 1.4 fps)

# Pond p09:

Field Note #31 Geometry to be confirmed by survey.

Inflow Area	a =	8.452 ac, Inflow Depth = $3.10^{\circ}$ for	or 100-yr event
Inflow	=	22.20 cfs @ 12.23 hrs, Volume=	2.185 af
Outflow	=	17.61 cfs @ 12.37 hrs, Volume=	2.146 af, Atten= 21%, Lag= 8.4 min
Primary	=	17.61 cfs @ 12.37 hrs, Volume=	2.146 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 550.64' @ 12.37 hrs Surf.Area= 7,127 sf Storage= 11,266 cf Flood Elev= 551.20' Surf.Area= 8,534 sf Storage= 15,673 cf Plug-Flow detention time= 29.2 min calculated for 2.146 af (98% of inflow) Center-of-Mass det. time= 18.8 min (870.4 - 851.6)

#	Invert	Avail.St	torage Storage D	Description			
1	547.50'	21,9	989 cf Custom S	Custom Stage Data (Conic) Listed below			
Eleva	ation feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)		
54	7.50	0	0	0	0		
54	8.00	1,080	180	180	1,080		
55	0.00	5,510	6,020	6,200	5,527		
55	2.00	10,550	15,790	21,989	10,606		
#	Routing	Invert	Outlet Devices				
1	Primary	548.50'		<b>0.0" x 70.0' long Culvert</b> CMP, projecting, no headwall, Ke= 0.900 utlet Invert= 542.00' S= 0.0929 '/' n= 0.012 Cc= 0.900			
2	Primary	551.20'		) deg Sharp-Crested Vee/Trap Weir $C= 2.46$			

Primary OutFlow Max=17.61 cfs @ 12.37 hrs HW=550.64' TW=542.75' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 17.61 cfs @ 3.9 fps) -2=Sharp-Crested Vee/Trap Weir (Controls 0.00 cfs)

## Pond p10:

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Field Note #25 Need to get full story on how this pond works

Inflow Area =		45.146 ac, I	nflow Depth = 1.34"	for 100-yr event
Inflow	=	19.59 cfs @	12.37 hrs, Volume=	5.056 af
Outflow	=	0.00 cfs @	0.00 hrs, Volume=	0.000 af, Atten= 100%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 498.40' Surf.Area= 36,110 sf Storage= 101.108 cf Peak Elev= 502.91' @ 48.00 hrs Surf.Area= 65,733 sf Storage= 321,332 cf (220,224 cf above start) Plug-Flow detention time= (not calculated) Center-of-Mass det. time= (not calculated)

# Invert		Avail.Storag	ge Storage Des	cription			
1	490.00'	581,029	cf Custom Sta	Custom Stage Data (Conic) Listed below			
Elevati (fee		Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)		
490.	00	0	0	0	0		
498.	40	36,110	101,108	101,108	36,221		
500.	00	42,400	62,741	163,849	42,610		
502.	00	54,880	97,012	260,861	55,187		
504.	00	78,730	132,895	393,755	79,107		
506.	00	109,382	187,274	581,029	109,836		

#### Pond p12:

No field note. Natural depression.

Inflow Area =	6.420 ac, Inflow Depth = 2.12"	for 100-yr event
Inflow =	7.15 cfs @ 12.64 hrs, Volume=	1.136 af
Outflow =	1.75 cfs @ 13.91 hrs, Volume=	0.633 af, Atten= 76%, Lag= 75.8 min
Primary =	1.75 cfs @ 13.91 hrs, Volume=	0.633 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 547.79' @ 13.91 hrs Surf.Area= 15,481 sf Storage= 24,744 cf Flood Elev= 547.50' Surf.Area= 13,848 sf Storage= 21,762 cf Plug-Flow detention time= 290.9 min calculated for 0.633 af (56% of inflow) Center-of-Mass det. time= 162.9 min (1,062.4 - 899.5)

# Existing Conditions\_10454-01

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Hydro	DCAD® 7.00	s/n 000927	© 1986-2003 Applied	Microcomputer Sys	stems	4/10/2006	2:34:06 Pl
			U				
1	543.50'	26,9	986 cf Custom Sta	age Data (Conic) ∟	isted below		
	ation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)		
54 54	43.50 44.00 46.00 48.00	0 1,140 5,260 16,710	0 190 5,899 20,897	0 190 6,089 26,986	0 1,140 5,278 16,750		
#	Routing	Invert	Outlet Devices				
1	Primary	547.50'	173.0 deg Sharp-C	rested Vee/Trap W	<b>/eir</b> C= 2.46		

Primary OutFlow Max=1.75 cfs @ 13.91 hrs HW=547.79' TW=544.12' (Dynamic Tailwater) -1=Sharp-Crested Vee/Trap Weir (Weir Controls 1.75 cfs @ 1.3 fps)

## Pond p13:

No Field Note Natural depression.

Inflow Area	a =	0.350 ac, Infl	low Depth = 2.12"	for 100-yr event	
Inflow	=	0.74 cfs @ 1	2.13 hrs, Volume=	0.062 af	
Outflow	=	0.00 cfs @	0.00 hrs, Volume=	0.000 af, Atten= 100%	, Lag= 0.0 min
Primary	=	0.00 cfs @	0.00 hrs, Volume=	0.000 af	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 514.55' @ 24.49 hrs Surf.Area= 1,586 sf Storage= 2,698 cf Flood Elev= 519.50' Surf.Area= 4,313 sf Storage= 16,523 cf Plug-Flow detention time= (not calculated) Center-of-Mass det. time= (not calculated)

#	Invert	Avail.St	orage Storage [	Description		
1	511.40'	18,4	190 cf Custom	Stage Data (Conic)	Listed below	
Elevat (fe	ion eet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
511	.40	0	0	0	0	
512	.00	390	78	78	391	
514	.00	1,360	1,652	1,730	1,381	
516	.00	2,180	3,508	5,238	2,253	
518	.00	3,240	5,385	10,623	3,375	
520	.00	4,670	7,867	18,490	4,872	
# F	Routing	Invert	Outlet Devices			
1 F	Primary	519.50'	176.0 deg Sharp	-Crested Vee/Trap	<b>Weir</b> C= 2.46	

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=511.40' TW=497.40' (Dynamic Tailwater) ↑ 1=Sharp-Crested Vee/Trap Weir (Controls 0.00 cfs) HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems

## Pond p14:

Field Note #26 Need to figure out how this pond works

Inflow Area =	36.186 ac, Inflow Depth = 2.96"	for 100-yr event
Inflow =	58.96 cfs @ 12.39 hrs, Volume=	8.941 af
Outflow =	4.19 cfs @ 17.17 hrs, Volume=	2.331 af, Atten= 93%, Lag= 287.0 min
Primary =	4.19 cfs @ 17.17 hrs, Volume=	2.331 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 497.40' Surf.Area= 22,200 sf Storage= 54,760 cf Peak Elev= 502.91' @ 47.99 hrs Surf.Area= 78,233 sf Storage= 342,674 cf (287,914 cf above start) Plug-Flow detention time= 617.9 min calculated for 1.074 af (12% of inflow) Center-of-Mass det. time= 246.9 min (1,104.6 - 857.7)

#	Invert	Avail.Sto	rage Storage Des	scription		
1	490.00'	805,06	62 cf Custom Sta	<b>ge Data (Conic)</b> Li	sted below	
Eleva		Surf.Area	Inc.Store	Cum.Store	Wet.Area	
(f	eet)	(sq-ft)	(cubic-feet)	(cubic-feet)	(sq-ft)	
490	0.00	0	0	0	0	
497	7.40	22,200	54,760	54,760	22,286	
498	3.00	25,330	14,249	69,009	25,433	
500	0.00	52,810	76,476	145,485	52,948	
502	2.00	73,360	125,608	271,093	73,574	
504	4.00	84,070	157,308	428,402	84,467	
506	5.00	92,130	176,139	604,540	92,797	
508	3.00	108,618	200,522	805,062	109,437	
#	Routing	Invert C	Dutlet Devices			
1	Primary	500.00' <b>2</b>	4.0" x 80.0' long C	Culvert CPP, end-	section conformi	ng to fill, Ke= 0.500
	-	C	Dutlet Invert= 502.0	0' S= -0.0250 '/'	n= 0.012 Cc= 0	.900

Primary OutFlow Max=4.19 cfs @ 17.17 hrs HW=502.87' TW=501.54' (Dynamic Tailwater)

# Pond p15:

Field Note # 43 Infiltration basin

Inflow Area	a =	5.770 ac, Inflow Depth = $4$	4.50" for 100-yr event	
Inflow	=	20.92 cfs @ 12.07 hrs, Volu	ume= 2.166 af	
Outflow	=	20.80 cfs @ 12.07 hrs, Volu	ume= 1.910 af, Atten= 1%, Lag= 0.5	min
Primary	=	20.80 cfs @ 12.07 hrs, Volu	ume= 1.910 af	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 536.25' @ 12.07 hrs Surf.Area= 3,308 sf Storage= 12,076 cf Flood Elev= 536.00' Surf.Area= 3,160 sf Storage= 11,127 cf Plug-Flow detention time= 102.4 min calculated for 1.910 af (88% of inflow)

#	Invert	Avail.St	orage Stora				
1	526.80'	18,	577 cf <b>Cust</b>	om S	Stage Data (Conic)	Listed below	
Elevat (fe	tion eet)	Surf.Area (sq-ft)	Inc.S (cubic-f		Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
526	.80	0		0	0	0	
528	6.00	310		124	124	312	
530	.00	660		948	1,072	694	
532	.00	1,180	1,	815	2,887	1,256	
534	.00	1,990	3,	135	6,022	2,113	
536	6.00	3,160	5,	105	11,127	3,337	
538	6.00	4,320	7,	450	18,577	4,575	
	Routing		Outlet Device				- 0 0 40
1 F	Primary	536.00'	1/1.0 deg x	50.0	long Sharp-Crest	ed vee/ I rap Wei	r = 2.46

Center-of-Mass det. time= 44.9 min (832.3 - 787.5)

Primary OutFlow Max=20.76 cfs @ 12.07 hrs HW=536.25' TW=508.25' (Dynamic Tailwater) ←1=Sharp-Crested Vee/Trap Weir (Weir Controls 20.76 cfs @ 1.5 fps)

#### Pond p16:

Field Note # 49 Large pond with man-made island. Geometry to be verified by survey. In particular, we are making big guesses about the outlets. Also need to find out about valves...

Inflow = Outflow =	252.10 cfs @ 126.16 cfs @	Inflow Depth = 3.38" 12.34 hrs, Volume= 14.14 hrs, Volume= 14.14 hrs, Volume=	for 100-yr event 62.209 af 50.283 af, Atten= 50%, Lag= 108.2 min 50.283 af						
Routing by Dyn-Stor-Ind method, Time Span= $0.00-48.00$ hrs, dt= $0.01$ hrs									

Starting Elev= 507.00' Surf.Area= 199,799 sf Storage= 878,320 cf Peak Elev= 511.52' @ 14.14 hrs Surf.Area= 316,556 sf Storage= 1,920,462 cf (1,042,143 cf above start) Flood Elev= 510.50' Surf.Area= 271,550 sf Storage= 1,623,217 cf (744,897 cf above start) Plug-Flow detention time= 622.4 min calculated for 30.119 af (48% of inflow) Center-of-Mass det. time= 261.1 min (1,170.0 - 908.9)

#	Invert	Avail.Storag	e Storage Des	scription		
1 5	500.00'	2,062,087	cf Custom Sta	age Data (Conic)	isted below	
Elevation (feet)		Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
500.00	)	0	0	0	0	
503.00	)	140,344	140,344	140,344	140,358	
509.20	)	232,500	1,143,862	1,284,206	232,994	
510.00	)	249,400	192,720	1,476,927	249,951	
512.00	)	338,000	585,160	2,062,087	338,634	

Existing Conditions 10454-01

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#	Routing	Invert	Outlet Devices
1	Primary	509.00'	18.0" x 110.0' long Culvert CMP, projecting, no headwall, Ke= 0.900
			Outlet Invert= 505.70' S= 0.0300 '/' n= 0.024 Cc= 0.900
2	Primary	500.00'	8.0" x 100.0' long assumed equalization pipe w/ valve X 0.00
			CMP, projecting, no headwall, Ke= 0.900
			Outlet Invert= 500.00' S= 0.0000 '/' n= 0.013 Cc= 0.900
3	Primary	510.50'	175.0 deg Sharp-Crested Vee/Trap Weir X 2.00 C= 2.46

Primary OutFlow Max=126.16 cfs @ 14.14 hrs HW=511.52' TW=506.60' (Dynamic Tailwater) -1=Culvert (Inlet Controls 8.93 cfs @ 5.1 fps)

-2=assumed equalization pipe w/ valve (Controls 0.00 cfs)

-3=Sharp-Crested Vee/Trap Weir (Weir Controls 117.23 cfs @ 2.5 fps)

# Pond p17:

Field Note #45 Golf pond Geometry to be confirmed by surveyed

Inflow Are	ea =	115.991 ac, Inflow Depth = 3.21 for 100-yr event
Inflow	=	104.13 cfs @ 13.77 hrs, Volume= 31.026 af
Outflow	=	104.09 cfs @ 13.77 hrs, Volume= 31.026 af, Atten= 0%, Lag= 0.3 min
Primary	=	104.09 cfs @ 13.77 hrs, Volume= 31.026 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 523.80' Surf.Area= 7,290 sf Storage= 9,234 cf Peak Elev= 525.64' @ 13.77 hrs Surf.Area= 11,520 sf Storage= 26,666 cf (17,432 cf above start) Flood Elev= 524.30' Surf.Area= 8,074 sf Storage= 13,623 cf (4,389 cf above start) Plug-Flow detention time= 14.4 min calculated for 30.814 af (99% of inflow) Center-of-Mass det. time= 8.0 min (957.8 - 949.8)

#	Invert	Avail.S	torage	Storage Des	scription		
1	520.00'	30,	224 cf	Custom Sta	ige Data (Conic) L	isted below	
-	ation feet)	Surf.Area (sq-ft)		Inc.Store ubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
-	20.00	0		0	0	0	
52	23.80	7,290		9,234	9,234	7,313	
52	24.00	7,300		1,459	10,693	7,374	
52	26.00	12,460		19,531	30,224	12,581	
#	Routing	Invert	Outlet D	evices			
1	Primary	523.80'	Head (fe	<b>2.2' long x 0.5' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 Coef. (English) 2.80 2.92 3.08 3.30 3.32			
2 3	Primary Primary	524.30' 525.20'	143.0 de	eg Sharp-Cr	rested Vee/Trap Woong Sharp-Crested	<b>/eir</b> C= 2.47	C= 2.46

Type III 24-hr 100-yr Rainfall=7.00" Page 191 4/10/2006 2:34:06 PM Primary OutFlow Max=104.08 cfs @ 13.77 hrs HW=525.64' TW=516.23' (Dynamic Tailwater) 1=Broad-Crested Rectangular Weir (Weir Controls 18.16 cfs @ 4.5 fps) 2=Sharp-Crested Vee/Trap Weir (Weir Controls 15.22 cfs @ 2.9 fps) -3=Sharp-Crested Vee/Trap Weir (Weir Controls 70.70 cfs @ 1.9 fps)

# Pond p18:

Field Note #46 Golf pond Geometry to be confirmed by surveyed

Inflow Area	a =	139.117 ac, Inflow Depth = 3.24	for 100-yr event
Inflow	=	111.66 cfs @ 13.77 hrs, Volume	= 37.544 af
Outflow	=	111.54 cfs @ 13.79 hrs, Volume	= 37.540 af, Atten= 0%, Lag= 1.1 min
Primary	=	111.54 cfs @ 13.79 hrs, Volume	= 37.540 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 513.90' Surf.Area= 20,680 sf Storage= 26,884 cf Peak Elev= 516.23' @ 13.79 hrs Surf.Area= 29,919 sf Storage= 85,679 cf (58,795 cf above start) Flood Elev= 514.81' Surf.Area= 23,768 sf Storage= 48,709 cf (21,825 cf above start) Plug-Flow detention time= 37.6 min calculated for 36.922 af (98% of inflow) Center-of-Mass det. time= 22.5 min (961.4 - 938.9)

Avail.Stora	age Storage Des	scription		
148,28	8 cf Custom Sta	ge Data (Conic) L	isted below	
Surf.Area (sq-ft) 0 20,680 20,690 28,290 42 760	Inc.Store (cubic-feet) 0 26,884 2,068 48,782 70,554	Cum.Store (cubic-feet) 0 26,884 28,952 77,735 148,288	Wet.Area (sq-ft) 0 20,704 20,756 28,436 42,967	
		110,200	,	
He Ce	ead (feet) 0.20 0. pef. (English) 2.80	40 0.60 0.80 1.0 ) 2.92 3.08 3.30	0 3.32	r
	<b>U</b> 1	•		= 2.46
	148,28 Surf.Area (sq-ft) 0 20,680 20,690 28,290 42,760 Invert O 513.90' 2. He Co 514.81' 14	148,288 cf       Custom Sta         Surf.Area       Inc.Store         (sq-ft)       (cubic-feet)         0       0         20,680       26,884         20,690       2,068         28,290       48,782         42,760       70,554         Invert       Outlet Devices         513.90'       2.0' long x 0.5' breat         Head (feet)       0.20 0.         Coef. (English)       2.80	148,288 cf       Custom Stage Data (Conic) L         Surf.Area       Inc.Store       Cum.Store         (sq-ft)       (cubic-feet)       (cubic-feet)         0       0       0         20,680       26,884       26,884         20,690       2,068       28,952         28,290       48,782       77,735         42,760       70,554       148,288         Invert       Outlet Devices       Encode Crester         513.90'       2.0' long x 0.5' breadth Broad-Crester         Head (feet)       0.20       0.40       0.60       0.80       1.0         Coef. (English)       2.80       2.92       3.08       3.30         514.81'       143.0 deg Sharp-Crested Vee/Trap W	148,288 cf       Custom Stage Data (Conic) Listed below         Surf.Area       Inc.Store       Cum.Store       Wet.Area         (sq-ft)       (cubic-feet)       (sq-ft)         0       0       0       0         20,680       26,884       26,884       20,704         20,690       2,068       28,952       20,756         28,290       48,782       77,735       28,436         42,760       70,554       148,288       42,967         Invert       Outlet Devices       Example       Example         513.90'       2.0' long x 0.5' breadth Broad-Crested Rectangular Wein       Head (feet) 0.20 0.40 0.60 0.80 1.00         Coef. (English) 2.80 2.92 3.08 3.30 3.32       514.81'       143.0 deg Sharp-Crested Vee/Trap Weir       C= 2.47

**Primary OutFlow** Max=111.53 cfs @ 13.79 hrs HW=516.23' TW=511.47' (Dynamic Tailwater) **1=Broad-Crested Rectangular Weir** (Weir Controls 23.54 cfs @ 5.1 fps)

-2=Sharp-Crested Vee/Trap Weir (Weir Controls 23.34 Cls @ 3.1 lps)

-3=Sharp-Crested Vee/Trap Weir (Weir Controls 70.40 cfs @ 2.5 fps)

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## Pond p19:

Wetland Geometry to be confirmed by survey Based off aerial topo, and assumed topo contour

Inflow Area =	15.520 ac, Inflow Depth = $2.60^{\circ}$	for 100-yr event
Inflow =	22.44 cfs @ 12.61 hrs, Volume=	3.367 af
Outflow =	15.21 cfs @ 12.94 hrs, Volume=	3.366 af, Atten= 32%, Lag= 19.9 min
Primary =	15.21 cfs @ 12.94 hrs, Volume=	3.366 af
Secondary =	0.00 cfs @ 0.00 hrs, Volume=	0.000 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 972.00' Surf.Area= 86,000 sf Storage= 57,333 cf Peak Elev= 972.29' @ 12.94 hrs Surf.Area= 93,885 sf Storage= 89,433 cf (32,099 cf above start) Plug-Flow detention time= 278.9 min calculated for 2.049 af (61% of inflow) Center-of-Mass det. time= 57.4 min (942.9 - 885.4)

#	Invert	Avail.Sto	orage Storage De	escription		
1	970.00'	282,3	329 cf Custom St	f Custom Stage Data (Conic) Listed below		
Elevati (fe	on et)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft <u>)</u>	
970.	00	0	0	0	0	
972.	00	86,000	57,333	57,333	86,006	
974.	00	141,270	224,996	282,329	141,327	
	Routing		Outlet Devices			0 0 40

 1
 Secondary
 973.60'
 178.0 deg x 51.0' long Sharp-Crested Vee/Trap Weir
 C= 2.46

 2
 Primary
 972.00'
 35.0' long x 0.5' breadth Broad-Crested Rectangular Weir
 Head (feet)
 0.20
 0.40
 0.60
 0.80
 1.00

 Coef. (English)
 2.80
 2.92
 3.08
 3.30
 3.32

Primary OutFlow Max=15.21 cfs @ 12.94 hrs HW=972.29' TW=970.22' (Dynamic Tailwater) ←2=Broad-Crested Rectangular Weir (Weir Controls 15.21 cfs @ 1.5 fps)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=972.00' TW=973.60' (Dynamic Tailwater)

# Pond p20:

Field Note #50 Spring Fed Pond Geometry to be confirmed by surveyed

 Inflow Area =
 241.484 ac, Inflow Depth =
 2.83" for 100-yr event

 Inflow =
 132.70 cfs @
 14.12 hrs, Volume=
 56.893 af

 Outflow =
 129.80 cfs @
 14.30 hrs, Volume=
 56.033 af, Atten= 2%, Lag= 10.7 min

 Primary =
 129.80 cfs @
 14.30 hrs, Volume=
 56.033 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

Existing Conditions_10454-01	Type III 24-hr 100-yr Rainfall=7.00"
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Starting Elev= 505.10' Surf.Area= 89,370 sf Storage= 138,524 cf Peak Elev= 506.61' @ 14.32 hrs Surf.Area= 92,408 sf Storage= 276,664 cf (138,141 cf above start) Plug-Flow detention time= 157.0 min calculated for 52.842 af (93% of inflow) Center-of-Mass det. time= 41.5 min (1,172.6 - 1,131.1)

#	Invert	Avail.St	torage Storage Description		
1	502.00'	615,	682 cf Custom Stage Data (Prismatic) Listed below		
	ration	Surf.Area	Inc.Store Cum.Store		
	(feet)	(sq-ft)	(cubic-feet) (cubic-feet)		
50	02.00	0	0 0		
50	05.10	89,370	138,524 138,524		
50	06.00	89,380	80,437 218,961		
50	08.00	99,280	188,660 407,621		
5	10.00	108,781	208,061 615,682		
#	Routing	Invert	Outlet Devices		
1	Primary	505.10'	3.0' long x 1.5' breadth Broad-Crested Rectangular Weir		
	-		Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50		
			3.00		
			Coef. (English) 2.62 2.64 2.64 2.68 2.75 2.86 2.92 3.07 3.07 3.03 3.28		
			3.32		
2	Primary	506.20'	6.5' long x 1.5' breadth Broad-Crested Rectangular Weir		
			Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50		
			3.00		
			Coef. (English) 2.62 2.64 2.64 2.68 2.75 2.86 2.92 3.07 3.07 3.03 3.28		
			3.32		
3	Primary	506.00'	176.0 deg x 97.0' long Sharp-Crested Vee/Trap Weir C= 2.46		
	Primary OutFlow Max=129.71 cfs @ 14.30 hrs HW=506.61' TW=506.49' (Dynamic Tailwater)				

**1=Broad-Crested Rectangular Weir** (Weir Controls 7.47 cfs @ 1.6 fps)

**2=Broad-Crested Rectangular Weir** (Weir Controls 3.25 cfs @ 1.2 fps)

-3=Sharp-Crested Vee/Trap Weir (Weir Controls 118.99 cfs @ 1.7 fps)

#### Pond p21:

Inflow Area	a =	489.305 ac, Inflow Depth = 3.19	for 100-yr event
Inflow	=	497.54 cfs @ 12.44 hrs, Volume=	= 130.048 af
Outflow	=	38.56 cfs @ 21.28 hrs, Volume=	= 101.325 af, Atten= 92%, Lag= 530.4 min
Primary	=	38.56 cfs @ 21.28 hrs, Volume=	= 101.325 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 486.84' @ 21.28 hrs Surf.Area= 1,369,088 sf Storage= 3,632,868 cf Plug-Flow detention time= 905.3 min calculated for 101.325 af (78% of inflow) Center-of-Mass det. time= 752.6 min (1,745.4 - 992.8)

#	Invert	Avail.Storage	Storage Description
1	480.40'	5,244,885 cf	Custom Stage Data (Conic) Listed below

# Existing Conditions\_10454-01

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Elevation	Surf.Area	Inc.Store	Cum.Store	Wet.Area
(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)	(sq-ft)
480.40	0	0	0	0
482.00 202,230		107,856	107,856	202,234
484.00	485,198	667,114	774,970	485,231
486.00 1,275,4		1,698,237	2,473,208	1,275,541
488.00	1,499,208	2,771,678	5,244,885	1,499,423
# Routing	Invert	Outlet Devices		
	400 40'	20.0" x 70.0' lang (	Subvert CMD pres	acting no boodu

1 Primary 480.40' **30.0" x 70.0' long Culvert** CMP, projecting, no headwall, Ke= 0.900 Outlet Invert= 480.40' S= 0.0000 '/' n= 0.024 Cc= 0.900

Primary OutFlow Max=38.56 cfs @ 21.28 hrs HW=486.84' TW=0.00' (Dynamic Tailwater) ↓ 1=Culvert (Barrel Controls 38.56 cfs @ 7.9 fps)

#### Pond p22:

Field Note #54 Golf Pond Geometry to be confirmed by survey

Inflow Are	a =	97.943 ac, Inflow Depth = 3.37" for 100-yr event
Inflow	=	190.35 cfs @ 12.45 hrs, Volume= 27.541 af
Outflow	=	189.22 cfs @ 12.49 hrs, Volume= 27.242 af, Atten= 1%, Lag= 2.2 min
Primary	=	189.22 cfs @ 12.49 hrs, Volume= 27.242 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 498.10' Surf.Area= 6,520 sf Storage= 10,106 cf Peak Elev= 502.72' @ 12.49 hrs Surf.Area= 12,617 sf Storage= 53,629 cf (43,523 cf above start) Plug-Flow detention time= 25.6 min calculated for 27.005 af (98% of inflow) Center-of-Mass det. time= 11.9 min (880.1 - 868.2)

#	Invert	Avail.St	prage Storage Description			
1	495.00'	143,	770 cf Custo	om Sta	ige Data (Prisma	atic) Listed below
Eleva		Surf.Area	Inc.St		Cum.Store	
(f	feet)	(sq-ft)	(cubic-fe	et)	(cubic-feet)	
49	5.00	0		0	0	
498	8.10	6,520	10,1	106	10,106	
500	0.00	8,390	14,1	164	24,270	
502	2.00	11,530	19,9	920	44,190	
504	4.00	14,530	26,0	060	70,250	
50	6.00	18,340	32,8	370	103,120	
508	8.00	22,310	40,6	650	143,770	
#	Routing	Invert	<b>Outlet Device</b>	s		
1	Primary	499.75'	18.0" x 21.0'	long (	Culvert CMP, p	rojecting, no headwall, Ke= 0.900
	-		Outlet Invert=	499.7	5' S= 0.0000 '/'	n= 0.024 Cc= 0.900
2	Primary	500.50'				ested Rectangular Weir
			Head (feet) C	.20 0.	.40 0.60 0.80 1	.00 1.20 1.40 1.60
			Coef. (English	n) 2.68	3 2.70 2.70 2.6	4 2.63 2.64 2.64 2.63

Existing Conditions_10454-01	Type III 24-hr 100-yr Rainfall=7.00"
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20.0' long x 13.5' breadth Broad-Crested Rectangular Weir 3 Primary 500.50' Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.62 2.66 2.70 2.66 2.65 2.66 2.65 2.63

Primary OutFlow Max=189.17 cfs @ 12.49 hrs HW=502.72' TW=500.79' (Dynamic Tailwater) -1=Culvert (Inlet Controls 9.34 cfs @ 5.3 fps) -2=Broad-Crested Rectangular Weir (Weir Controls 8.56 cfs @ 3.9 fps)

-3=Broad-Crested Rectangular Weir (Weir Controls 171.27 cfs @ 3.9 fps)

## Pond p23:

Inflow Are	a =	41.587 ac, Inflow Depth = $3.8$	33" for 100-yr event
Inflow	=	84.32 cfs @ 12.64 hrs, Volum	ne= 13.270 af
Outflow	=	84.28 cfs @ 12.65 hrs, Volum	ne= 12.352 af, Atten= 0%, Lag= 0.7 min
Primary	=	84.28 cfs @ 12.65 hrs, Volum	ne= 12.352 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 507.97' @ 12.65 hrs Surf.Area= 19,354 sf Storage= 44,202 cf Plug-Flow detention time= 52.4 min calculated for 12.349 af (93% of inflow) Center-of-Mass det. time= 16.2 min (880.6 - 864.4)

#	Invert	Avail.St	prage Storage Description		
1	503.50'	100,3	303 cf Custom	Stage Data (Pris	matic) Listed below
Eleva (	ation feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet	Cum.Stor (cubic-fee	-
	3.50	0	(	40.00	0
	)6.00 )8.00	11,170 19,460	13,963 30,630	13,96 44,59	
51	0.00	36,250	55,710	100,30	3
#	Routing	Invert	Outlet Devices		
1	Primary	507.70'	178.0 deg x 178	0' long Sharp-C	crested Vee/Trap Weir C= 2.46

Primary OutFlow Max=84.27 cfs @ 12.65 hrs HW=507.97' TW=507.33' (Dynamic Tailwater) **1=Sharp-Crested Vee/Trap Weir** (Weir Controls 84.27 cfs @ 1.6 fps)

# Pond zDP1: Design Point 1

Field note #10. Culvert dimensions to be confirmed by survey.

Inflow Area	1 =	26.659 ac, Inflow	Depth = 3.47"	for 100-yr event	
Inflow	=	54.23 cfs @ 12.48	3 hrs, Volume=	7.707 af	
Outflow	=	54.22 cfs @ 12.48	3 hrs, Volume=	7.707 af, Atten= 0%, Lag= 0.1	1 min
Primary	=	54.22 cfs @ 12.48	3 hrs, Volume=	7.707 af	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 723.21' @ 12.48 hrs Surf.Area= 241 sf Storage= 301 cf Flood Elev= 727.00' Surf.Area= 1,105 sf Storage= 2,619 cf Plug-Flow detention time= 0.2 min calculated for 7.707 af (100% of inflow)

# Invert	Avail.Stora	age Storage Des	scription	
1 720.10'	3,70	6 cf Custom Sta	i <b>ge Data (Conic)</b> Li	sted below
Elevation	Surf.Area	Inc.Store	Cum.Store	Wet.Area
(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)	(sq-ft)
720.10	0	0	0	0
722.00	90	57	57	96
724.00	340	403	460	364
726.00	760	1,072	1,533	815
728.00	1,450	2,173	3,706	1,543

Center-of-Mass det. time= 0.1 min (862.3 - 862.2)

_	Ħ	Routing	Invert	Outlet Devices
	1	Primary	720.10'	42.0" x 120.0' long Culvert CMP, square edge headwall, Ke= 0.500
				Outlet Invert= 700.00' S= 0.1675 '/' n= 0.024 Cc= 0.900
	2	Primary	727.00'	155.0 deg Sharp-Crested Vee/Trap Weir C= 2.47

Primary OutFlow Max=54.22 cfs @ 12.48 hrs HW=723.21' TW=686.95' (Dynamic Tailwater) -1=Culvert (Inlet Controls 54.22 cfs @ 6.0 fps) -2=Sharp-Crested Vee/Trap Weir (Controls 0.00 cfs)

Pond zDP2: Design Point 2

Field Note #15 Culvert dimensions to be confirmed by survey Overflow to ditch is currently discarded... We may have to model that area...

Inflow Area = Inflow = Outflow = Discarded =	136.27 cfs @ 12.87 136.21 cfs @ 12.87 99.78 cfs @ 12.87	hrs, Volume= hrs, Volume= hrs, Volume=	25.263 af 25.263 af, <i>A</i> 8.347 af	Atten= 0%, Lag= 0.2 min
Primary =	36.43 cfs @ 12.87	hrs, Volume=	16.917 af	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 626.40' @ 12.87 hrs Surf.Area= 1,652 sf Storage= 4,127 cf Flood Elev= 624.50' Surf.Area= 925 sf Storage= 1,728 cf Plug-Flow detention time= 0.7 min calculated for 25.263 af (100% of inflow) Center-of-Mass det. time= 0.5 min (893.8 - 893.3)

#	Routing	Invert	Outlet Devices
1	Primary	619.60'	24.0" x 150.0' long Culvert RCP, end-section conforming to fill, Ke= 0.500
			Outlet Invert= 608.00' S= 0.0773 '/' n= 0.012 Cc= 0.900
2	Discarded	624.50'	166.0 deg Sharp-Crested Vee/Trap Weir C= 2.46

Discarded OutFlow Max=99.76 cfs @ 12.87 hrs HW=626.40' (Free Discharge) ← 2=Sharp-Crested Vee/Trap Weir (Weir Controls 99.76 cfs @ 3.4 fps)

**Primary OutFlow** Max=36.43 cfs @ 12.87 hrs HW=626.40' TW=607.69' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 36.43 cfs @ 11.6 fps)

# Pond zDP3: Design Point 3

Inflow Are	a =	212.742 ac, I	nflow Depth = 20.4	40" for	100-yr event		
Inflow	=	319.24 cfs @	12.38 hrs, Volun	ne=	361.644 af		
Primary	=	319.24 cfs @	12.38 hrs, Volun	ne=	361.644 af,	Atten= 0%,	Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

## Pond zDP4: Design Point 4

Inflow Area	a =	489.305 ac, Inflow Depth = 2.48"	for	100-yr event
Inflow	=	38.56 cfs @ 21.28 hrs, Volume=	:	101.325 af
Primary	=	38.56 cfs @ 21.28 hrs, Volume=	í.	101.325 af, Atten= 0%, Lag= 0.0 min

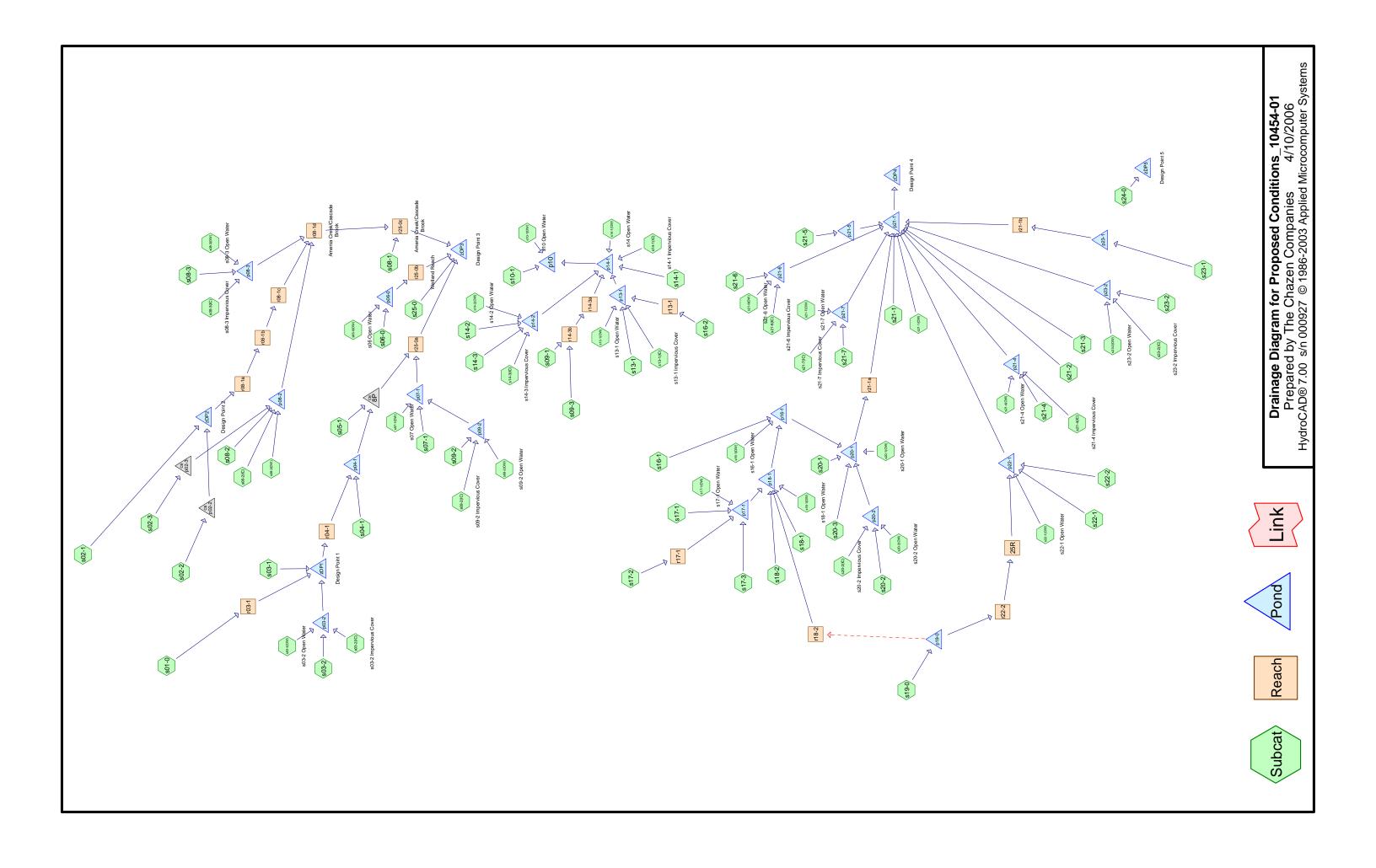
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

# Pond zDP5: Design Point 5

Inflow Area =		28.325 ac, Inflow Depth = 3.62"	for 100-yr event
Inflow	=	67.08 cfs @ 12.45 hrs, Volume=	8.541 af
Primary	=	67.08 cfs @ 12.45 hrs, Volume=	8.541 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

# Appendix K: Post-Development Watershed Conditions Modeling



# Post-Development Conditions 1 year 24 hour Storm Event Model Computations

Proposed Conditions_10454-01Type III 24-hr 1-yr Rainfall=2.70"Prepared by The Chazen CompaniesPage 1HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems4/10/2006 3:14:46 PM		
Subcatchment s01-0:		
Runoff = 2.38 cfs @ 12.70 hrs, Volume= 0.458 af, Depth= 0.48"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 1-yr Rainfall=2.70"		
Area (ac) CN Description		
11.485 68		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
42.8 Direct Entry,		
Subcatchment s02-1:		
Runoff = 10.07 cfs @ 13.01 hrs, Volume= 2.681 af, Depth= 0.38"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 1-yr Rainfall=2.70"		
Area (ac) CN Description		
85.591 65		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
61.3 Direct Entry,		
Subcatchment s02-2:		
Runoff = 1.52 cfs @ 12.54 hrs, Volume= 0.265 af, Depth= 0.41"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 1-yr Rainfall=2.70"		
Area (ac) CN Description		
7.776 66		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
29.4   Direct Entry,		
Subcatchment s02-3:		
Runoff = 7.91 cfs @ 12.03 hrs, Volume= 0.480 af, Depth= 1.41"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs		

Proposed Conditions_10454-01	Type III 24-hr 1-yr Rainfall=2.70"	
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Area (ac) CN Description 4.088 86		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
1.6Direct Entry,		
Subcatchment s03-1:		
Runoff = 3.22 cfs @ 12.48 hrs, Volume= 0.482 af,	Depth= 0.55"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, Type III 24-hr 1-yr Rainfall=2.70"	dt= 0.01 hrs	
Area (ac) CN Description		
10.435 70		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
28.8 Direct Entry,		
Subcatchment s03-2:		
Runoff = 2.72 cfs @ 12.03 hrs, Volume= 0.182 af,	Depth= 0.72"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, Type III 24-hr 1-yr Rainfall=2.70"	dt= 0.01 hrs	
Area (ac) CN Description		
3.021 74		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
1.5 Direct Entry,		
Subcatchment s03-2(IC): s03-2 Impervious Cover		
Runoff = 5.09 cfs @ 12.02 hrs, Volume= 0.342 af,	Depth= 2.47"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, Type III 24-hr 1-yr Rainfall=2.70"	dt= 0.01 hrs	
Area (ac) CN Description		
1.663 98		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
1.5 Direct Entry,		

Proposed Conditions_10454-01 Prepared by The Chazen Companies HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer System	Type III 24-hr 1-yr         Rainfall=2.70"           Page 3           ns         4/10/2006 3:14:46 PM	
Subcatchment s03-2(OW): s03-2 Ope	en Water	
Runoff = 0.18 cfs @ 12.00 hrs, Volume= 0.012 af,	Depth= 2.70"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, Type III 24-hr 1-yr Rainfall=2.70"	dt= 0.01 hrs	
Area (ac) CN Description		
0.054 100		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
0.0 Direct Entry,		
Subcatchment s04-1:		
Runoff = 3.19 cfs @ 12.11 hrs, Volume= 0.301 af,	Depth= 0.48"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, Type III 24-hr 1-yr Rainfall=2.70"	dt= 0.01 hrs	
Area (ac) CN Description		
7.549 68		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
6.1 Direct Entry,		
Subcatchment s05-1:		
Runoff = 0.77 cfs @ 12.42 hrs, Volume= 0.147 af,	Depth= 0.26"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 1-yr Rainfall=2.70"		
Area (ac) CN Description		
6.842 61		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
14.4   Direct Entry,		
Subcatchment s06-0:		
Runoff = 1.16 cfs @ 12.44 hrs, Volume= 0.215 af,	Depth= 0.29"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 brs	dt= 0.01 brs	

Proposed Conditions_10454-01	Type III 24-hr 1-yr Rainfall=2.70"	
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	<u></u>	
Area (ac) CN Description 9.007 62		
3.007 02		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
17.3 Direct Entry,		
Subcatchment s06-0(OW): s06 Ope	n Water	
Runoff = 1.39 cfs @ 12.00 hrs, Volume= 0.096 af,	Depth= 2.70"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 1-yr Rainfall=2.70"		
Area (ac) CN Description		
0.428 100		
Subcatchment s07-1:		
Runoff = 0.95 cfs @ 12.19 hrs, Volume= 0.134 af,	Depth= 0.34"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 1-yr Rainfall=2.70"		
Area (ac) CN Description 4.656 64		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
9.3 Direct Entry,		
Subcatchment s07-1(OW): s07 Open Water		
Runoff = 1.64 cfs @ 12.00 hrs, Volume= 0.114 af,	Depth= 2.70"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, Type III 24-hr 1-yr Rainfall=2.70"	dt= 0.01 hrs	
Area (ac) CN Description		
0.506 100		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
0.0 Direct Entry,		

	Type III 24-hr 1-yr Rainfall=2.70"	
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Subcatchment s08-1:		
Runoff = 1.89 cfs @ 12.60 hrs, Volume= 0.448 af,	Depth= 0.23"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, d Type III 24-hr 1-yr Rainfall=2.70"	lt= 0.01 hrs	
Area (ac) CN Description 23.126 60		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
24.5Direct Entry,		
Subcatchment s08-2:		
Runoff = 1.46 cfs @ 12.27 hrs, Volume= 0.235 af,	Depth= 0.31"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, d Type III 24-hr 1-yr Rainfall=2.70"	it= 0.01 hrs	
Area (ac) CN Description 8.958 63		
Tc Length Slope Velocity Capacity Description		
(min) (feet) (ft/ft) (ft/sec) (cfs) 11.4 Direct Entry,		
Subcatchment s08-2(IC):		
Runoff = 16.10 cfs @ 12.04 hrs, Volume= 1.137 af,	Depth= 2.47"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 1-yr Rainfall=2.70"		
Area (ac) CN Description 5.524 98		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
2.9 Direct Entry,		
Subcatchment s08-2(OW):		
Runoff = 0.62 cfs @ 12.00 hrs, Volume= 0.043 af,	Depth= 2.70"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 1-yr Rainfall=2.70"		

Proposed Conditions_10454-01 Type Prepared by The Chazen Companies	be III 24-hr 1-yr Rainfall=2.70" Page 6	
HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems	4/10/2006 3:14:46 PM	
Area (ap) CN Departmention		
Area (ac) CN Description 0.192 100		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
0.0 Direct Entry,		
Subcatchment s08-3:		
Runoff = 0.19 cfs @ 12.41 hrs, Volume= 0.037 af, De	pth= 0.26"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 1-yr Rainfall=2.70"		
Area (ac) CN Description		
1.700 61		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
13.6 Direct Entry,		
Subcatchment s08-3(IC): s08-3 Impervious	s Cover	
Runoff = 3.38 cfs @ 12.01 hrs, Volume= 0.224 af, De	pth= 2.47"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 1-yr Rainfall=2.70"		
Area (ac) CN Description		
1.086 98		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
1.0 Direct Entry,		
Subcatchment s08-3(OW): s08-3 Open Water		
Runoff = 0.14 cfs @ 12.00 hrs, Volume= 0.009 af, De	pth= 2.70"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0 Type III 24-hr 1-yr Rainfall=2.70"	0.01 hrs	
Area (ac) CN Description		
0.042 100		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
0.0 Direct Entry,		

Proposed Conditions_10454-01	Type III 24-hr 1-yr Rainfall=2.70"	
Prepared by The Chazen Companies HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer System	Page 7 ns 4/10/2006 3:14:46 PM	
Subcatchment s09-1:		
Runoff = 0.26 cfs @ 12.35 hrs, Volume= 0.050 af,	Depth= 0.23"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, Type III 24-hr 1-yr Rainfall=2.70"	dt= 0.01 hrs	
Area (ac) CN Description		
2.604 60		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
9.0 Direct Entry,		
Subcatchment s09-2:		
Runoff = 4.69 cfs @ 12.38 hrs, Volume= 0.687 af,	Depth= 0.44"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, Type III 24-hr 1-yr Rainfall=2.70"	dt= 0.01 hrs	
Area (ac) CN Description		
18.608 67		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
20.6 Direct Entry,		
Subcatchment s09-2(IC): s09-2 Impervi	ious Cover	
Runoff = 6.90 cfs @ 12.04 hrs, Volume= 0.481 af,	Depth= 2.47"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 1-yr Rainfall=2.70"		
Area (ac) CN Description 2.336 98		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
2.5 Direct Entry,		
Subcatchment s09-2(OW): s09-2 Open Water		
Runoff = 0.76 cfs @ 12.00 hrs, Volume= 0.053 af,	Depth= 2.70"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 1-yr Rainfall=2.70"		

Proposed Conditions_10454-01 Type III 24-hr 1-yr Rainfall=2.70"		
Prepared by The Chazen Companies Page 8 HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems 4/10/2006 3:14:46 PM		
Area (ac) CN Description 0.236 100		
0.236 100		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
0.0 Direct Entry,		
Subcatchment s09-3:		
Runoff = 1.35 cfs @ 12.19 hrs, Volume= 0.152 af, Depth= 0.48"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 1-yr Rainfall=2.70"		
Area (ac) CN Description		
3.818 68		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
10.9 Direct Entry,		
Subcatchment s10-1:		
Subcatenment STO-1.		
Runoff = 2.04 cfs @ 12.49 hrs, Volume= 0.321 af, Depth= 0.48"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 1-yr Rainfall=2.70"		
Area (ac) CN Description		
8.038 68		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
27.9 Direct Entry,		
Subastabrant a10 1/040, a10 Open Water		
Subcatchment s10-1(OW): s10 Open Water		
Runoff = 2.69 cfs @ 12.00 hrs, Volume= 0.187 af, Depth= 2.70"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 1-yr Rainfall=2.70"		
Area (ac) CN Description		
0.830 100		

Proposed Conditions_10454-01Type III 24-hr 1-yr Rainfall=2.70"Prepared by The Chazen CompaniesPage 9HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems4/10/2006 3:14:47 PM		
Subcatchment s13-1:		
Runoff = 0.48 cfs @ 12.10 hrs, Volume= 0.077 af, Depth= 0.26"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 1-yr Rainfall=2.70"		
Area (ac) CN Description 3.555 61		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
2.8 Direct Entry,		
Subcatchment s13-1(IC): s13-1 Impervious Cover		
Runoff = 18.60 cfs @ 12.04 hrs, Volume= 1.309 af, Depth= 2.47"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 1-yr Rainfall=2.70"		
Area (ac) CN Description		
6.360 98		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
2.8Direct Entry,		
Subcatchment s13-1(OW): s13-1 Open Water		
Runoff = 0.42 cfs @ 12.00 hrs, Volume= 0.029 af, Depth= 2.70"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 1-yr Rainfall=2.70"		
Area (ac) CN Description		
0.131 100		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
0.0 Direct Entry,		
Subcatchment s14-1:		
Runoff = 3.87 cfs @ 12.48 hrs, Volume= 0.590 af, Depth= 0.52"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 1-yr Rainfall=2.70"		

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Area (ac) CN Description 13.727 69		
13.727 09		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
28.1 Direct Entry,		
Subcatchment s14-1(IC): s14-1 Impervious Cover		
Runoff = 5.48 cfs @ 12.03 hrs, Volume= 0.379 af, Depth= 2.47"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 1-yr Rainfall=2.70"		
Area (ac) CN Description		
1.840 98		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
2.3   Direct Entry,		
Subcatchment s14-1(OW): s14 Open Water		
Runoff = 1.68 cfs @ 12.00 hrs, Volume= 0.117 af, Depth= 2.70"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 1-yr Rainfall=2.70"		
Area (ac) CN Description		
0.518 100		
Subcatchment s14-2:		
Runoff = 0.06 cfs @ 12.29 hrs, Volume= 0.011 af, Depth= 0.26"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 1-yr Rainfall=2.70"		
Area (ac) CN Description		
0.504 61		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
6.4 Direct Entry,		

Proposed Conditions_10454-01	Type III 24-hr 1-yr Rainfall=2.70"	
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Subcatchment s14-2(OW): s14-2 Open Water		
Runoff = 0.57 cfs @ 12.00 hrs, Volume= 0.040 af,	Depth= 2.70"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, Type III 24-hr 1-yr Rainfall=2.70"	dt= 0.01 hrs	
Area (ac) CN Description 0.176 100		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
0.0 Direct Entry,		
Subcatchment s14-3:		
Runoff = 2.95 cfs @ 12.14 hrs, Volume= 0.292 af,	Depth= 0.52"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 1-yr Rainfall=2.70"		
Area (ac) CN Description		
6.794 69		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
8.5 Direct Entry,		
Subcatchment s14-3(IC): s14-3 Impervious Cover		
Runoff = 24.66 cfs @ 12.04 hrs, Volume= 1.741 af,	Depth= 2.47"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 1-yr Rainfall=2.70"		
Area (ac) CN Description		
8.460 98		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
2.9Direct Entry,		
Subcatchment s16-1:		
Runoff = 10.27 cfs @ 12.35 hrs, Volume= 1.465 af,	Depth= 0.44"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs		

Proposed Conditions_10454-01	Type III 24-hr 1-yr Rainfall=2.70"	
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Area (ac) CN Description		
39.680 67		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
19.0 Direct Entry,		
Subcatchment s16-1(OW): s16-1 Op	en Water	
Runoff = 17.34 cfs @ 12.00 hrs, Volume= 1.204 af	, Depth= 2.70"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, Type III 24-hr 1-yr Rainfall=2.70"	dt= 0.01 hrs	
Area (ac) CN Description		
5.351 100		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
0.0 Direct Entry,		
Subcatchment s16-2:		
Runoff = 1.43 cfs @ 12.24 hrs, Volume= 0.148 af	, Depth= 0.82"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 1-yr Rainfall=2.70"		
Area (ac) CN Description		
2.176 76		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
15.8 Direct Entry,		
Subcatchment s17-1:		
Runoff = 0.86 cfs @ 12.65 hrs, Volume= 0.175 af	, Depth= 0.34"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 1-yr Rainfall=2.70"		
Area (ac) CN Description		
6.110 64		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
33.8 Direct Entry,		

Proposed Conditions_10454-01	Type III 24-hr 1-yr Rainfall=2.70"
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Subcatchment s17-1(OW): s17-1 Ope	en Water
Runoff = 0.53 cfs @ 12.00 hrs, Volume= 0.037 af,	Depth= 2.70"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, Type III 24-hr 1-yr Rainfall=2.70"	dt= 0.01 hrs
Area (ac) CN Description	
0.164 100	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
0.0 Direct Entry,	
Subcatchment s17-2:	
Runoff = 7.69 cfs @ 13.51 hrs, Volume= 2.591 af,	Depth= 0.41"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, Type III 24-hr 1-yr Rainfall=2.70"	dt= 0.01 hrs
Area (ac) CN Description	
76.086 66	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
95.7 Direct Entry,	
Subcatchment s17-3:	
Runoff = 5.74 cfs @ 12.55 hrs, Volume= 1.018 af,	Depth= 0.41"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, Type III 24-hr 1-yr Rainfall=2.70"	dt= 0.01 hrs
Area (ac) CN Description	
29.880 66	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
30.7 Direct Entry,	
Subcatchment s18-1:	
Runoff = 1.50 cfs @ 12.34 hrs, Volume= 0.242 af,	Depth= 0.34"

Proposed Conditions_10454-01	Type III 24-hr 1-yr Rainfall=2.70"
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	1,10,2000 0.1 1.1 T M
Area (ac) CN Description	
8.429 64	
Tc Length Slope Velocity Capacity Description	
(min) (feet) (ft/ft) (ft/sec) (cfs) 16.4 Direct Entry,	
Subcatchment s18-1(OW): s18-1 Ope	en Water
Runoff = 1.53 cfs @ 12.00 hrs, Volume= 0.106 af,	Depth= 2.70"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, Type III 24-hr 1-yr Rainfall=2.70"	dt= 0.01 hrs
Area (ac) CN Description	
0.472 100	
Subcatchment s18-2:	
Runoff = 3.57 cfs @ 12.30 hrs, Volume= 0.461 af,	Depth= 0.52"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, Type III 24-hr 1-yr Rainfall=2.70"	dt= 0.01 hrs
Area (ac) CN Description	
10.721 69	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
18.0 <b>Direct Entry</b> ,	
Subcatchment s19-0:	
Runoff = 1.03 cfs @ 12.80 hrs, Volume= 0.301 af,	Depth= 0.23"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, Type III 24-hr 1-yr Rainfall=2.70"	dt= 0.01 hrs
Area (ac) CN Description	
Area (ac) CN Description 15.520 60	

Proposed Conditions_10454-01Type III 24-hr 1-yr Rainfall=2.70"Prepared by The Chazen CompaniesPage 15HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems4/10/2006 3:14:47 PM
Subcatchment s20-1:
Runoff = 1.87 cfs @ 12.40 hrs, Volume= 0.292 af, Depth= 0.41"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 1-yr Rainfall=2.70"
Area (ac) CN Description 8.559 66
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
21.5 Direct Entry,
Subcatchment s20-1(OW): s20-1 Open Water
Runoff = 6.38 cfs @ 12.00 hrs, Volume= 0.443 af, Depth= 2.70"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 1-yr Rainfall=2.70"
Area (ac) CN Description
1.968 100
Subcatchment s20-2:
Runoff = 5.74 cfs @ 12.13 hrs, Volume= 0.492 af, Depth= 0.72"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 1-yr Rainfall=2.70"
Area (ac) CN Description 8.157 74
Tc Length Slope Velocity Capacity Description
(min) (feet) (ft/ft) (ft/sec) (cfs)
8.4 Direct Entry,
Subcatchment s20-2(IC): s20-2 Impervious Cover
Runoff = 13.96 cfs @ 12.07 hrs, Volume= 1.052 af, Depth= 2.47"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 1-yr Rainfall=2.70"
Area (ac) CN Description 5.112 98

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Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
4.7 Direct Entry,	
Subcatchment s20-2(OW): s20-2 Ope	en Water
Runoff = 0.78 cfs @ 12.00 hrs, Volume= 0.054 af,	Depth= 2.70"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, Type III 24-hr 1-yr Rainfall=2.70"	dt= 0.01 hrs
Area (ac) CN Description	
0.242 100	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
0.0 Direct Entry,	
Subcatchment s20-3:	
Runoff = 2.37 cfs @ 12.37 hrs, Volume= 0.318 af,	Depth= 0.55"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, Type III 24-hr 1-yr Rainfall=2.70"	dt= 0.01 hrs
Area (ac) CN Description	
6.886 70	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
22.0 Direct Entry,	
Subcatchment s21-1:	
Runoff = 19.54 cfs @ 12.29 hrs, Volume= 2.550 af,	Depth= 0.48"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, Type III 24-hr 1-yr Rainfall=2.70"	dt= 0.01 hrs
Area (ac) CN Description	
63.942 68	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
16.6 <b>Direct Entry</b> ,	

Proposed Conditions_10454-01Type III 24-hr 1-yr Rainfall=2.70"Prepared by The Chazen CompaniesPage 17HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems4/10/2006 3:14:47 PM
Subcatchment s21-1(OW):
Runoff = 39.66 cfs @ 12.00 hrs, Volume= 2.753 af, Depth= 2.70"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 1-yr Rainfall=2.70"
Area (ac) CN Description 12.235 100
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
0.0 Direct Entry,
Subcatchment s21-2:
Runoff = 6.24 cfs @ 12.52 hrs, Volume= 0.967 af, Depth= 0.55"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 1-yr Rainfall=2.70"
Area (ac) CN Description
20.941 70
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
31.4 Direct Entry,
Subcatchment s21-3:
Runoff = 5.48 cfs @ 12.17 hrs, Volume= 0.517 af, Depth= 0.72"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 1-yr Rainfall=2.70"
Area (ac) CN Description
8.567 74
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
11.2 Direct Entry,
Subcatchment s21-4:
Runoff = 0.97 cfs @ 12.25 hrs, Volume= 0.125 af, Depth= 0.44"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 1-yr Rainfall=2.70"

Proposed Conditions_10454-01 Type III 24-hr 1-yr Rainfall=2.70"
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Area (ac) CN Description
3.392 67
Tc Length Slope Velocity Capacity Description
(min) (feet) (ft/ft) (ft/sec) (cfs)
13.8Direct Entry,
Subcatchment s21-4(IC): s21-4 Impervious Cover
Runoff = 5.01 cfs @ 12.02 hrs, Volume= 0.338 af, Depth= 2.47"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 1-yr Rainfall=2.70"
Area (ac) CN Description
1.643 98
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
1.6Direct Entry,
Subcatchment s21-4(OW): s21-4 Open Water
Runoff = 0.38 cfs @ 12.00 hrs, Volume= 0.026 af, Depth= 2.70"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 1-yr Rainfall=2.70"
Area (ac) CN Description
0.117 100
Subcatchment s21-5:
Runoff = 1.54 cfs @ 12.21 hrs, Volume= 0.154 af, Depth= 0.77"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 1-yr Rainfall=2.70"
Area (ac) CN Description
2.398 75
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
13.9 Direct Entry,

Proposed Conditions_10454-01Type III 24-hr 1-yr Rainfall=2.70"Prepared by The Chazen CompaniesPage 19HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems4/10/2006 3:14:48 PM
Subcatchment s21-6:
Runoff = 3.46 cfs @ 12.27 hrs, Volume= 0.373 af, Depth= 0.82"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 1-yr Rainfall=2.70"
Area (ac) CN Description
5.463 76
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
17.5   Direct Entry,
Subcatchment s21-6(IC): s21-6 Impervious Cover
Runoff = 1.97 cfs @ 12.02 hrs, Volume= 0.132 af, Depth= 2.47"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 1-yr Rainfall=2.70"
Area (ac) CN Description
0.643 98
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
1.5Direct Entry,
Subcatchment s21-6(OW): s21-6 Open Water
Runoff = 0.25 cfs @ 12.00 hrs, Volume= 0.017 af, Depth= 2.70"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 1-yr Rainfall=2.70"
Area (ac) CN Description
0.076 100
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
0.0 Direct Entry,
Subcatchment s21-7:
Runoff = 0.82 cfs @ 12.27 hrs, Volume= 0.126 af, Depth= 0.34"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 1-yr Rainfall=2.70"

Proposed Conditions_10454-01	Type III 24-hr 1-yr Rainfall=2.70"	
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Area (ac) CN Description 4.375 64		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
13.1 Direct Entry,		
Subcatchment s21-7(IC): s21-7 Impervious Cover		
Runoff = 11.50 cfs @ 12.04 hrs, Volume= 0.801 af	, Depth= 2.47"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, Type III 24-hr 1-yr Rainfall=2.70"	dt= 0.01 hrs	
Area (ac) CN Description		
3.890 98		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
2.5 <b>Direct Entry</b> ,		
Subsetshment 21 7(0)4/v 221 7 On		
Subcatchment s21-7(OW): s21-7 Op	en water	
Runoff = 0.29 cfs @ 12.00 hrs, Volume= 0.020 af	, Depth= 2.70"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, Type III 24-hr 1-yr Rainfall=2.70"	dt= 0.01 hrs	
Area (ac) CN Description		
0.090 100		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
0.0 Direct Entry,		
Subcatchment s22-1:		
Runoff = 8.70 cfs @ 12.23 hrs, Volume= 0.947 af	, Depth= 0.64"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, Type III 24-hr 1-yr Rainfall=2.70"	dt= 0.01 hrs	
Area (ac) CN Description		
17.878 72		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
(min) (feet) (ft/ft) (ft/sec) (cfs) 14.7 Direct Entry,		

Proposed Conditions_10454-01Type III 24-hr 1-yr Rainfall=2.70"Prepared by The Chazen CompaniesPage 21HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems4/10/2006 3:14:48 PM
Subcatchment s22-1(OW): s22-1 Open Water
Runoff = 0.44 cfs @ 12.00 hrs, Volume= 0.031 af, Depth= 2.70"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 1-yr Rainfall=2.70"
Area (ac) CN Description 0.136 100
Subcatchment s22-2:
Runoff = 12.04 cfs @ 12.42 hrs, Volume= 1.788 af, Depth= 0.48"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 1-yr Rainfall=2.70"
Area (ac) CN Description
44.848 68
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
24.0 Direct Entry,
Subcatchment s23-1:
Runoff = 9.49 cfs @ 12.62 hrs, Volume= 1.543 af, Depth= 0.64"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 1-yr Rainfall=2.70"
Area (ac) CN Description
29.123 72
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
38.5 Direct Entry,
Subcatchment s23-2:
Runoff = 9.06 cfs @ 12.07 hrs, Volume= 0.633 af, Depth= 0.87"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 1-yr Rainfall=2.70"
Area (ac) CN Description
8.741 77

Proposed Conditions_10454-01 Prepared by The Chazen Companies	Type III 24-hr 1-yr Rainfall=2.70" Page 22
HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer System	ns 4/10/2006 3:14:48 PM
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
4.0 Direct Entry,	
Subcatchment s23-2(IC): s23-2 Imperv	ious Cover
Runoff = 20.12 cfs @ 12.06 hrs, Volume= 1.479 af,	Depth= 2.47"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, Type III 24-hr 1-yr Rainfall=2.70"	dt= 0.01 hrs
Area (ac) CN Description	
7.185 98	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
4.0 Direct Entry,	
Subcatchment s23-2(OW): s23-2 Op	en Water
Runoff = 0.54 cfs @ 12.00 hrs, Volume= 0.038 af,	Depth= 2.70"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, Type III 24-hr 1-yr Rainfall=2.70"	dt= 0.01 hrs
Area (ac) CN Description	
0.168 100	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
0.0 Direct Entry,	
Subcatchment s24-0:	
Runoff = 8.52 cfs @ 12.52 hrs, Volume= 1.308 af,	Depth= 0.55"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, Type III 24-hr 1-yr Rainfall=2.70"	dt= 0.01 hrs
Area (ac) CN Description	
28.325 70	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
30.7 Direct Entry,	

Proposed Conditions_10454-01Type III 24-hr 1-yrRainfall=2.70"Prepared by The Chazen CompaniesPage 23HydroCAD® 7.00s/n 000927© 1986-2003 Applied Microcomputer Systems4/10/20063:14:48 PM
Subcatchment s25-0:
Runoff = 3.03 cfs @ 12.37 hrs, Volume= 0.462 af, Depth= 0.41"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 1-yr Rainfall=2.70"
Area (ac) CN Description 13.562 66
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
20.2 Direct Entry,
Reach 25R:
Overland Flow Reach
Inflow Area =       15.520 ac, Inflow Depth =       0.23" for 1-yr event         Inflow =       0.45 cfs @       14.75 hrs, Volume=       0.299 af         Outflow =       0.45 cfs @       14.92 hrs, Volume=       0.299 af, Atten= 0%, Lag= 9.9 min
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 0.8 fps, Min. Travel Time= 12.3 min Avg. Velocity = 0.4 fps, Avg. Travel Time= 23.8 min
Peak Depth= 0.06' @ 14.92 hrs Capacity at bank full= 175.17 cfs Inlet Invert= 560.00', Outlet Invert= 512.00' 50.00' x 1.00' deep Parabolic Channel, n= 0.060 Length= 620.0' Slope= 0.0774 '/'
Reach r03-1:
Overland Flow Reach Requires more survey
Inflow Area =       11.485 ac, Inflow Depth =       0.48" for 1-yr event         Inflow =       2.38 cfs @       12.70 hrs, Volume=       0.458 af         Outflow =       2.35 cfs @       12.76 hrs, Volume=       0.458 af, Atten= 1%, Lag= 3.7 min
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 3.0 fps, Min. Travel Time= 4.3 min Avg. Velocity = 1.4 fps, Avg. Travel Time= 9.2 min
Peak Depth= 0.27' @ 12.76 hrs Capacity at bank full= 92.14 cfs Inlet Invert= 845.00', Outlet Invert= 728.00' 10.00' x 1.50' deep Parabolic Channel, n= 0.060 Length= 785.0' Slope= 0.1490 '/'

Proposed Conditions\_10454-01

## Reach r04-1:

Channel

Inflow Area =       26.658 ac, Inflow Depth =       0.66" for 1-yr event         Inflow =       5.36 cfs @       12.61 hrs, Volume=       1.473 af         Outflow =       5.35 cfs @       12.63 hrs, Volume=       1.473 af, Atten= 0%, Lag= 0.8 min
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 4.4 fps, Min. Travel Time= 1.3 min Avg. Velocity = 1.6 fps, Avg. Travel Time= 3.5 min
Peak Depth= 0.45' @ 12.63 hrs Capacity at bank full= 530.15 cfs Inlet Invert= 685.50', Outlet Invert= 632.00' 12.00' x 4.00' deep Parabolic Channel, n= 0.060 Length= 330.0' Slope= 0.1621 '/'
Reach r08-1a:
Man Made Ditch Inverts of pipe to be surveyed
Inflow Area =       93.367 ac, Inflow Depth =       0.38" for 1-yr event         Inflow =       10.87 cfs @       13.01 hrs, Volume=       2.946 af         Outflow =       10.86 cfs @       13.02 hrs, Volume=       2.946 af, Atten= 0%, Lag= 0.4 min
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 6.6 fps, Min. Travel Time= 0.6 min Avg. Velocity = 3.5 fps, Avg. Travel Time= 1.1 min
Peak Depth= 0.39' @ 13.02 hrs Capacity at bank full= 81.88 cfs Inlet Invert= 607.00', Outlet Invert= 587.00' 10.00' x 1.00' deep Parabolic Channel, n= 0.027 Length= 226.0' Slope= 0.0885 '/'

## Reach r08-1b:

24" HDPE Inverts to be surveyed

Inflow Area = 93.367 ac, Inflow Depth = 0.38" for 1-yr event Inflow = 10.86 cfs @ 13.02 hrs, Volume= 2.946 af Outflow = 10.86 cfs @ 13.02 hrs, Volume= 2.946 af, Atten= 0%, Lag= 0.2 min Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 17.3 fps, Min. Travel Time= 0.3 min Avg. Velocity = 9.5 fps, Avg. Travel Time= 0.5 min

#### Proposed Conditions\_10454-01 *Typ* Prepared by The Chazen Companies HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems

Peak Depth= 0.51' @ 13.02 hrs Capacity at bank full= 77.17 cfs Inlet Invert= 587.00', Outlet Invert= 557.75' 24.0" Diameter Pipe n= 0.012 Length= 295.0' Slope= 0.0992 '/'

## Reach r08-1c:

Ditch Pipe inverts to be surveyed

Inflow Area =	93.367 ac, Inflow Depth = 0.38"	for 1-yr event
Inflow =	10.86 cfs @ 13.02 hrs, Volume=	2.946 af
Outflow =	10.85 cfs @ 13.04 hrs, Volume=	2.946 af, Atten= 0%, Lag= 1.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 6.3 fps, Min. Travel Time= 1.6 min Avg. Velocity = 3.3 fps, Avg. Travel Time= 3.0 min

Peak Depth= 0.40' @ 13.04 hrs Capacity at bank full= 76.65 cfs Inlet Invert= 557.75', Outlet Invert= 512.00' 10.00' x 1.00' deep Parabolic Channel, n= 0.027 Length= 590.0' Slope= 0.0775 '/

## Reach r08-1d: Amenia Creek/Cascade Brook

(Steam) parallel to Route 22 Base Flow estimated from field observation (see field note 21)

Inflow Area = 114.957 ac, Inflow Depth = 17.04" for 1-yr event Inflow = 51.60 cfs @ 13.04 hrs, Volume= 163.225 af, Incl. 40.00 cfs Base Flow Outflow = 51.53 cfs @ 13.09 hrs, Volume= 162.980 af, Atten= 0%, Lag= 3.3 min Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 3.2 fps, Min. Travel Time= 4.1 min Avg. Velocity = 3.1 fps, Avg. Travel Time= 4.4 min

Peak Depth= 2.83' @ 13.09 hrs Capacity at bank full= 104.49 cfs Inlet Invert= 512.00', Outlet Invert= 504.00' 10.00' x 4.00' deep Parabolic Channel, n= 0.060 Length= 805.0' Slope= 0.0099 '/'

## Reach r13-1:

 Inflow Area =
 2.176 ac, Inflow Depth =
 0.82" for 1-yr event

 Inflow =
 1.43 cfs @
 12.24 hrs, Volume=
 0.148 af

 Outflow =
 1.40 cfs @
 12.27 hrs, Volume=
 0.148 af, Atten= 2%, Lag= 2.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 6.0 fps, Min. Travel Time= 2.5 min Avg. Velocity = 2.5 fps, Avg. Travel Time= 6.1 min

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Peak Depth= 0.29' @ 12.27 hrs Capacity at bank full= 17.79 cfs Inlet Invert= 546.00', Outlet Invert= 524.00' 18.0" Diameter Pipe n= 0.012 Length= 900.0' Slope= 0.0244 '/'

## Reach r14-3a:

30" HDPE Under Main Entrance Road

Inflow Are Inflow Outflow	a = = =	1.53 cfs @	nflow Depth = 0.38" 12.22 hrs, Volume= 12.24 hrs, Volume=	0.203 af	Atten= 0%, Lag= 0.8 min		
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs							

Max. Velocity= 7.2 fps, Min. Travel Time= 1.0 min Avg. Velocity = 3.5 fps, Avg. Travel Time= 2.1 min

Peak Depth= 0.22' @ 12.24 hrs Capacity at bank full= 94.91 cfs Inlet Invert= 526.00', Outlet Invert= 505.70' 30.0" Diameter Pipe n= 0.012 Length= 445.0' Slope= 0.0456 '/'

## Reach r14-3b:

Grass lined channel

Inflow Area = Inflow = Outflow =	6.422 ac, Inflow Depth = 0.38" for 1-yr event 1.55 cfs @ 12.20 hrs, Volume= 0.203 af 1.53 cfs @ 12.22 hrs, Volume= 0.203 af, Atten= 1%, Lag= 1.6 min	
Max. Velocity= 3.4	tor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs 4 fps, Min. Travel Time= 1.8 min 6 fps, Avg. Travel Time= 3.7 min	
Peak Depth= 0.24 Capacity at bank f Inlet Invert= 542.0		

10.00' x 3.00' deep Parabolic Channel, n= 0.027 Length= 360.0' Slope= 0.0444 '/'

## Reach r17-1:

Inflow Are	a =	76.086 ac, I	nflow Depth	n = 0.41"	for 1-yr e	event		
Inflow	=	7.69 cfs @	13.51 hrs,	Volume=	2.5	591 af		
Outflow	=	7.66 cfs @	13.62 hrs,	Volume=	2.5	591 af,	Atten= 0%,	Lag= 6.5 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 4.4 fps, Min. Travel Time= 5.3 min Avg. Velocity = 2.2 fps, Avg. Travel Time= 10.8 min

Prepared by T	nditions_10454-01         Type III 24-hr 1-yr         Rainfall=2.70"           ne Chazen Companies         Page 27           s/n 000927         © 1986-2003 Applied Microcomputer Systems         4/10/2006         3:14:48 PM
Inlet Invert= 646	46' @ 13.62 hrs < full= 181.28 cfs .00', Outlet Invert= 524.00' deep Parabolic Channel, n= 0.045 Length= 1,390.0' Slope= 0.0878 '/'
	Reach r18-2:
Overland Flow F	Reach
Inflow = Outflow =	0.00 cfs @ 0.00 hrs, Volume= 0.000 af 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Atten= 0%, Lag= 0.0 min
Max. Velocity=	Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs 0.0 fps, Min. Travel Time= 0.0 min 0.0 fps, Avg. Travel Time= 0.0 min
Inlet Invert= 973	00' @ 0.00 hrs < full= 434.91 cfs .60', Outlet Invert= 630.00' deep Parabolic Channel, n= 0.060 Length= 720.0' Slope= 0.4772 '/'
	Reach r21-1a:
Man Made Ditch	
Inflow Area = Inflow = Outflow =	207.817 ac, Inflow Depth = 0.11"       for 1-yr event         1.25 cfs @ 15.12 hrs, Volume=       1.933 af         1.25 cfs @ 15.17 hrs, Volume=       1.930 af, Atten= 0%, Lag= 3.4 min
Max. Velocity=2	Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs 2.2 fps, Min. Travel Time= 4.9 min 1.5 fps, Avg. Travel Time= 7.3 min
Inlet Invert= 504	28' @ 15.17 hrs < full= 191.76 cfs .00', Outlet Invert= 494.00' deep Parabolic Channel, n= 0.027 Length= 648.0' Slope= 0.0154 '/'
	Reach r21-1b:
Overland Flow F	Reach
Inflow Area = Inflow = Outflow =	29.123 ac, Inflow Depth = 0.34" for 1-yr event 2.76 cfs @ 13.67 hrs, Volume= 0.828 af 2.73 cfs @ 13.70 hrs, Volume= 0.828 af, Atten= 1%, Lag= 1.9 min
Routing by Dyn-	Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 1.8 fps, Min. Travel Time= 1.5 min Avg. Velocity = 1.0 fps, Avg. Travel Time= 2.6 min Proposed Conditions\_10454-01Type III 24-hPrepared by The Chazen CompaniesHydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems

Peak Depth= 0.13' @ 13.70 hrs Capacity at bank full= 227.81 cfs Inlet Invert= 506.70', Outlet Invert= 485.75' 50.00' x 1.00' deep Parabolic Channel, n= 0.060 Length= 160.0' Slope= 0.1309 '/'

### Reach r22-2:

**Overland Flow Reach** 

Inflow Area = Inflow = Outflow =	15.520 ac, Inflow Depth =0.23"for1-yr event0.45 cfs @14.68 hrs, Volume=0.299 af0.45 cfs @14.75 hrs, Volume=0.299 af, Atten= 0%, Lag= 4.2 min
Max. Velocity= 1.	Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs .7 fps, Min. Travel Time= 6.3 min .9 fps, Avg. Travel Time= 11.2 min

#### Reach r25-0a:

Ditch Pipe inverts need to be surveyed

Inflow Area = Inflow = Outflow =	67.391 ac, Inflow Depth = 0.52" 7.21 cfs @ 12.52 hrs, Volume= 7.16 cfs @ 12.56 hrs, Volume=	2.934 af	n
Max. Velocity= 5	Stor-Ind method, Time Span= 0.00-48 .7 fps, Min. Travel Time= 3.2 min .4 fps, Avg. Travel Time= 7.5 min	8.00 hrs, dt= 0.01 hrs	
Peak Depth= 0.4 Capacity at bank			

Capacity at bank full= 205.50 cfs Inlet Invert= 570.00', Outlet Invert= 504.00' 10.00' x 2.00' deep Parabolic Channel, n= 0.027 Length= 1,090.0' Slope= 0.0606 '/'

## Reach r25-0b: Wetland Reach

Wetland Reach Has wetland vegetation within reach

Inflow Area =	9.435 ac, Inflow Depth = 0.36"	for 1-yr event
Inflow =	0.29 cfs @ 15.40 hrs, Volume=	0.285 af
Outflow =	0.28 cfs @ 15.62 hrs, Volume=	0.284 af, Atten= 0%, Lag= 13.3 min

= 0.52" for 1-vr event

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Type III 24-hr 1-yr Rainfall=2.70" Page 29 er Systems 4/10/2006 3:14:48 PM

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Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 0.6 fps, Min. Travel Time= 21.7 min Avg. Velocity = 0.3 fps, Avg. Travel Time= 38.1 min

Peak Depth= 0.16' @ 15.62 hrs Capacity at bank full= 156.51 cfs Inlet Invert= 504.00', Outlet Invert= 499.50' 20.00' x 3.00' deep Parabolic Channel, n= 0.045 Length= 750.0' Slope= 0.0060 '/'

## Reach r25-0c: Amenia Creek/Cascade Brook

(Steam) parallel to Route 22 Base Flow estimated from field observation (see field note 21)

Inflow Area =138.083 ac, Inflow Depth = 28.00"for1-yr eventInflow =92.62 cfs @13.07 hrs, Volume=322.138 af, Incl. 40.00 cfs Base FlowOutflow =92.48 cfs @13.15 hrs, Volume=321.455 af, Atten= 0%, Lag= 5.0 min								
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 2.7 fps, Min. Travel Time= 6.0 min Avg. Velocity = 2.6 fps, Avg. Travel Time= 6.2 min								
Peak Depth= 4.75' @ 13.15 hrs Capacity at bank full= 67.14 cfs Inlet Invert= 504.00', Outlet Invert= 500.00' 10.00' x 4.00' deep Parabolic Channel, n= 0.060 Length= 975.0' Slope= 0.0041 '/'								
Pond 8P:								
No field note. Water spills over cart path; no storage.								
Inflow Area =       41.049 ac, Inflow Depth =       0.56" for 1-yr event         Inflow =       6.97 cfs @       12.52 hrs, Volume=       1.922 af         Outflow =       6.97 cfs @       12.52 hrs, Volume=       1.922 af, Atten= 0%, Lag= 0.0 min         Primary =       6.97 cfs @       12.52 hrs, Volume=       1.922 af								
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 575.05' @ 12.52 hrs Flood Elev= 574.70' Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= (not calculated)								
# Routing     Invert     Outlet Devices       1     Primary     574.70'     177.0 deg Sharp-Crested Vee/Trap Weir     C= 2.46								

## Pond p02-2:

Proposed culvert under proposed road at intersection with 44.

Inflow Inflow Outfle Prima	ow =	1.52 cfs 1.52 cfs	c, Inflow Depth = 0.41" for 1-yr event @ 12.54 hrs, Volume= 0.265 af @ 12.54 hrs, Volume= 0.265 af, Atten= 0%, Lag= 0.0 min @ 12.54 hrs, Volume= 0.265 af			
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 640.51' @ 12.54 hrs Flood Elev= 645.00' Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= (not calculated)						
#	Routing	Invert	Outlet Devices			
1	Primary	640.00'	<b>24.0"</b> x <b>100.0' long Culvert</b> CPP, end-section conforming to fill, Ke= 0.500 Outlet Invert= 638.00' S= 0.0200 '/' n= 0.012 Cc= 0.900			

## Pond p02-3:

Simulates last DMH at bottom of small road, at intersection with 44. This culvert is only used to size the drain pipe under 44.

Inflov Inflov Outflo Prima	= wc	7.91 cfs 7.91 cfs	c, Inflow Depth = 1.41" for 1-yr event @ 12.03 hrs, Volume= 0.480 af @ 12.03 hrs, Volume= 0.480 af, Atten= 0%, Lag= 0.0 min @ 12.03 hrs, Volume= 0.480 af					
Peak Flood Plug- Cente	Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 636.45' @ 12.03 hrs Flood Elev= 645.00' Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= (not calculated)							
#	Routing	Invert	Outlet Devices					
1	Primary	635.00'	24.0" x 100.0' long Culvert CPP, projecting, no headwall, Ke= 0.900					

Primary	635.00	24.0 X 100.0 long C	uivent CPP, p	rojecting, n	o neadwall,	Ke= 0.900
		Outlet Invert= 634.00'	S= 0.0100 '/'	n= 0.012	Cc= 0.900	

Primary OutFlow Max=7.87 cfs @ 12.03 hrs HW=636.45' TW=551.96' (Dynamic Tailwater) -1=Culvert (Inlet Controls 7.87 cfs @ 3.2 fps)

## Pond p03-2:

Inflow Area =	4.738 ac, Inflow Depth = 1.36"	for 1-yr event
Inflow =	7.95 cfs @ 12.02 hrs, Volume=	0.537 af
Outflow =	0.34 cfs @ 14.98 hrs, Volume=	0.534 af, Atten= 96%, Lag= 177.6 min
Primary =	0.34 cfs @ 14.98 hrs, Volume=	0.534 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 774.00' Surf.Area= 2,315 sf Storage= 4,095 cf Peak Elev= 776.20' @ 14.98 hrs Surf.Area= 7,193 sf Storage= 17,170 cf (13,075 cf above start) Flood Elev= 779.00' Surf.Area= 9,991 sf Storage= 41,391 cf (37,296 cf above start) Plug-Flow detention time= 663.3 min calculated for 0.440 af (82% of inflow) Center-of-Mass det. time= 465.7 min (1,259.4 - 793.7)

#	Invert	Avail.St	orage Sto	orage Descrip	otion		
1	768.00'	51,	363 cf <b>Cu</b>	istom Stage	Data (Conic)	Listed below	
Elevat	ion eet)	Surf.Area (sq-ft)	-	c.Store c-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
768.	.00	67		0	0	67	
770.	.00	345		376	376	361	
772	.00	729		1,050	1,426	777	
772	.50	842		392	1,819	901	
774.	.00	2,315		2,277	4,095	2,388	
774.	.50	5,704		1,942	6,037	5,779	
776.	.00	6,996		9,509	15,546	7,138	
778.	.00	8,917		15,874	31,420	9,160	
780.	.00	11,064		19,942	51,363	11,421	
# F	Routing	Invert	Outlet Dev	vices			
1 F	Primary	774.00'	3.0" Vert.	Orifice/Grate	C= 0.600		

2 Primary 776.20' **6.0" Vert. Orifice/Grate X 2.00** C= 0.600

3 Primary 778.50' 4.0' long x 6.0' high Sharp-Crested Rectangular Weir 2 End Contraction(s)

Primary OutFlow Max=0.34 cfs @ 14.98 hrs HW=776.20' TW=720.52' (Dynamic Tailwater)

-1=Orifice/Grate (Orifice Controls 0.34 cfs @ 6.9 fps)

-2=Orifice/Grate (Orifice Controls 0.00 cfs @ 0.2 fps)

-3=Sharp-Crested Rectangular Weir (Controls 0.00 cfs)

#### Pond p04-1:

Storage, inverts and culvert length based on assumed grading, check when final grading becomes available

Inflow Area	a =	34.207 ac, Inflow Depth = 0.62" for 1-yr event	
Inflow	=	6.26 cfs @ 12.52 hrs, Volume= 1.774 af	
Outflow	=	6.26 cfs @ 12.53 hrs, Volume= 1.774 af, Atten= 0%, Lag= 0	).7 min
Primary	=	6.26 cfs @ 12.53 hrs, Volume= 1.774 af	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

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Peak Elev= 639.09' @ 12.53 hrs Surf.Area= 711 sf Storage= 474 cf Flood Elev= 648.00' Surf.Area= 15,680 sf Storage= 66,062 cf Plug-Flow detention time= 3.0 min calculated for 1.774 af (100% of inflow) Center-of-Mass det. time= 2.8 min (1,025.8 - 1,023.0)

#	Invert	Avail.St	torage	Storage Des	scription		
1	638.00'	66,	062 cf	Custom Sta	ige Data (Conic) L	isted below	
Eleva (f	tion eet)	Surf.Area (sq-ft)	(0	Inc.Store cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft <u>)</u>	
638	8.00	0		0	0	0	
640	0.00	1,300		867	867	1,306	
642	2.00	6,180		6,876	7,743	6,203	
644	4.00	7,270		13,435	21,178	7,438	
646	6.00	11,100		18,235	39,414	11,327	
648	8.00	15,680		26,648	66,062	15,980	
#	Routing	Invert		Devices			
1	Primary	638.00'			Culvert CPP, er 0' S= 0.0584 '/'		ming to fill, Ke= 0.500 ).900

Primary OutFlow Max=6.26 cfs @ 12.53 hrs HW=639.09' TW=575.05' (Dynamic Tailwater) ←1=Culvert (Inlet Controls 6.26 cfs @ 3.6 fps)

#### Pond p06-0:

Field Note #22 Geometry to be confirmed by survey.

Inflow Area	=	9.435 ac, Inflow Depth = 0.40"	for 1-yr event
Inflow =	=	1.40 cfs @ 12.36 hrs, Volume=	0.311 af
Outflow =	=	0.29 cfs @ 15.40 hrs, Volume=	0.285 af, Atten= 80%, Lag= 181.9 min
Primary =	=	0.29 cfs @ 15.40 hrs, Volume=	0.285 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 506.80' Surf.Area= 18,600 sf Storage= 42,160 cf Peak Elev= 507.10' @ 15.40 hrs Surf.Area= 19,941 sf Storage= 48,459 cf (6,299 cf above start) Flood Elev= 507.10' Surf.Area= 19,958 sf Storage= 48,537 cf (6,377 cf above start) Plug-Flow detention time= (not calculated) Center-of-Mass det. time= (not calculated)

#	Invert	Avail.Stora	ige Storage Des	cription		
1	500.00'	67,669	ocf Custom Stag	<b>ge Data (Conic)</b> Li	sted below	
Elevat (fe	tion eet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
500		0	0	0	0	
506		18,600	42,160	42,160	18,672	
508	.00	24,030	25,509	67,669	24,138	

Proposed Conditions_10454-01Type III 24-hr 1-yr Rainfall=2.70"Prepared by The Chazen CompaniesPage 33HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems4/10/2006 3:14:49 PM						
HydroCAD® 7.00	<u>s/n 000927 @</u>	9 1986-2003 Applied	Microcomputer Syste	ems 4/10/2006 3:14:49 PM		
# Routing	Invert C	Outlet Devices				
1 Primary			culvert CMP. proje	cting, no headwall, Ke= 0.900		
· · · · · · · · · · · · · · · · · · ·	C	Outlet Invert= 506.0	0' S= 0.0400 '/' n=	= 0.024 Cc= 0.900		
2 Primary	507.10' <b>1</b>	78.0 deg Sharp-Cr	ested Vee/Trap We	ir C= 2.46		
1=Culvert (In	let Controls C	cfs @ 15.40 hrs H\ ).29 cfs @ 1.5 fps) <b>5 Weir</b> (Controls 0		.16' (Dynamic Tailwater)		
		Ро	nd p07-1:			
Field Note # 29 Outlet geometry t	o be confirme	ed by survey.				
Starting Elev= 57 Peak Elev= 573.1	Inflow = $1.85 \text{ cfs} @ 12.00 \text{ hrs}$ , Volume= $1.144 \text{ af}$ Outflow = $0.46 \text{ cfs} @ 16.82 \text{ hrs}$ , Volume= $1.012 \text{ af}$ , Atten= 75%, Lag= 289.2 min Primary = $0.46 \text{ cfs} @ 16.82 \text{ hrs}$ , Volume= $1.012 \text{ af}$ Routing by Dyn-Stor-Ind method, Time Span= $0.00-48.00 \text{ hrs}$ , dt= $0.01 \text{ hrs}$ Starting Elev= $572.80'$ Surf.Area= $21,640 \text{ sf}$ Storage= $56,264 \text{ cf}$ Peak Elev= $573.14' @ 16.82 \text{ hrs}$ Surf.Area= $23,227 \text{ sf}$ Storage= $64,490 \text{ cf}$ ( $8,226 \text{ cf}$ above start) Flood Elev= $573.50'$ Surf.Area= $24,936 \text{ sf}$ Storage= $73,351 \text{ cf}$ ( $17,087 \text{ cf}$ above start)					
Center-of-Mass d	let. time= (no	t calculated)				
# Invert	Avail.Sto	rage Storage Des	cription			
1 565.00'	85,55		ge Data (Conic) Lis	ted below		
Elevation	Surf.Area	Inc.Store	Cum.Store	Wet.Area		
(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)	(sq-ft)		
565.00	0	0	0	0		
572.80	21,640	56,264	56,264	21,735		
574.00	27,290	29,293	85,557	27,424		
# Routing	Invert C	Outlet Devices				
1 Primary	572.80' <b>1</b>	8.0" x 20.0' long C		cting, no headwall, Ke= 0.900		
2 Primary			0' S= 0.0400 '/' n= <b>ested Vee/Trap We</b>			
		cfs @ 16.82 hrs HV	N=573.14' TW=570	0.22' (Dynamic Tailwater)		

L=Culvert (Inlet Controls 0.46 cfs @ 1.6 fps) 2=Sharp-Crested Vee/Trap Weir (Controls 0.00 cfs)

# Pond p08-2:

Inflow Area	a =	18.762 ac, Inflow Depth = 1.21	for 1-yr event
Inflow	=	24.36 cfs @ 12.04 hrs, Volume=	= 1.895 af
Outflow	=	0.47 cfs @ 19.22 hrs, Volume=	= 1.299 af, Atten= 98%, Lag= 430.9 min
Primary	=	0.47 cfs @ 19.22 hrs, Volume=	= 1.299 af

Proposed Conditions_10454-01	Type III 24-hr 1-y
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l 24-hr 1-yr Rainfall=2.70" Page 34 4/10/2006 3:14:49 PM

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 550.00' Surf.Area= 8,558 sf Storage= 24,834 cf Peak Elev= 554.09' @ 19.22 hrs Surf.Area= 18,385 sf Storage= 86,322 cf (61,488 cf above start) Flood Elev= 557.00' Surf.Area= 23,344 sf Storage= 147,597 cf (122,763 cf above start) Plug-Flow detention time= 1,517.7 min calculated for 0.729 af (38% of inflow) Center-of-Mass det. time= 885.9 min (1,681.3 - 795.4)

#		Avail.Storage	<u> </u>			
1	544.00'	170,918 c	Custom Sta	age Data (Conic) L	isted below	
Elevat	ion	Surf.Area	Inc.Store	Cum.Store	Wet.Area	
(fe	et)	(sq-ft)	(cubic-feet)	(cubic-feet)	(sq-ft)	
544.	.00	1,962	0	0	1,962	
546.	.00	3,155	5,070	5,070	3,207	
548.	.00	4,454	7,572	12,642	4,577	
548.	.50	4,796	2,312	14,954	4,940	
550	.00	8,558	9,880	24,834	8,726	
550.	.50	12,948	5,339	30,173	13,120	
552.	.00	15,129	21,037	51,209	15,390	
554.	.00	18,234	33,315	84,524	18,627	
556.	.00	21,565	39,752	124,277	22,105	
558.	.00	25,122	46,642	170,918	25,823	

# Rodding invert Outlet Devices	
# Routing Invert Outlet Devices	

1 Primary 550.00' **3.0" Vert. Orifice/Grate** C= 0.600

2 Primary 554.09' **12.0" Vert. Orifice/Grate X 2.00** C= 0.600

3 Primary 556.00' **11.0' long x 6.0' high Sharp-Crested Rectangular Weir** 2 End Contraction(s)

Primary OutFlow Max=0.47 cfs @ 19.22 hrs HW=554.09' TW=514.57' (Dynamic Tailwater) 1=Orifice/Grate (Orifice Controls 0.47 cfs @ 9.6 fps) 2=Orifice/Grate (Orifice Controls 0.00 cfs @ 0.1 fps) -3=Sharp-Crested Rectangular Weir (Controls 0.00 cfs)

#### Pond p08-3:

Inflow Area =	2.828 ac, Inflow Depth = 1.14"	for 1-yr event
Inflow =	3.50 cfs @ 12.01 hrs, Volume=	0.270 af
Outflow =	0.32 cfs @ 12.95 hrs, Volume=	0.269 af, Atten= 91%, Lag= 56.3 min
Primary =	0.32 cfs @ 12.95 hrs, Volume=	0.269 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 528.00' Surf.Area= 1,849 sf Storage= 2,615 cf Peak Elev= 530.00' @ 12.95 hrs Surf.Area= 3,341 sf Storage= 7,723 cf (5,108 cf above start) Flood Elev= 533.00' Surf.Area= 6,389 sf Storage= 22,602 cf (19,987 cf above start) Plug-Flow detention time= 386.6 min calculated for 0.209 af (77% of inflow) Center-of-Mass det. time= 202.4 min (982.8 - 780.3)

Type III	24-hr 1-yr Rainfall=2.70"
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#	Invert	Avail.S	torage Storage D	escription		
1	524.00'	28,	956 cf Custom S	tage Data (Conic)	Listed below	
Elevat (fe	ion eet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
524	.00	178	0	0	178	
526	.00	500	651	651	524	
526	.50	548	262	913	587	
528	.00	1,849	1,702	2,615	1,900	
530	.00	3,344	5,120	7,734	3,437	
532	.00	5,240	8,513	16,248	5,388	
534	.00	7,538	12,709	28,956	7,755	
# F	Routing	Invert	Outlet Devices	·		
1 F	Primary	528.00'	3.0" Vert. Orifice/	Grate C= 0.600		
2 F	Primary	530.00'	12.0" Vert. Orifice	<b>/Grate</b> C= 0.600		

Primary OutFlow Max=0.32 cfs @ 12.95 hrs HW=530.00' TW=514.82' (Dynamic Tailwater) -1=Orifice/Grate (Orifice Controls 0.32 cfs @ 6.6 fps) -2=Orifice/Grate (Controls 0.00 cfs)

### Pond p09-2:

Inflow Area =	21.180 ac, Inflow Depth = $0.69$ "	for 1-yr event
Inflow =	7.86 cfs @ 12.04 hrs, Volume=	1.221 af
Outflow =	0.34 cfs @ 21.33 hrs, Volume=	0.896 af, Atten= 96%, Lag= 557.5 min
Primary =	0.34 cfs @ 21.33 hrs, Volume=	0.896 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 586.00' Surf.Area= 10,285 sf Storage= 36,340 cf Peak Elev= 588.21' @ 21.33 hrs Surf.Area= 19,486 sf Storage= 74,506 cf (38,166 cf above start) Flood Elev= 593.00' Surf.Area= 27,610 sf Storage= 187,200 cf (150,860 cf above start) Plug-Flow detention time= 2,350.8 min calculated for 0.062 af (5% of inflow) Center-of-Mass det. time= 851.6 min (1,697.2 - 845.6)

#	Invert	Avail.Storag	je Storage Des	cription		
1	580.00'	214,790	cf Custom Sta	ge Data (Conic) Li	sted below	
Elevat (fe	ion eet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
580	.00	3,968	0	0	3,968	
582	.00	5,102	9,046	9,046	5,198	
584	.00	6,343	11,423	20,469	6,550	
584	.50	6,670	3,253	23,722	6,907	
586	.00	10,285	12,619	36,340	10,554	
586	.50	16,887	6,725	43,066	17,159	
588	.00	19,143	27,005	70,070	19,525	
590	.00	22,349	41,451	111,521	22,890	
592	.00	25,781	48,089	159,610	26,494	
594	.00	29,439	55,180	214,790	30,336	

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	ŧ	Routing	Invert	Outlet Devices
	1	Primary	586.00'	3.0" Vert. Orifice/Grate C= 0.600
2	2	Primary	588.21'	8.0" Vert. Orifice/Grate C= 0.600
	3	Primary	592.00'	2.0' long x 6.0' high Sharp-Crested Rectangular Weir 2 End Contraction(s)

Primary OutFlow Max=0.34 cfs @ 21.33 hrs HW=588.21' TW=573.12' (Dynamic Tailwater)

-1=Orifice/Grate (Orifice Controls 0.34 cfs @ 7.0 fps)

-2=Orifice/Grate (Orifice Controls 0.00 cfs @ 0.2 fps)

-3=Sharp-Crested Rectangular Weir (Controls 0.00 cfs)

#### Pond p10:

Field Note #25 Need to get full story on how this pond works

Inflow Area = Inflow = Outflow =	59.531 ac, Inflow Depth =0.10"for 1-yr event2.75 cfs @12.00 hrs, Volume=0.507 af0.00 cfs @0.00 hrs, Volume=0.000 af, Atten= 100%, Lag= 0.0 min
Starting Elev= 49 Peak Elev= 498.9 Plug-Flow detenti	Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs 98.40' Surf.Area= 36,110 sf Storage= 101,108 cf 96' @ 25.62 hrs Surf.Area= 38,325 sf Storage= 123,205 cf (22,097 cf above start) ion time= (not calculated) det. time= (not calculated)

# Inver	Avail.Storage	Storage De	escription	
1 490.00	581,029 cf	Custom St	tage Data (Conic)	isted below
Elevation	Surf.Area	Inc.Store	Cum.Store	Wet.Area
(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)	(sq-ft)
490.00	0	0	0	0
498.40	36,110	101,108	101,108	36,221
500.00	42,400	62,741	163,849	42,610
502.00	54,880	97,012	260,861	55,187
504.00	78,730	132,895	393,755	79,107
506.00	109,382	187,274	581,029	109,836

#### Pond p13-1:

No Field Note Natural depression.

Inflow Area	a =	12.222 ac, Inflow Depth = 1.54"	for 1-yr event	
Inflow	=	19.60 cfs @ 12.04 hrs, Volume=	1.563 af	
Outflow	=	16.50 cfs @ 12.08 hrs, Volume=	1.549 af,	Atten= 16%, Lag= 2.6 min
Primary	=	16.50 cfs @ 12.08 hrs, Volume=	1.549 af	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 524.00' Surf.Area= 5,894 sf Storage= 16,480 cf Peak Elev= 526.37' @ 12.08 hrs Surf.Area= 9,099 sf Storage= 34,496 cf (18,017 cf above start) Flood Elev= 527.00' Surf.Area= 10,067 sf Storage= 40,862 cf (24,383 cf above start) Plug-Flow detention time= 458.5 min calculated for 1.170 af (75% of inflow) Center-of-Mass det. time= 255.2 min (1,032.1 - 776.9)

#	Invert	Avail.S	torage S	Storage Description			
1	518.00'	50,	891 cf C	Custom Stage Data (Conic) Listed below			
-	ation (feet)	Surf.Area (sq-ft)		nc.Store bic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
5	18.00	1,331		0	0	1,331	
52	20.00	2,048		3,353	3,353	2,104	
52	22.00	2,912		4,935	8,288	3,037	
52	22.50	3,150		1,515	9,803	3,294	
52	24.00	5,894		6,676	16,480	6,061	
52	26.00	8,542		14,354	30,834	8,776	
52	28.00	11,592		20,057	50,891	11,908	
<u>#</u> 1	Routing Primary	Invert 524.00'		. Orifice/G	rate C= 0.600	Pootongular Wai	
Z	2 Primary 525.90' <b>15.0' long x 1.3' high Sharp-Crested Rectangular Weir</b> 2 End Contraction(s)						

Primary OutFlow Max=16.47 cfs @ 12.08 hrs HW=526.36' TW=498.74' (Dynamic Tailwater) -1=Orifice/Grate (Orifice Controls 0.35 cfs @ 7.2 fps)

-2=Sharp-Crested Rectangular Weir (Weir Controls 16.12 cfs @ 2.3 fps)

# Pond p14-1:

Field Note #26 Need to figure out how this pond works

Inflow Area = Inflow = Outflow = Primary =	44.49 cfs @ 0.00 cfs @	nflow Depth = 1.16" 12.08 hrs, Volume= 0.00 hrs, Volume= 0.00 hrs, Volume=	4.892 af	= 100%, Lag= 0.0 min
Starting Elev= 49	97.40' Surf.Are 95' @ 48.00 hrs tion time= (not o	calculated)	e= 54,760 cf	(213,101 cf above start)

#	Invert	Avail.Storage	Storage Description
1	490.00'	805,062 cf	Custom Stage Data (Conic) Listed below

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Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
490.00	0	0	0	
497.40	22,200	54,760	54,760	22,286
498.00	25,330	14,249	69,009	25,433
500.00	52,810	76,476	145,485	52,948
502.00	73,360	125,608	271,093	73,574
504.00	84,070	157,308	428,402	84,467
506.00	92,130	176,139	604,540	92,797
508.00	108,618	200,522	805,062	109,437

# Routing Invert Outlet Devices

1 Primary

500.00' **24.0" x 80.0' long Culvert** CPP, end-section conforming to fill, Ke= 0.500 Outlet Invert= 502.00' S= -0.0250 '/' n= 0.012 Cc= 0.900

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=497.40' TW=498.40' (Dynamic Tailwater) ←1=Culvert (Controls 0.00 cfs)

# Pond p14-2:

Inflow Area	a =	15.934 ac, Inflow Depth =	1.57" for 1-y	r event	
Inflow	=	26.57 cfs @ 12.04 hrs, Vol	ume= 2	2.083 af	
Outflow	=	22.08 cfs @ 12.09 hrs, Vol	ume= 2	2.056 af, Atten= 17%	, Lag= 2.9 min
Primary	=	22.08 cfs @ 12.09 hrs, Vol	ume= 2	2.056 af	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 532.00' Surf.Area= 7,681 sf Storage= 23,903 cf Peak Elev= 534.19' @ 12.09 hrs Surf.Area= 11,015 sf Storage= 44,574 cf (20,671 cf above start) Flood Elev= 535.00' Surf.Area= 12,390 sf Storage= 54,538 cf (30,635 cf above start) Plug-Flow detention time= 429.5 min calculated for 1.507 af (72% of inflow) Center-of-Mass det. time= 221.7 min (998.7 - 777.0)

#	Invert	Avail.St	torage Storage D	escription			
1	526.00'	66,	889 cf Custom S	Custom Stage Data (Conic) Listed below			
-	ation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)		
52	26.00	2,239	0	0	2,239		
52	28.00	3,156	5,369	5,369	3,227		
53	30.00	4,207	7,338	12,707	4,362		
53	80.50	4,491	2,174	14,881	4,669		
53	32.00	7,681	9,023	23,903	7,885		
53	84.00	10,686	18,285	42,188	10,966		
53	36.00	14,093	24,701	66,889	14,463		
	Routing	Invert	Outlet Devices				
1 2	Primary Primary	532.00' 533.60'	14.0' long x 1.5' h	<ul> <li><b>b.0" Vert. Orifice/Grate</b> C= 0.600</li> <li><b>4.0' long x 1.5' high Sharp-Crested Rectangular Weir</b></li> <li>End Contraction(s)</li> </ul>			

Primary OutFlow Max=22.06 cfs @ 12.09 hrs HW=534.19' TW=498.77' (Dynamic Tailwater) 1=Orifice/Grate (Orifice Controls 0.34 cfs @ 6.9 fps) 2=Sharp-Crested Rectangular Weir (Weir Controls 21.72 cfs @ 2.6 fps)

# Pond p16-1:

Field Note # 49 Large pond with man-made island. Geometry to be verified by survey. In particular, we are making big guesses about the outlets. Also need to find out about valves...

Inflow Area =	176.893 ac, I	nflow Depth = 0.49"	for 1-yr event
Inflow =	17.97 cfs @	12.00 hrs, Volume=	7.295 af
Outflow =	0.00 cfs @	0.00 hrs, Volume=	0.000 af, Atten= 100%, Lag= 0.0 min
Primary =	0.00 cfs @	0.00 hrs, Volume=	0.000 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 507.00' Surf.Area= 199,799 sf Storage= 878,320 cf Peak Elev= 508.72' @ 48.00 hrs Surf.Area= 225,401 sf Storage= 1,196,089 cf (317,769 cf above start) Flood Elev= 510.50' Surf.Area= 271,550 sf Storage= 1,623,217 cf (744,897 cf above start) Plug-Flow detention time= (not calculated) Center-of-Mass det. time= (not calculated)

#	Invert	Avail.S	torage Storag	e Des	cription		
1	500.00'	2,062,	087 cf Custo	m Stag	ge Data (Conic) L	isted below	
Eleva	ation	Surf.Area	Inc.Sto	aro	Cum.Store	Wet.Area	
	feet)	(sq-ft)			(cubic-feet)	(sq-ft)	
	0.00	0		0	0	<u>(34 10)</u>	
	3.00	140,344	140,3	•	140,344	140,358	
	9.20	232,500	,		1,284,206	232,994	
51	0.00	249,400	192,7	20	1,476,927	249,951	
51	2.00	338,000	585,1	60	2,062,087	338,634	
#	Routing	Invert	Outlet Devices	6			
1	Primary	509.00'	18.0" x 110.0	long (	Culvert CMP, pr	ojecting, no head	wall, Ke= 0.900
	-		Outlet Invert=	505.70	)' S= 0.0300 '/'	n= 0.024 Cc= 0	.900
2	Primary	500.00'			ssumed equaliza		e X 0.00
	CMP, projecting, no headwall, Ke= 0.900						
2	Drimori	E10 E0'			)' S= 0.0000 '/'		
3	Primary	510.50'	175.0 deg Sha	arp-Cre	ested Vee/Trap W	veir $x 2.00 \ C= 2$	.40

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=507.00' TW=505.10' (Dynamic Tailwater)

-2=assumed equalization pipe w/ valve (Controls 0.00 cfs)

-3=Sharp-Crested Vee/Trap Weir (Controls 0.00 cfs)

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# Pond p17-1:

Field Note #45 Golf pond Geometry to be confirmed by surveyed

Inflow Area =	112.240 ac, Inflow Depth = 0.41"	for 1-yr event
Inflow =	9.88 cfs @ 13.54 hrs, Volume=	3.821 af
Outflow =	9.76 cfs @ 13.67 hrs, Volume=	3.821 af, Atten= 1%, Lag= 8.3 min
Primary =	9.76 cfs @ 13.67 hrs, Volume=	3.821 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 523.80' Surf.Area= 7,290 sf Storage= 9,234 cf Peak Elev= 524.86' @ 13.67 hrs Surf.Area= 9,527 sf Storage= 19,121 cf (9,887 cf above start) Flood Elev= 524.30' Surf.Area= 8,074 sf Storage= 13,623 cf (4,389 cf above start) Plug-Flow detention time= 69.2 min calculated for 3.609 af (94% of inflow) Center-of-Mass det. time= 25.8 min (1,002.1 - 976.3)

#	Invert	Avail.St	torage	je Storage Description			
1	520.00'	30,	224 cf	Custom Sta	age Data (Conic) L	isted below	
Eleva (1	ation feet)	Surf.Area (sq-ft)		Inc.Store cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
-	0.00	0		0	0	0	
-	3.80	7,290		9,234	9,234	7,313	
-	4.00	7,300		1,459	10,693	7,374	
52	6.00	12,460		19,531	30,224	12,581	
#	Routing	Invert	Outlet	Devices			
1	Primary	523.80'		•	adth Broad-Crest	•	Neir
2 3	Primary Primary	524.30' 525.20'	Head (feet) 0.20 0.40 0.60 0.80 1.00 Coef. (English) 2.80 2.92 3.08 3.30 3.32 143.0 deg Sharp-Crested Vee/Trap Weir C= 2.47 178.0 deg x 60.0' long Sharp-Crested Vee/Trap Weir C= 2.46				

Primary OutFlow Max=9.76 cfs @ 13.67 hrs HW=524.86' TW=515.06' (Dynamic Tailwater) 1=Broad-Crested Rectangular Weir (Weir Controls 8.00 cfs @ 3.4 fps) 2 Share Created Vac Tran Weir (Weir Controls 4.76 cfa @ 4.9 fps)

-2=Sharp-Crested Vee/Trap Weir (Weir Controls 1.76 cfs @ 1.9 fps)

-3=Sharp-Crested Vee/Trap Weir (Controls 0.00 cfs)

# Pond p18-1:

Field Note #46 Golf pond Geometry to be confirmed by surveyed

Inflow Area =	131.862  ac,  Inflow Depth = 0.42"	for 1-yr event
Inflow =	10.93 cfs @ 13.65 hrs, Volume=	4.630 af
Outflow =	9.66 cfs @ 14.16 hrs, Volume=	4.626 af, Atten= 12%, Lag= 30.9 min
Primary =	9.66 cfs @ 14.16 hrs, Volume=	4.626 af

Proposed Conditions_10454-01	Type III 24-hr 1-yr F
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24-hr 1-yr Rainfall=2.70" Page 41 4/10/2006 3:14:49 PM

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 513.90' Surf.Area= 20,680 sf Storage= 26,884 cf Peak Elev= 515.14' @ 14.16 hrs Surf.Area= 25,028 sf Storage= 56,800 cf (29,916 cf above start) Flood Elev= 514.81' Surf.Area= 23,768 sf Storage= 48,709 cf (21,825 cf above start) Plug-Flow detention time= 181.9 min calculated for 4.009 af (87% of inflow) Center-of-Mass det. time= 74.9 min (1,057.2 - 982.2)

#	Invert	Avail.Storag	ge Storage Des	cription				
1	510.00'	148,288	cf Custom Sta	Custom Stage Data (Conic) Listed below				
Elevat (fe	ion et)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft <u>)</u>			
510	.00	0	0	0	0			
513	.90	20,680	26,884	26,884	20,704			
514.	.00	20,690	2,068	28,952	20,756			
516	.00	28,290	48,782	77,735	28,436			
518	.00	42,760	70,554	148,288	42,967			

#	Routing	Invert	Outlet Devices
1	Primary	513.90'	2.0' long x 0.5' breadth Broad-Crested Rectangular Weir
	-		Head (feet) 0.20 0.40 0.60 0.80 1.00
			Coef. (English) 2.80 2.92 3.08 3.30 3.32
2	Primary	514.81'	143.0 deg Sharp-Crested Vee/Trap Weir C= 2.47
3	Primary	515.32'	175.0 deg x 10.0' long Sharp-Crested Vee/Trap Weir C= 2.46

Primary OutFlow Max=9.66 cfs @ 14.16 hrs HW=515.14' TW=507.63' (Dynamic Tailwater) =Broad-Crested Rectangular Weir (Weir Controls 9.19 cfs @ 3.7 fps)

-2=Sharp-Crested Vee/Trap Weir (Weir Controls 0.47 cfs @ 1.4 fps)

-3=Sharp-Crested Vee/Trap Weir (Controls 0.00 cfs)

# Pond p19-0:

Wetland Geometry to be confirmed by survey Based off aerial topo, and assumed topo contour

Pond Unchanged from existing to proposed conditions

Inflow Area = Inflow = Outflow = Primary = Secondary =	15.520 ac, Inflow Depth =       0.23" for 1-yr event         1.03 cfs @       12.80 hrs, Volume=       0.301 af         0.45 cfs @       14.68 hrs, Volume=       0.299 af, Atten= 56%, Lag= 112.7         0.45 cfs @       14.68 hrs, Volume=       0.299 af         0.00 cfs @       0.00 hrs, Volume=       0.000 af	' min
Starting Elev= 972 Peak Elev= 972.0 Plug-Flow detention	or-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs 00' Surf.Area= 86,000 sf Storage= 57,333 cf '@ 14.68 hrs Surf.Area= 86,763 sf Storage= 60,440 cf (3,107 cf above st n time= (not calculated) t. time= (not calculated)	art)

#### **Proposed Conditions\_10454-01** Prepared by The Chazen Companies

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#	Invert		Avail.Storage Storage Description					
1 9	70.00'	282,	329 cf <b>Cust</b>	om S	Stage Data (Conic)	Listed below		
Elevation (feet)		Surf.Area (sq-ft)	Inc.Si (cubic-f	eet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)		
970.00		0	57	0	0 57 222			
972.00 974.00		86,000 141,270	,	333 996	57,333 282,329	86,006 141,327		
# Rou	ting	Invert	Outlet Device	es				
1 Sec 2 Prim	ondary nary	973.60' 972.00'	<b>35.0' long x</b> Head (feet)	<b>0.5'  </b> ).20	long Sharp-Crest breadth Broad-Cre 0.40 0.60 0.80 1 .80 2.92 3.08 3.3	ested Rectangula		

Primary OutFlow Max=0.45 cfs @ 14.68 hrs HW=972.03' TW=970.04' (Dynamic Tailwater) ←2=Broad-Crested Rectangular Weir (Weir Controls 0.45 cfs @ 0.5 fps)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=972.00' TW=973.60' (Dynamic Tailwater)

# Pond p20-1:

Field Note #50 Spring Fed Pond Geometry to be confirmed by surveyed

Inflow Area = Inflow = Outflow = Primary =		2.169 af
Starting Elev= 5 Peak Elev= 505	Stor-Ind method, Time Span= 0.00-48 05.10' Surf.Area= 89,370 sf Storag .39' @ 15.12 hrs Surf.Area= 89,373 ntion time= (not calculated)	

Center-of-Mass det. time= (not calculated)

#	Invert	Avail.Stora	ge Storage Desc	cription	
1	502.00'	615,682	cf Custom Stag	ge Data (Prismatic	Listed below
Elevati (fee		Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
502.	00	0	0	0	
505.	10	89,370	138,524	138,524	
506.	00	89,380	80,437	218,961	
508.	00	99,280	188,660	407,621	
510.	00	108,781	208,061	615,682	

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Type III 24-hr 1-yr Rainfall=2.70" Page 43 HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems 4/10/2006 3:14:49 PM

#	Routing	Invert	Outlet Devices
1	Primary	505.10'	<b>3.0' long x 1.5' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 Coef. (English) 2.62 2.64 2.64 2.68 2.75 2.86 2.92 3.07 3.07 3.03 3.28
2	Primary	506.20'	3.32 6.5' long x 1.5' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 Coef. (English) 2.62 2.64 2.64 2.68 2.75 2.86 2.92 3.07 3.07 3.03 3.28 3.32
3	Primary	506.00'	176.0 deg x 97.0' long Sharp-Crested Vee/Trap Weir C= 2.46

Primary OutFlow Max=1.25 cfs @ 15.12 hrs HW=505.39' TW=504.28' (Dynamic Tailwater)

-1=Broad-Crested Rectangular Weir (Weir Controls 1.25 cfs @ 1.4 fps)

-2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

-3=Sharp-Crested Vee/Trap Weir (Controls 0.00 cfs)

#### Pond p20-2:

Inflow Are	ea =	13.511 ac, Inflow Depth = 1.42"	for 1-yr event
Inflow	=	18.97 cfs @ 12.08 hrs, Volume=	1.599 af
Outflow	=	0.41 cfs @ 18.69 hrs, Volume=	1.117 af, Atten= 98%, Lag= 396.6 min
Primary	=	0.41 cfs @ 18.69 hrs, Volume=	1.117 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 552.00' Surf.Area= 10,535 sf Storage= 35,913 cf Peak Elev= 555.13' @ 18.69 hrs Surf.Area= 19,054 sf Storage= 87,687 cf (51,774 cf above start) Flood Elev= 559.00' Surf.Area= 25,653 sf Storage= 174,016 cf (138,102 cf above start) Plug-Flow detention time= 1,958.5 min calculated for 0.293 af (18% of inflow) Center-of-Mass det. time= 885.5 min (1,679.1 - 793.6)

#I	nvert	Avail.St	torage	Storage De	escription		
1 54	16.00'	199,	647 cf	Custom St	age Data (Conic)	Listed below	
		o ( )					
Elevation		Surf.Area		Inc.Store	Cum.Store	Wet.Area	
(feet)		(sq-ft)	(0	cubic-feet)	(cubic-feet)	(sq-ft)	
546.00		3,714		0	0	3,714	
548.00		4,960		8,644	8,644	5,044	
550.00		6,308		11,241	19,885	6,493	
550.50		6,661		3,242	23,127	6,874	
552.00		10,535		12,786	35,913	10,779	
552.50		15,037		6,360	42,273	15,285	
554.00		17,268		24,209	66,483	17,616	
556.00		20,441		37,664	104,147	20,935	
558.00		23,840		44,237	148,384	24,494	
560.00		27,465		51,262	199,647	28,292	
# Rout	ing	Invert	Outlet	Devices			
1 Prima	ary	552.00'	3.0" Ve	ert. Orifice/G	Grate C= 0.600		
2 Prima	ary	558.20'	6.1' lor	ng x 6.2' hig	h Sharp-Crested	Rectangular Weir	2 End Contraction(s)

Primary OutFlow Max=0.41 cfs @ 18.69 hrs HW=555.13' TW=505.36' (Dynamic Tailwater) -1=Orifice/Grate (Orifice Controls 0.41 cfs @ 8.3 fps) -2=Sharp-Crested Rectangular Weir (Controls 0.00 cfs)

### Pond p21-1:

Inflow Area =	459.188 ac, Inflow Depth = 0.41"	for 1-yr event
Inflow =	46.84 cfs @ 12.48 hrs, Volume=	15.804 af
Outflow =	9.22 cfs @ 16.99 hrs, Volume=	14.715 af, Atten= 80%, Lag= 270.2 min
Primary =	9.22 cfs @ 16.99 hrs, Volume=	14.715 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 482.43' @ 16.99 hrs Surf.Area= 263,757 sf Storage= 252,909 cf Plug-Flow detention time= 421.7 min calculated for 14.712 af (93% of inflow) Center-of-Mass det. time= 323.9 min (1,420.8 - 1,096.9)

**1**=**Culvert** (Barrel Controls 9.22 cfs @ 2.9 fps)

#	Invert	Avail.Sto	<u> </u>			
1	480.40'	5,244,88	35 cf Custom Sta	<b>ge Data (Conic)</b> Li	sted below	
Eleva (f	ition eet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
	0.40 2.00	0 202,230	0 107,856	0 107,856	0 202,234	
	4.00 5.00	485,198 1,275,481	667,114 1,698,237	774,970 2,473,208	485,231 1,275,541	
	3.00	1,499,208	2,771,678	5,244,885	1,499,423	
#	Routing	Invert C	Dutlet Devices			
1	Primary	480.40' <b>3</b>	0.0" x 70.0' long C	ulvert CMP, proj	ecting, no headwal	l, Ke= 0.900

Outlet Invert= 480.40' S= 0.0000 '/' n= 0.024 Cc= 0.900 **Primary OutFlow** Max=9.22 cfs @ 16.99 hrs HW=482.43' TW=0.00' (Dynamic Tailwater)

#### Pond p21-4:

Inflow Area =	5.152 ac, Inflow Depth = $1.14$ "	for 1-yr event
Inflow =	5.49 cfs @ 12.02 hrs, Volume=	0.490 af
Outflow =	0.32 cfs @ 15.01 hrs, Volume=	0.482 af, Atten= 94%, Lag= 179.5 min
Primary =	0.32 cfs @ 15.01 hrs, Volume=	0.482 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 496.00' Surf.Area= 5,112 sf Storage= 14,306 cf Peak Elev= 497.92' @ 15.01 hrs Surf.Area= 7,372 sf Storage= 26,302 cf (11,996 cf above start) Flood Elev= 499.00' Surf.Area= 8,847 sf Storage= 35,622 cf (21,317 cf above start) Plug-Flow detention time= 1,197.3 min calculated for 0.154 af (31% of inflow) Center-of-Mass det. time= 485.1 min (1,278.8 - 793.7)

Type III	24-hr 1-yr Rainfall=2.70"
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# Invert	Avail.Storage	e Storage Des	scription		
1 490.00'	44,433 c	f Custom Sta	ge Data (Conic) Li	sted below	
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
490.00	1,146	0	0	1,146	
492.00	1,784	2,907	2,907	1,839	
494.00	2,530	4,292	7,199	2,654	
494.50	2,733	1,315	8,514	2,876	
496.00	5,112	5,791	14,306	5,278	
498.00	7,468	12,506	26,812	7,699	
500.00	10,226	17,622	44,433	10,536	
<ul><li># Routing</li><li>1 Primary</li><li>2 Primary</li></ul>	496.00' <b>3.0</b> "	et Devices Vert. Orifice/Gr ong x 2.0' high		ectangular Weir	2 End Contraction(s)

Primary OutFlow Max=0.32 cfs @ 15.01 hrs HW=497.92' TW=482.39' (Dynamic Tailwater) -1=Orifice/Grate (Orifice Controls 0.32 cfs @ 6.4 fps) -2=Sharp-Crested Rectangular Weir (Controls 0.00 cfs)

#### Pond p21-5:

Inflow Area	a =	2.398 ac, Inflow	Depth = 0.77"	for 1-yr event	
Inflow	=	1.54 cfs @ 12.2	1 hrs, Volume=	0.154 af	
Primary	=	1.54 cfs @ 12.2	1 hrs, Volume=	0.154 af,	Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

#### Pond p21-6:

Inflow Area	=	6.182 ac, Inflow Depth = 1.01"	for 1-yr event
Inflow =	=	4.14 cfs @ 12.25 hrs, Volume=	= 0.522 af
Outflow =	=	0.30 cfs @ 16.05 hrs, Volume=	= 0.517 af, Atten= 93%, Lag= 228.4 min
Primary =	=	0.30 cfs @ 16.05 hrs, Volume=	= 0.517 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 492.00' Surf.Area= 3,323 sf Storage= 4,847 cf Peak Elev= 493.72' @ 16.05 hrs Surf.Area= 11,011 sf Storage= 18,105 cf (13,259 cf above start) Flood Elev= 495.00' Surf.Area= 13,824 sf Storage= 34,456 cf (29,609 cf above start) Plug-Flow detention time= 796.8 min calculated for 0.406 af (78% of inflow) Center-of-Mass det. time= 549.7 min (1,390.8 - 841.1)

#	Invert	Avail.Storage	Storage Description
1	488.00'	48,245 cf	Custom Stage Data (Conic) Listed below

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Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
488.00	296	0	0	296
490.00	924	1,162	1,162	946
490.50	1,110	508	1,670	1,141
492.00	3,323	3,177	4,847	3,367
492.50	6,166	2,336	7,182	6,212
494.00	12,147	13,484	20,666	12,214
496.00	15,500	27,579	48,245	15,669

Invert Outlet Devices # Routing 1

Primary 492.00' **3.0" Vert. Orifice/Grate** C= 0.600

5.0' long x 2.0' high Sharp-Crested Rectangular Weir 2 End Contraction(s) 2 Primary 494.00'

Primary OutFlow Max=0.30 cfs @ 16.05 hrs HW=493.72' TW=482.43' (Dynamic Tailwater) -1=Orifice/Grate (Orifice Controls 0.30 cfs @ 6.1 fps)

-2=Sharp-Crested Rectangular Weir (Controls 0.00 cfs)

#### Pond p21-7:

Inflow Are	ea =	8.355  ac,  Inflow Depth = 1.36"	for 1-yr event
Inflow	=	11.78 cfs @ 12.04 hrs, Volume=	0.946 af
Outflow	=	0.47 cfs @ 15.56 hrs, Volume=	0.936 af, Atten= 96%, Lag= 211.4 min
Primary	=	0.47 cfs @ 15.56 hrs, Volume=	0.936 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 492.00' Surf.Area= 3,941 sf Storage= 8,984 cf Peak Elev= 496.05' @ 15.56 hrs Surf.Area= 8,772 sf Storage= 34,187 cf (25,203 cf above start) Flood Elev= 499.00' Surf.Area= 13,379 sf Storage= 67,369 cf (58,385 cf above start) Plug-Flow detention time= 908.1 min calculated for 0.730 af (77% of inflow) Center-of-Mass det. time= 629.7 min (1,408.3 - 778.7)

	vert Avail.S	Storage Storage D	Description		
1 486.	.00' 80	,712 cf Custom S	Stage Data (Conic)	Listed below	
Elevation (feet)	Surf.Area (sq-ft		Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
486.00	478	· · · · · · · · · · · · · · · · · · ·	0	478	
488.00	964	1,414	1,414	999	
490.00	1,60 <i>1</i>	2,538	3,952	1,684	
490.50	1,782	2 845	4,797	1,879	
492.00	3,941	4,187	8,984	4,056	
494.00	6,120	) 9,981	18,965	6,292	
496.00	8,702	2 14,746	33,712	8,944	
498.00	11,686	5 20,315	54,027	12,012	
500.00	15,072	26,685	80,712	15,495	
# Routing 1 Primar 2 Primar	y 492.00' y 496.05'		Grate X 2.00 C= 0		
3 Primar	y 498.00'	5.0' long x 6.0' hi	gh Sharp-Crested	Rectangular Weir	2 End Contraction(s)

Primary OutFlow Max=0.47 cfs @ 15.56 hrs HW=496.05' TW=482.41' (Dynamic Tailwater) 1=Orifice/Grate (Orifice Controls 0.47 cfs @ 9.5 fps) 2=Orifice/Grate (Controls 0.00 cfs) -3=Sharp-Crested Rectangular Weir (Controls 0.00 cfs)

# Pond p22-1:

Field Note #54 Golf Pond Geometry to be confirmed by survey

Pond unchanged from existing to proposed conditions

Inflow Area	a =	78.382 ac, Inflow Depth = 0.47"	for 1-yr event
Inflow	=	19.01 cfs @ 12.36 hrs, Volume=	3.066 af
Outflow	=	16.10 cfs @ 12.55 hrs, Volume=	2.768 af, Atten= 15%, Lag= 11.0 min
Primary	=	16.10 cfs @ 12.55 hrs, Volume=	2.768 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 498.10' Surf.Area= 6,520 sf Storage= 10,106 cf Peak Elev= 500.89' @ 12.55 hrs Surf.Area= 9,786 sf Storage= 33,127 cf (23,021 cf above start) Plug-Flow detention time= 177.8 min calculated for 2.536 af (83% of inflow) Center-of-Mass det. time= 73.1 min (1,001.1 - 928.0)

ert Avail.S	torage Storage De	escription	
0' 143,	770 cf Custom S	tage Data (Prism	atic) Listed below
Surf.Area		Cum.Store	
(sq-ft)	(cubic-feet)	(cubic-feet)	
0	0	0	
6,520	10,106	10,106	
8,390	14,164	24,270	
11,530	19,920	44,190	
14,530	26,060	70,250	
18,340	32,870	103,120	
22,310	40,650	143,770	
Invert	Outlet Devices		
499.75'	18.0" x 21.0' long	Culvert CMP, p	projecting, no headwall, Ke= 0.900
	Outlet Invert= 499.	.75' S= 0.0000 '/	n= 0.024 Cc= 0.900
500.50'	1.0' long x 15.0' b	readth Broad-Cr	ested Rectangular Weir
	Head (feet) 0.20	0.40 0.60 0.80 <sup>·</sup>	1.00 1.20 1.40 1.60
	Coef. (English) 2.0	68 2.70 2.70 2.6	64 2.63 2.64 2.64 2.63
500.50'	20.0' long x 13.5'	breadth Broad-C	rested Rectangular Weir
	Head (feet) 0.20	0.40 0.60 0.80 <sup>·</sup>	1.00 1.20 1.40 1.60
	( )		
	0' 143, Surf.Area (sq-ft) 0 6,520 8,390 11,530 14,530 14,530 18,340 22,310 Invert 499.75' 500.50'	0' 143,770 cf <b>Custom S</b> Surf.Area Inc.Store (sq-ft) (cubic-feet) 0 0 6,520 10,106 8,390 14,164 11,530 19,920 14,530 26,060 18,340 32,870 22,310 40,650 Invert Outlet Devices 499.75' <b>18.0" x 21.0' long</b> Outlet Invert= 499 500.50' <b>1.0' long x 15.0' b</b> Head (feet) 0.20 Coef. (English) 2.1 500.50' <b>20.0' long x 13.5'</b> Head (feet) 0.20	O'         143,770 cf         Custom Stage Data (Prism           Surf.Area         Inc.Store         Cum.Store           (sq-ft)         (cubic-feet)         (cubic-feet)           0         0         0           6,520         10,106         10,106           8,390         14,164         24,270           11,530         19,920         44,190           14,530         26,060         70,250           18,340         32,870         103,120           22,310         40,650         143,770           Invert         Outlet Devices         Outlet Invert= 499.75'         S= 0.0000 '/           500.50'         1.0' long x 15.0' breadth Broad-Cr         Head (feet)         0.20         0.40         0.60         0.80

Primary OutFlow Max=16.09 cfs @ 12.55 hrs HW=500.89' TW=482.09' (Dynamic Tailwater) -1=Culvert (Barrel Controls 2.54 cfs @ 2.4 fps) -2=Broad-Crested Rectangular Weir (Weir Controls 0.66 cfs @ 1.7 fps) -3=Broad-Crested Rectangular Weir (Weir Controls 12.90 cfs @ 1.7 fps)

# Pond p23-1:

Inflow Area =	29.123 ac, Inflow Depth = 0.64"	for 1-yr event
Inflow =	9.49 cfs @ 12.62 hrs, Volume=	1.543 af
Outflow =	2.76 cfs @ 13.67 hrs, Volume=	0.828 af, Atten= 71%, Lag= 63.2 min
Primary =	2.76 cfs @ 13.67 hrs, Volume=	0.828 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 507.73' @ 13.67 hrs Surf.Area= 13,906 sf Storage= 31,503 cf Plug-Flow detention time= 265.1 min calculated for 0.828 af (54% of inflow) Center-of-Mass det. time= 128.4 min (1,040.0 - 911.6)

#	Invert	Avail.Stora	ge Storage Des	cription	
1	503.50'	68,915	cf Custom Sta	<b>ge Data (Conic)</b> Lis	sted below
Elevat (fe	tion eet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft <u>)</u>
503	.50	0	0	0	0
504	.00	2,390	398	398	2,390
506	.00	9,090	10,761	11,159	9,110
508	.00	14,660	23,529	34,688	14,732
510	.00	19,690	34,227	68,915	19,847
# F	Routing	Invert Ou	itlet Devices		

1 Primary 507.70' **178.0 deg x 178.0' long Sharp-Crested Vee/Trap Weir** C= 2.46

Primary OutFlow Max=2.76 cfs @ 13.67 hrs HW=507.73' TW=506.83' (Dynamic Tailwater) ←1=Sharp-Crested Vee/Trap Weir (Weir Controls 2.76 cfs @ 0.5 fps)

# Pond p23-2:

Inflow Are	ea =	16.094  ac,  Inflow Depth = 1.60"	for 1-yr event	
Inflow	=	29.46 cfs @ 12.06 hrs, Volume=	2.149 af	
Outflow	=	0.50 cfs @ 19.28 hrs, Volume=	1.403 af, Atten= 98%, Lag= 432.9 mi	n
Primary	=	0.50 cfs @ 19.28 hrs, Volume=	1.403 af	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 508.00' Surf.Area= 7,318 sf Storage= 15,927 cf Peak Elev= 512.54' @ 19.28 hrs Surf.Area= 19,905 sf Storage= 86,895 cf (70,968 cf above start) Flood Elev= 515.00' Surf.Area= 24,788 sf Storage= 141,986 cf (126,059 cf above start) Plug-Flow detention time= 1,312.2 min calculated for 1.037 af (48% of inflow) Center-of-Mass det. time= 894.7 min (1,682.4 - 787.7)

Proposed Conditions\_10454-01Type III 24Prepared by The Chazen CompaniesHydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems

Type III 24-hr 1-yr Rainfall=2.70" Page 49 4/10/2006 3:14:50 PM

# Invert	Avail.Storage	Storage Desc	cription			
1 502.00'	166,746 cf	Custom Stag	<b>ge Data (Conic)</b> Lis	sted below		
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)		
502.00	826	0	0	826		
504.00	1,667	2,444	2,444	1,702		
506.00	2,788	4,407	6,852	2,872		
506.50	3,112	1,474	8,326	3,210		
508.00	7,318	7,601	15,927	7,432		
508.50	12,618	4,924	20,851	12,735		
510.00	15,208	20,839	41,690	15,400		
512.00	18,859	34,002	75,692	19,166		
514.00	22,736	41,535	117,227	23,175		
516.00	26,840	49,519	166,746	27,428		
# Routing	Invert Outlet	Devices				
2 Primary 3 Primary Primary OutFlow -1=Orifice/Gra -2=Orifice/Gra	2 Primary 512.55' 12.0" Vert. Orifice/Grate X 2.00 C= 0.600					
	Pond zDP1: Design Point 1					
Field note #10. Culvert dimensio	ns to be confirmed	d by survey.				
Inflow Area = Inflow = Outflow = Primary =	26.658 ac, Inflo 5.36 cfs @ 12 5.36 cfs @ 12 5.36 cfs @ 12	.61 hrs, Volum .61 hrs, Volum	e= 1.473 a e= 1.473 a	af af, Atten= 0%, Lag= 0.0 min		
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 720.93' @ 12.61 hrs Surf.Area= 39 sf Storage= 25 cf Flood Elev= 727.00' Surf.Area= 1,105 sf Storage= 2,619 cf Plug-Flow detention time= 0.2 min calculated for 1.473 af (100% of inflow) Center-of-Mass det. time= 0.2 min (1,046.3 - 1,046.1)						

#	Invert	Avail.Storage	Storage Description
1	720.10'	3,706 cf	Custom Stage Data (Conic) Listed below

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Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
720.10	0	0	0	0
722.00	90	57	57	96
724.00	340	403	460	364
726.00	760	1,072	1,533	815
728.00	1,450	2,173	3,706	1,543

#	Routing	Invert	Outlet Devices
1	Primary	720.10'	<b>42.0" x 120.0' long Culvert</b> CMP, square edge headwall, Ke= 0.500
	-		Outlet Invert= 700.00' S= 0.1675 '/' n= 0.024 Cc= 0.900
2	Primary	727.00'	155.0 deg Sharp-Crested Vee/Trap Weir C= 2.47

Primary OutFlow Max=5.36 cfs @ 12.61 hrs HW=720.93' TW=685.95' (Dynamic Tailwater) -1=Culvert (Inlet Controls 5.36 cfs @ 3.1 fps)

-2=Sharp-Crested Vee/Trap Weir (Controls 0.00 cfs)

# Pond zDP2: Design Point 2

Field Note #15 Culvert dimensions to be confirmed by survey Overflow to ditch is currently discarded... We may have to model that area...

Inflow Area =	93.367 ac, Inflow Depth = 0.38"	for 1-yr event
Inflow =	10.87 cfs @ 13.01 hrs, Volume=	2.946 af
Outflow =	10.87 cfs @ 13.01 hrs, Volume=	2.946 af, Atten= 0%, Lag= 0.2 min
Discarded =	0.00 cfs @ 0.00 hrs, Volume=	0.000 af
Primary =	10.87 cfs @ 13.01 hrs, Volume=	2.946 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 621.13' @ 13.01 hrs Surf.Area= 151 sf Storage= 122 cf Flood Elev= 624.50' Surf.Area= 925 sf Storage= 1,728 cf Plug-Flow detention time= 0.2 min calculated for 2.946 af (100% of inflow) Center-of-Mass det. time= 0.2 min (962.9 - 962.7)

#	Invert	Avail.S	torage	Storage Description				
1	619.60'	7,	280 cf	Custom Stage Data (Conic) Listed below				
	ation (feet)	Surf.Area (sq-ft)		Inc.Store ubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)		
	19.60	0	X	0	0			
62	20.00	10		1	1	10		
62	22.00	260		214	215	269		
62	24.00	760		976	1,192	793		
62	26.00	1,420		2,146	3,338	1,492		
62	28.00	2,580		3,943	7,280	2,694		
#	Routing	Invert	Outlet E	Devices				
1	Primary	619.60'			<b>Culvert</b> RCP, er 0' S= 0.0773 '/'		ming to fill, Ke= 0.500 .900	
2	Discarded	624.50'	166.0 d	eg Sharp-Cr	ested Vee/Trap W	<b>Veir</b> C= 2.46		

**Discarded OutFlow** Max=0.00 cfs @ 0.00 hrs HW=619.60' (Free Discharge) **1**–2=Sharp-Crested Vee/Trap Weir (Controls 0.00 cfs)

Primary OutFlow Max=10.87 cfs @ 13.01 hrs HW=621.13' TW=607.39' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 10.87 cfs @ 4.2 fps)

# Pond zDP3: Design Point 3

Inflow Are	a =	228.471 ac, Inflow Depth = 17.08"	for	1-yr event	
Inflow	=	98.63 cfs @ 12.99 hrs, Volume=		325.132 af	
Primary	=	98.63 cfs @ 12.99 hrs, Volume=		325.132 af,	Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

#### Pond zDP4: Design Point 4

Inflow Area	a =	459.188 ac, Inflow Depth = 0.38	8" for 1-yr event
Inflow	=	9.22 cfs @ 16.99 hrs, Volume	e= 14.715 af
Primary	=	9.22 cfs @ 16.99 hrs, Volume	e= 14.715 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

#### Pond zDP5: Design Point 5

Inflow Area =	28.325 ac, Inflow Depth = 0.55"	for 1-yr event
Inflow =	8.52 cfs @ 12.52 hrs, Volume=	1.308 af
Primary =	8.52 cfs @ 12.52 hrs, Volume=	1.308 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

# Post-Development Conditions 2 year 24 hour Storm Event Model Computations

Proposed Conditions_10454-01 Prepared by The Chazen Companies	Type III 24-hr 2-yr Rainfall=3.40" Page 52
HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer System	
Subcatchment s01-0:	
Runoff = 4.77 cfs @ 12.66 hrs, Volume= 0.808 af,	Depth= 0.84"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, Type III 24-hr 2-yr Rainfall=3.40"	dt= 0.01 hrs
Area (ac) CN Description 11.485 68	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
42.8 Direct Entry,	
Subcatchment s02-1:	
Runoff = 22.37 cfs @ 12.95 hrs, Volume= 4.994 af,	Depth= 0.70"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, Type III 24-hr 2-yr Rainfall=3.40"	dt= 0.01 hrs
Area (ac) CN Description 85.591 65	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
61.3Direct Entry,	
Subcatchment s02-2:	
Runoff = 3.29 cfs @ 12.48 hrs, Volume= 0.484 af,	Depth= 0.75"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, Type III 24-hr 2-yr Rainfall=3.40"	dt= 0.01 hrs
Area (ac) CN Description	
7.776 66	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
29.4 Direct Entry,	
Subcatchment s02-3:	
Runoff = 11.27 cfs @ 12.03 hrs, Volume= 0.685 af,	Depth= 2.01"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, Type III 24-hr 2-yr Rainfall=3.40"	dt= 0.01 hrs

Proposed Conditions_10454-01	Type III 24-hr 2-yr Rainfall=3.40"
Prepared by The Chazen Companies HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer System	Page 53 ns 4/10/2006 3:15:04 PM
Area (ac) CN Description 4.088 86	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
1.6Direct Entry,	
Subcatchment s03-1:	
Runoff = 6.10 cfs @ 12.45 hrs, Volume= 0.823 af,	Depth= 0.95"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, Type III 24-hr 2-yr Rainfall=3.40"	dt= 0.01 hrs
Area (ac) CN Description	
10.435 70	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
28.8 Direct Entry,	
Subcatchment s03-2:	
Runoff = 4.67 cfs @ 12.03 hrs, Volume= 0.295 af,	Depth= 1.17"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, Type III 24-hr 2-yr Rainfall=3.40"	dt= 0.01 hrs
Area (ac) CN Description	
3.021 74	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
1.5 Direct Entry,	
Subcatchment s03-2(IC): s03-2 Imperv	ious Cover
Runoff = 6.45 cfs @ 12.02 hrs, Volume= 0.439 af,	Depth= 3.17"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, Type III 24-hr 2-yr Rainfall=3.40"	dt= 0.01 hrs
Area (ac) CN Description	
1.663 98	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
1.5 <b>Direct Entry</b> ,	

<b>Proposed Conditions_10454-01</b> Prepared by The Chazen Companies	Type III 24-hr 2-yr Rainfall=3.40" Page 54
HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer System	•
Subcatchment s03-2(OW): s03-2 Ope	en Water
Runoff = 0.22 cfs @ 12.00 hrs, Volume= 0.015 af,	Depth= 3.40"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, Type III 24-hr 2-yr Rainfall=3.40"	dt= 0.01 hrs
Area (ac) CN Description 0.054 100	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
0.0 Direct Entry,	
Subcatchment s04-1:	
Runoff = 6.57 cfs @ 12.10 hrs, Volume= 0.531 af,	Depth= 0.84"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, Type III 24-hr 2-yr Rainfall=3.40"	dt= 0.01 hrs
Area (ac) CN Description 7.549 68	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
6.1 Direct Entry,	
Subcatchment s05-1:	
Runoff = 2.23 cfs @ 12.27 hrs, Volume= 0.301 af,	Depth= 0.53"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, Type III 24-hr 2-yr Rainfall=3.40"	dt= 0.01 hrs
Area (ac) CN Description	
6.842 61	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
14.4 Direct Entry,	
Subcatchment s06-0:	
Runoff = 3.12 cfs @ 12.31 hrs, Volume= 0.427 af,	Depth= 0.57"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, Type III 24-hr 2-yr Rainfall=3.40"	dt= 0.01 hrs

Proposed Conditions_10454-01Type III 24-hr 2-yrRainfall=3.407Prepared by The Chazen CompaniesPage 55HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems4/10/2006 3:15:04 PM
HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems 4/10/2006 3:15:04 PM
Area (ac) CN Description
9.007 62
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
17.3 Direct Entry,
Subcatchment s06-0(OW): s06 Open Water
Runoff = 1.75 cfs @ 12.00 hrs, Volume= 0.121 af, Depth= 3.40"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 2-yr Rainfall=3.40"
Area (ac) CN Description
0.428 100
Subcatchment s07-1:
Runoff = 2.50 cfs @ 12.16 hrs, Volume= 0.254 af, Depth= 0.66"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 2-yr Rainfall=3.40"
Area (ac) CN Description 4.656 64
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
9.3 Direct Entry,
Subcatchment s07-1(OW): s07 Open Water
Runoff = 2.07 cfs @ 12.00 hrs, Volume= 0.143 af, Depth= 3.40"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 2-yr Rainfall=3.40"
Area (ac) CN Description 0.506 100
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
0.0 Direct Entry,

Proposed Conditions_10454-01Type III 24-hr 2-yrRainfall=3.40"Prepared by The Chazen CompaniesPage 56HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems4/10/2006 3:15:04 PM
Subcatchment s08-1:
Runoff = 5.62 cfs @ 12.47 hrs, Volume= 0.942 af, Depth= 0.49"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 2-yr Rainfall=3.40"
Area (ac) CN Description 23.126 60
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
24.5 Direct Entry,
Subcatchment s08-2:
Runoff = 4.03 cfs @ 12.19 hrs, Volume= 0.457 af, Depth= 0.61"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 2-yr Rainfall=3.40"
Area (ac) CN Description
8.958 63
Tc       Length       Slope       Velocity       Capacity       Description         (min)       (feet)       (ft/ft)       (ft/sec)       (cfs)
11.4 Direct Entry,
Subcatchment s08-2(IC):
Runoff = 20.40 cfs @ 12.04 hrs, Volume= 1.458 af, Depth= 3.17"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 2-yr Rainfall=3.40"
Area (ac)CNDescription5.52498
Tc     Length     Slope     Velocity     Capacity     Description       (min)     (feet)     (ft/ft)     (ft/sec)     (cfs)
2.9 Direct Entry,
Subcatchment s08-2(OW):
Runoff = 0.78 cfs @ 12.00 hrs, Volume= 0.054 af, Depth= 3.40"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 2-yr Rainfall=3.40"

Proposed Conditions_10454-01	Type III 24-hr 2-yr Rainfall=3.40"
Prepared by The Chazen Companies HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Syster	Page 57 ns 4/10/2006 3:15:04 PM
	4/10/2000 3.13.04 FIV
Area (ac) CN Description	
0.192 100	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
0.0 Direct Entry,	
Subcatchment s08-3:	
Runoff = 0.56 cfs @ 12.25 hrs, Volume= 0.075 af,	Depth= 0.53"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, Type III 24-hr 2-yr Rainfall=3.40"	dt= 0.01 hrs
Area (ac) CN Description	
1.700 61	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
13.6 Direct Entry,	
Subcatchment s08-3(IC): s08-3 Imperv	ious Cover
Runoff = 4.28 cfs @ 12.01 hrs, Volume= 0.287 af,	Depth= 3.17"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, Type III 24-hr 2-yr Rainfall=3.40"	dt= 0.01 hrs
Area (ac) CN Description	
1.086 98	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
1.0Direct Entry,	
Subcatchment s08-3(OW): s08-3 Op	en Water
Runoff = 0.17 cfs @ 12.00 hrs, Volume= 0.012 af,	Depth= 3.40"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, Type III 24-hr 2-yr Rainfall=3.40"	dt= 0.01 hrs
Area (ac) CN Description	
0.042 100	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
0.0 Direct Entry,	

Proposed Conditions_10454-01Type III 24-hr 2-yrRainfall=3.40"Prepared by The Chazen CompaniesPage 58HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems4/10/2006 3:15:04 PM
Subcatchment s09-1:
Runoff = 0.86 cfs @ 12.17 hrs, Volume= 0.106 af, Depth= 0.49"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 2-yr Rainfall=3.40"
Area (ac) CN Description 2.604 60
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
9.0 Direct Entry,
Subcatchment s09-2:
Runoff = 9.91 cfs @ 12.34 hrs, Volume= 1.232 af, Depth= 0.79"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 2-yr Rainfall=3.40"
Area (ac) CN Description
18.608 67
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
20.6 Direct Entry,
Subcatchment s09-2(IC): s09-2 Impervious Cover
Runoff = 8.75 cfs @ 12.04 hrs, Volume= 0.616 af, Depth= 3.17"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 2-yr Rainfall=3.40"
Area (ac) CN Description
2.336 98
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
2.5 Direct Entry,
Subcatchment s09-2(OW): s09-2 Open Water
Runoff = 0.96 cfs @ 12.00 hrs, Volume= 0.067 af, Depth= 3.40"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 2-yr Rainfall=3.40"

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Area (ac) CN Description
0.236 100
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
0.0 Direct Entry,
Subcatchment s09-3:
Runoff = 2.80 cfs @ 12.17 hrs, Volume= 0.268 af, Depth= 0.84"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 2-yr Rainfall=3.40"
Area (ac) CN Description
3.818 68
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
10.9 Direct Entry,
Subcatchment s10-1:
Runoff = 4.10 cfs @ 12.44 hrs, Volume= 0.565 af, Depth= 0.84"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 2-yr Rainfall=3.40"
Area (ac) CN Description
8.038 68
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
27.9 Direct Entry,
Subcatchment s10-1(OW): s10 Open Water
Runoff = 3.39 cfs @ 12.00 hrs, Volume= 0.235 af, Depth= 3.40"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 2-yr Rainfall=3.40"
Area (ac) CN Description 0.830 100

Proposed Conditions_10454-01Type III 24-hr 2-yrRainfall=3.40"Prepared by The Chazen CompaniesPage 60HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems4/10/2006 3:15:05 PM
Subcatchment s13-1:
Runoff = 1.71 cfs @ 12.07 hrs, Volume= 0.157 af, Depth= 0.53"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 2-yr Rainfall=3.40"
Area (ac) CN Description
3.555 61
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
2.8Direct Entry,
Subcatchment s13-1(IC): s13-1 Impervious Cover
Runoff = 23.58 cfs @ 12.04 hrs, Volume= 1.678 af, Depth= 3.17"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 2-yr Rainfall=3.40"
Area (ac) CN Description
6.360 98
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
2.8     Direct Entry,
Subcatchment s13-1(OW): s13-1 Open Water
Runoff = 0.53 cfs @ 12.00 hrs, Volume= 0.037 af, Depth= 3.40"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 2-yr Rainfall=3.40"
Area (ac) CN Description
0.131 100
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
0.0 Direct Entry,
Subcatchment s14-1:
Runoff = 7.53 cfs @ 12.45 hrs, Volume= 1.023 af, Depth= 0.89"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 2-yr Rainfall=3.40"

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Area (ac) CN Description 13.727 69
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
28.1Direct Entry,
Subcatchment s14-1(IC): s14-1 Impervious Cover
Runoff = 6.95 cfs @ 12.03 hrs, Volume= 0.486 af, Depth= 3.17"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 2-yr Rainfall=3.40"
Area (ac) CN Description
1.840 98
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
2.3Direct Entry,
Subcatchment s14-1(OW): s14 Open Water
Runoff = 2.11 cfs @ 12.00 hrs, Volume= 0.147 af, Depth= 3.40"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 2-yr Rainfall=3.40"
Area (ac) CN Description
0.518 100
Subcatchment s14-2:
Runoff = 0.21 cfs @ 12.12 hrs, Volume= 0.022 af, Depth= 0.53"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 2-yr Rainfall=3.40"
Area (ac) CN Description
0.504 61
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
6.4 Direct Entry,

Proposed Conditions_10454-01	Type III 24-hr 2-yr Rainfall=3.40"	
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Subcatchment s14-2(OW): s14-2 Open Water		
Runoff = 0.72 cfs @ 12.00 hrs, Volume= 0.050 af,	Depth= 3.40"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, Type III 24-hr 2-yr Rainfall=3.40"	dt= 0.01 hrs	
Area (ac) CN Description 0.176 100		
Tc Length Slope Velocity Capacity Description		
(min) (feet) (ft/ft) (ft/sec) (cfs) 0.0 Direct Entry,		
Subcatchment s14-3:		
Runoff = 5.85 cfs @ 12.13 hrs, Volume= 0.507 af,	Depth= 0.89"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, Type III 24-hr 2-yr Rainfall=3.40"	dt= 0.01 hrs	
Area (ac) CN Description 6.794 69		
Tc Length Slope Velocity Capacity Description		
(min) (feet) (ft/ft) (ft/sec) (cfs) 8.5 <b>Direct Entry</b> ,		
Subcatchment s14-3(IC): s14-3 Imperv		
Runoff = 31.25 cfs @ 12.04 hrs, Volume= 2.233 af,	Depth= 3.17"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, Type III 24-hr 2-yr Rainfall=3.40"	dt= 0.01 hrs	
Area (ac) CN Description 8.460 98		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
2.9 Direct Entry,		
Subcatchment s16-1:		
Runoff = 21.81 cfs @ 12.31 hrs, Volume= 2.627 af,	Depth= 0.79"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, Type III 24-hr 2-yr Rainfall=3.40"	dt= 0.01 hrs	

Proposed Conditions_10454-01	Type III 24-hr 2-yr Rainfall=3.40"
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Area (ac) CN Description	
39.680 67	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
19.0 Direct Entry,	
Subcatchment s16-1(OW): s16-1 Ope	en Water
Runoff = 21.84 cfs @ 12.00 hrs, Volume= 1.516 af,	Depth= 3.40"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, Type III 24-hr 2-yr Rainfall=3.40"	dt= 0.01 hrs
Area (ac) CN Description	
5.351 100	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
(min) (feet) (ft/ft) (ft/sec) (cfs) 0.0 Direct Entry,	
Subcatchment s16-2:	
Runoff = 2.37 cfs @ 12.23 hrs, Volume= 0.235 af,	Depth= 1.29"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, Type III 24-hr 2-yr Rainfall=3.40"	dt= 0.01 hrs
Area (ac) CN Description	
2.176 76	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
15.8 Direct Entry,	
Subcatchment s17-1:	
Runoff = 2.03 cfs @ 12.58 hrs, Volume= 0.334 af,	Depth= 0.66"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, Type III 24-hr 2-yr Rainfall=3.40"	dt= 0.01 hrs
Area (ac) CN Description	
6.110 64	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
33.8 Direct Entry,	

Proposed Conditions_10454-01	Type III 24-hr 2-yr Rainfall=3.40"	
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HydroCAD® 7.00 S/H 000927 @ 1986-2003 Applied Microcomputer System	s 4/10/2006 3:15:05 PM	
Subcatchment s17-1(OW): s17-1 Ope	n Water	
Runoff = $0.67 \text{ cfs} @ 12.00 \text{ hrs}$ , Volume= $0.046 \text{ af}$ ,	Depth= 3.40"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, o Type III 24-hr 2-yr Rainfall=3.40"	dt= 0.01 hrs	
Area (ac) CN Description		
0.164 100		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
0.0 Direct Entry,		
Subcatchment s17-2:		
Runoff = 16.23 cfs @ 13.50 hrs, Volume= 4.734 af,	Depth= 0.75"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, o Type III 24-hr 2-yr Rainfall=3.40"	dt= 0.01 hrs	
Area (ac) CN Description		
76.086 66		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
95.7 Direct Entry,		
Subcatchment s17-3:		
Runoff = 12.41 cfs @ 12.51 hrs, Volume= 1.859 af,	Depth= 0.75"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, o Type III 24-hr 2-yr Rainfall=3.40"	dt= 0.01 hrs	
Area (ac) CN Description		
29.880 66		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
30.7 Direct Entry,		
Subcatchment s18-1:		
Runoff = 3.71 cfs @ 12.27 hrs, Volume= 0.460 af,	Depth= 0.66"	
	1 0 04 has	

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 2-yr Rainfall=3.40"

	Type III 24-hr 2-yr Rainfall=3.40"
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Area (ac) CN Description	
8.429 64	
Tc Length Slope Velocity Capacity Description	
(min) (feet) (ft/ft) (ft/sec) (cfs)	
16.4 Direct Entry,	
Subcatchment s18-1(OW): s18-1 Oper	ו Water
Runoff = 1.93 cfs @ 12.00 hrs, Volume= 0.134 af, I	Depth= 3.40"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, df Type III 24-hr 2-yr Rainfall=3.40"	t= 0.01 hrs
Area (ac) CN Description 0.472 100	
Subcatchment s18-2:	
Runoff = 7.04 cfs @ 12.28 hrs, Volume= 0.799 af, [	Depth= 0.89"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, df Type III 24-hr 2-yr Rainfall=3.40"	t= 0.01 hrs
Area (ac) CN Description 10.721 69	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
18.0 Direct Entry,	
Subcatchment s19-0:	
Runoff = 3.05 cfs @ 12.70 hrs, Volume= 0.633 af, I	Depth= 0.49"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt Type III 24-hr 2-yr Rainfall=3.40"	t= 0.01 hrs
Area (ac) CN Description	
15.520 60	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
40.4 Direct Entry,	

Proposed Conditions_10454-01Type III 24-hr 2-yrRainfall=3.40"Prepared by The Chazen CompaniesPage 66HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems4/10/2006 3:15:05 PM		
Subcatchment s20-1:		
Runoff = 4.12 cfs @ 12.35 hrs, Volume= 0.533 af, Depth= 0.75"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 2-yr Rainfall=3.40"		
Area (ac) CN Description 8.559 66		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
21.5 Direct Entry,		
Subcatchment s20-1(OW): s20-1 Open Water		
Runoff = 8.03 cfs @ 12.00 hrs, Volume= 0.558 af, Depth= 3.40"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 2-yr Rainfall=3.40"		
Area (ac) CN Description		
1.968 100		
Subcatchment s20-2:		
Runoff = 9.85 cfs @ 12.13 hrs, Volume= 0.796 af, Depth= 1.17"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 2-yr Rainfall=3.40"		
Area (ac) CN Description		
8.157 74		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
8.4 Direct Entry,		
Subcatchment s20-2(IC): s20-2 Impervious Cover		
Runoff = 17.69 cfs @ 12.07 hrs, Volume= 1.349 af, Depth= 3.17"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 2-yr Rainfall=3.40"		
Area (ac) CN Description 5.112 98		

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Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
4.7 Direct Entry,	
Subcatchment s20-2(OW): s20-2 Ope	en Water
Runoff = 0.99 cfs @ 12.00 hrs, Volume= 0.069 af,	Depth= 3.40"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, Type III 24-hr 2-yr Rainfall=3.40"	dt= 0.01 hrs
Area (ac) CN Description	
0.242 100	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
0.0 Direct Entry,	
Cubertahment c20.2	
Subcatchment s20-3:	
Runoff = 4.50 cfs @ 12.34 hrs, Volume= 0.543 af,	Depth= 0.95"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, Type III 24-hr 2-yr Rainfall=3.40"	dt= 0.01 hrs
Area (ac) CN Description	
6.886 70	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
22.0 Direct Entry,	
Subcatchment s21-1:	
Runoff = 40.15 cfs @ 12.26 hrs, Volume= 4.496 af,	Depth= 0.84"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, Type III 24-hr 2-yr Rainfall=3.40"	dt= 0.01 hrs
Area (ac) CN Description 63.942 68	
00.042 00	
Tc Length Slope Velocity Capacity Description	
(min) (feet) (ft/ft) (ft/sec) (cfs) 16.6 Direct Entry,	

Proposed Conditions_10454-01Type III 24-hr 2-yrRainfall=3.40"Prepared by The Chazen CompaniesPage 68HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems4/10/2006 3:15:05 PM
Subcatchment s21-1(OW):
Runoff = 49.94 cfs @ 12.00 hrs, Volume= 3.467 af, Depth= 3.40"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 2-yr Rainfall=3.40"
Area (ac) CN Description 12.235 100
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
0.0 Direct Entry,
Subcatchment s21-2:
Runoff = 11.75 cfs @ 12.49 hrs, Volume= 1.652 af, Depth= 0.95"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 2-yr Rainfall=3.40"
Area (ac) CN Description 20.941 70
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
31.4 Direct Entry,
Subcatchment s21-3:
Runoff = 9.44 cfs @ 12.16 hrs, Volume= 0.836 af, Depth= 1.17"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 2-yr Rainfall=3.40"
Area (ac) CN Description
8.567 74
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
11.2 Direct Entry,
Subcatchment s21-4:
Runoff = 2.11 cfs @ 12.22 hrs, Volume= 0.225 af, Depth= 0.79"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 2-yr Rainfall=3.40"

Proposed Conditions_10454-01 Type III 24-hr 2-yr Rainfall=3.40"
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Area (ac) CN Description
3.392 67
Tc Length Slope Velocity Capacity Description
(min) (feet) (ft/ft) (ft/sec) (cfs)
13.8 Direct Entry,
Subcatchment s21-4(IC): s21-4 Impervious Cover
Runoff = 6.36 cfs @ 12.02 hrs, Volume= 0.434 af, Depth= 3.17"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 2-yr Rainfall=3.40"
Area (ac) CN Description
1.643 98
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
1.6Direct Entry,
Subcatchment s21-4(OW): s21-4 Open Water
Runoff = 0.48 cfs @ 12.00 hrs, Volume= 0.033 af, Depth= 3.40"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 2-yr Rainfall=3.40"
Area (ac) CN Description
0.117 100
Subcatchment s21-5:
Runoff = 2.59 cfs @ 12.20 hrs, Volume= 0.246 af, Depth= 1.23"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 2-yr Rainfall=3.40"
Area (ac) CN Description
2.398 75
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
13.9 Direct Entry,

Proposed Conditions_10454-01Type III 24-hr 2-yrRainfall=3.40"Prepared by The Chazen CompaniesPage 70HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems4/10/2006 3:15:06 PM					
Subcatchment s21-6:					
Runoff = 5.70 cfs @ 12.25 hrs, Volume= 0.589 af, Depth= 1.29"					
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 2-yr Rainfall=3.40"					
Area (ac) CN Description 5.463 76					
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)					
17.5 Direct Entry,					
Subcatchment s21-6(IC): s21-6 Impervious Cover					
Runoff = 2.50 cfs @ 12.02 hrs, Volume= 0.170 af, Depth= 3.17"					
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 2-yr Rainfall=3.40"					
Area (ac) CN Description					
0.643 98 Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)					
1.5 Direct Entry,					
Subcatchment s21-6(OW): s21-6 Open Water					
Runoff = 0.31 cfs @ 12.00 hrs, Volume= 0.022 af, Depth= 3.40"					
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 2-yr Rainfall=3.40"					
Area (ac) CN Description					
0.076 100					
Tc       Length       Slope       Velocity       Capacity       Description         (min)       (feet)       (ft/ft)       (ft/sec)       (cfs)					
0.0 Direct Entry,					
Subcatchment s21-7:					
Runoff = 2.09 cfs @ 12.21 hrs, Volume= 0.239 af, Depth= 0.66"					
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 2-yr Rainfall=3.40"					

• –	Type III 24-hr 2-yr Rainfall=3.40"
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Area (ac) CN Description 4.375 64	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
13.1 Direct Entry,	
Subcatchment s21-7(IC): s21-7 Impervic	ous Cover
Runoff = 14.57 cfs @ 12.04 hrs, Volume= 1.027 af, I	Depth= 3.17"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, d Type III 24-hr 2-yr Rainfall=3.40"	t= 0.01 hrs
Area (ac) CN Description	
3.890 98	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
2.5 Direct Entry,	
Subcatchment s21-7(OW): s21-7 Oper	n Water
Runoff = 0.37 cfs @ 12.00 hrs, Volume= 0.025 af, I	Depth= 3.40"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, d Type III 24-hr 2-yr Rainfall=3.40"	t= 0.01 hrs
Area (ac) CN Description	
0.090 100	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
0.0 Direct Entry,	
Subcatchment s22-1:	
Runoff = 15.70 cfs @ 12.22 hrs, Volume= 1.573 af, I	Depth= 1.06"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, d Type III 24-hr 2-yr Rainfall=3.40"	t= 0.01 hrs
Area (ac) CN Description	
17.878 72	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
14.7 Direct Entry,	

Proposed Conditions_10454-01	Type III 24-hr 2-yr Rainfall=3.40"					
Prepared by The Chazen Companies HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer System	Page 72 ns 4/10/2006 3:15:06 PM					
Subcatchment s22-1(OW): s22-1 Open Water						
Runoff = 0.56 cfs @ 12.00 hrs, Volume= 0.039 af,	Depth= 3.40"					
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, Type III 24-hr 2-yr Rainfall=3.40"	dt= 0.01 hrs					
Area (ac) CN Description 0.136 100						
Subcatchment s22-2:						
Oubcatchinent 322-2.						
Runoff = 24.34 cfs @ 12.39 hrs, Volume= 3.154 af,	Depth= 0.84"					
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, Type III 24-hr 2-yr Rainfall=3.40"	dt= 0.01 hrs					
Area (ac) CN Description						
44.848 68						
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)						
24.0 Direct Entry,						
Subcatchment s23-1:						
Runoff = 16.95 cfs @ 12.58 hrs, Volume= 2.563 af,	Depth= 1.06"					
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, Type III 24-hr 2-yr Rainfall=3.40"	dt= 0.01 hrs					
Area (ac) CN Description						
29.123 72						
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)						
38.5 Direct Entry,						
Subcatchment s23-2:						
Runoff = 14.66 cfs @ 12.06 hrs, Volume= 0.988 af,	Depth= 1.36"					
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, Type III 24-hr 2-yr Rainfall=3.40"	dt= 0.01 hrs					
Area (ac) CN Description						
8.741 77						

Proposed Conditions_10454-01 Prepared by The Chazen Companies	Type III 24-hr 2-yr Rainfall=3.40" Page 73			
HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer System	ns 4/10/2006 3:15:06 PM			
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)				
4.0 Direct Entry,				
Subcatchment s23-2(IC): s23-2 Imperv	ious Cover			
Runoff = 25.49 cfs @ 12.06 hrs, Volume= 1.896 af,	Depth= 3.17"			
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, Type III 24-hr 2-yr Rainfall=3.40"	dt= 0.01 hrs			
Area (ac) CN Description				
7.185 98				
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)				
4.0 Direct Entry,				
Subcatchment s23-2(OW): s23-2 Op	en Water			
Runoff = 0.69 cfs @ 12.00 hrs, Volume= 0.048 af,	Depth= 3.40"			
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 2-yr Rainfall=3.40"				
Area (ac) CN Description				
0.168 100				
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)				
0.0 Direct Entry,				
Subcatchment s24-0:				
Runoff = 16.05 cfs @ 12.48 hrs, Volume= 2.235 af,	Depth= 0.95"			
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, Type III 24-hr 2-yr Rainfall=3.40"	dt= 0.01 hrs			
Area (ac) CN Description				
28.325 70				
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)				
30.7 Direct Entry,				

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Subcatchment s25-0:
Runoff = 6.68 cfs @ 12.33 hrs, Volume= 0.844 af, Depth= 0.75"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 2-yr Rainfall=3.40"
Area (ac) CN Description 13.562 66
Tc       Length       Slope       Velocity       Capacity       Description         (min)       (feet)       (ft/ft)       (ft/sec)       (cfs)         20.2       Direct Entry,
Reach 25R:
Overland Flow Reach
Inflow Area =       15.520 ac, Inflow Depth =       0.49" for 2-yr event         Inflow =       1.34 cfs @       13.61 hrs, Volume=       0.631 af         Outflow =       1.34 cfs @       13.73 hrs, Volume=       0.631 af, Atten= 1%, Lag= 7.5 min
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 1.2 fps, Min. Travel Time= 8.8 min Avg. Velocity = 0.5 fps, Avg. Travel Time= 20.5 min
Peak Depth= 0.11' @ 13.73 hrs Capacity at bank full= 175.17 cfs Inlet Invert= 560.00', Outlet Invert= 512.00' 50.00' x 1.00' deep Parabolic Channel, n= 0.060 Length= 620.0' Slope= 0.0774 '/'
Reach r03-1:
Overland Flow Reach Requires more survey
Inflow Area =       11.485 ac, Inflow Depth =       0.84"       for 2-yr event         Inflow =       4.77 cfs @       12.66 hrs, Volume=       0.808 af         Outflow =       4.74 cfs @       12.71 hrs, Volume=       0.808 af, Atten= 1%, Lag= 2.9 min
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 3.8 fps, Min. Travel Time= 3.5 min Avg. Velocity = 1.6 fps, Avg. Travel Time= 8.1 min
Peak Depth= 0.38' @ 12.71 hrs Capacity at bank full= 92.14 cfs Inlet Invert= 845.00', Outlet Invert= 728.00' 10.00' x 1.50' deep Parabolic Channel, n= 0.060 Length= 785.0' Slope= 0.1490 '/'

Type III 24-hr 2-yr Rainfall=3.40"

Proposed Conditions\_10454-01

# Reach r04-1:

Channel

Inflow Area =       26.658 ac, Inflow Depth =       1.07" for 2-yr event         Inflow =       10.95 cfs @       12.56 hrs, Volume=       2.377 af         Outflow =       10.94 cfs @       12.58 hrs, Volume=       2.377 af, Atten= 0%, Lag= 0.7 min						
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 5.4 fps, Min. Travel Time= 1.0 min Avg. Velocity = 1.7 fps, Avg. Travel Time= 3.2 min						
Peak Depth= 0.63' @ 12.58 hrs Capacity at bank full= 530.15 cfs Inlet Invert= 685.50', Outlet Invert= 632.00' 12.00' x 4.00' deep Parabolic Channel, n= 0.060 Length= 330.0' Slope= 0.1621 '/'						
Reach r08-1a:						
Man Made Ditch Inverts of pipe to be surveyed						
Inflow Area =       93.367 ac, Inflow Depth =       0.70" for 2-yr event         Inflow =       24.00 cfs @       12.95 hrs, Volume=       5.478 af         Outflow =       24.00 cfs @       12.96 hrs, Volume=       5.478 af, Atten= 0%, Lag= 0.3 min						
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 8.5 fps, Min. Travel Time= 0.4 min Avg. Velocity = 4.2 fps, Avg. Travel Time= 0.9 min						
Doold Dooth 0 EZ @ 12.06 hro						

Peak Depth= 0.57' @ 12.96 hrs Capacity at bank full= 81.88 cfs Inlet Invert= 607.00', Outlet Invert= 587.00' 10.00' x 1.00' deep Parabolic Channel, n= 0.027 Length= 226.0' Slope= 0.0885 '/'

# Reach r08-1b:

24" HDPE Inverts to be surveyed

Inflow Area = 93.367 ac, Inflow Depth = 0.70" for 2-yr event Inflow = 24.00 cfs @ 12.96 hrs, Volume= 5.478 af Outflow = 24.00 cfs @ 12.96 hrs, Volume= 5.478 af, Atten= 0%, Lag= 0.2 min Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 21.7 fps, Min. Travel Time= 0.2 min Avg. Velocity = 11.1 fps, Avg. Travel Time= 0.4 min

#### Proposed Conditions\_10454-01 *Type* Prepared by The Chazen Companies HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems

Peak Depth= 0.77' @ 12.96 hrs Capacity at bank full= 77.17 cfs Inlet Invert= 587.00', Outlet Invert= 557.75' 24.0" Diameter Pipe n= 0.012 Length= 295.0' Slope= 0.0992 '/'

# Reach r08-1c:

Ditch Pipe inverts to be surveyed

Inflow Area	a =	93.367 ac, I	nflow Depth = 0.70"	for 2-yr event	
Inflow	=	24.00 cfs @	12.96 hrs, Volume=	5.478 af	
Outflow	=	23.98 cfs @	12.98 hrs, Volume=	5.478 af,	Atten= 0%, Lag= 0.8 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 8.1 fps, Min. Travel Time= 1.2 min Avg. Velocity = 3.8 fps, Avg. Travel Time= 2.6 min

Peak Depth= 0.58' @ 12.98 hrs Capacity at bank full= 76.65 cfs Inlet Invert= 557.75', Outlet Invert= 512.00' 10.00' x 1.00' deep Parabolic Channel, n= 0.027 Length= 590.0' Slope= 0.0775 '/

# Reach r08-1d: Amenia Creek/Cascade Brook

(Steam) parallel to Route 22 Base Flow estimated from field observation (see field note 21)

Inflow Area = 114.957 ac, Inflow Depth = 17.39" for 2-yr event Inflow = 66.20 cfs @ 12.99 hrs, Volume= 166.568 af, Incl. 40.00 cfs Base Flow Outflow = 66.09 cfs @ 13.04 hrs, Volume= 166.323 af, Atten= 0%, Lag= 3.2 min Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrsMax. Velocity= 3.5 fps, Min. Travel Time= 3.9 minAvg. Velocity = 3.1 fps, Avg. Travel Time= 4.4 min

Peak Depth= 3.19' @ 13.04 hrs Capacity at bank full= 104.49 cfs Inlet Invert= 512.00', Outlet Invert= 504.00' 10.00' x 4.00' deep Parabolic Channel, n= 0.060 Length= 805.0' Slope= 0.0099 '/'

# Reach r13-1:

Inflow Area	a =	2.176 ac, Inflo	ow Depth = $1.29$ "	for 2-yr event	
Inflow	=	2.37 cfs @ 12	2.23 hrs, Volume=	0.235 af	
Outflow	=	2.33 cfs @ 12	2.25 hrs, Volume=	0.235 af,	Atten= 1%, Lag= 1.5 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 7.0 fps, Min. Travel Time= 2.2 min Avg. Velocity = 2.7 fps, Avg. Travel Time= 5.5 min

# Proposed Conditions\_10454-01TypePrepared by The Chazen CompaniesHydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems

Peak Depth= 0.37' @ 12.25 hrs Capacity at bank full= 17.79 cfs Inlet Invert= 546.00', Outlet Invert= 524.00' 18.0" Diameter Pipe n= 0.012 Length= 900.0' Slope= 0.0244 '/'

# Reach r14-3a:

30" HDPE Under Main Entrance Road

Inflow Area	=	6.422 ac, I	nflow Depth = 0.70'	for 2-yr event	
Inflow =	=	3.62 cfs @	12.19 hrs, Volume	= 0.375 af	
Outflow =	=	3.61 cfs @	12.20 hrs, Volume	= 0.375 af, Atten= 0%, Lag= 0.6 min	
				40.00 has alt 0.04 has	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 9.3 fps, Min. Travel Time= 0.8 min Avg. Velocity = 4.0 fps, Avg. Travel Time= 1.8 min

Peak Depth= 0.33' @ 12.20 hrs Capacity at bank full= 94.91 cfs Inlet Invert= 526.00', Outlet Invert= 505.70' 30.0" Diameter Pipe n= 0.012 Length= 445.0' Slope= 0.0456 '/'

# Reach r14-3b:

Grass lined channel

Inflow Area = Inflow = Outflow =	6.422 ac, Inflow Depth = 0.70" for 2-yr event 3.66 cfs @ 12.17 hrs, Volume= 0.375 af 3.62 cfs @ 12.19 hrs, Volume= 0.375 af, Atten= 1%, Lag= 1.1 min	
Max. Velocity= 4.	tor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs 4 fps, Min. Travel Time= 1.4 min 9 fps, Avg. Travel Time= 3.2 min	

#### Reach r17-1:

Inflow Are	a =	76.086 ac, I	nflow Depth	= 0.75"	for 2-yr event	
Inflow	=	16.23 cfs @	13.50 hrs,	Volume=	4.734 af	
Outflow	=	16.20 cfs @	13.52 hrs,	Volume=	4.734 af,	Atten= 0%, Lag= 1.1 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 5.5 fps, Min. Travel Time= 4.2 min Avg. Velocity = 2.5 fps, Avg. Travel Time= 9.3 min

Proposed Conditions_10454-01Type III 24-hr 2-yrRainfall=Prepared by The Chazen CompaniesPageHydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems4/10/2006 3:15:0					
Capacity at bar Inlet Invert= 640	Peak Depth= 0.65' @ 13.52 hrs Capacity at bank full= 181.28 cfs Inlet Invert= 646.00', Outlet Invert= 524.00' 12.00' x 2.00' deep Parabolic Channel, n= 0.045 Length= 1,390.0' Slope= 0.0878 '/'				
	Reach r18-2:				
Overland Flow	Reach				
Inflow = Outflow =	0.00 cfs @ 0.00 hrs, Volume= 0.000 af 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Atten= 0%, Lag= 0.0 min				
Max. Velocity=	-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs 0.0 fps, Min. Travel Time= 0.0 min 0.0 fps, Avg. Travel Time= 0.0 min				
Inlet Invert= 973	00' @ 0.00 hrs k full= 434.91 cfs 3.60', Outlet Invert= 630.00' deep Parabolic Channel, n= 0.060 Length= 720.0' Slope= 0.4772 '/'				
	Reach r21-1a:				
Man Made Ditc	n				
Inflow Area = Inflow = Outflow =	207.817 ac, Inflow Depth =       0.28" for 2-yr event         2.15 cfs @       14.07 hrs, Volume=       4.789 af         2.15 cfs @       14.12 hrs, Volume=       4.782 af, Atten= 0%, Lag= 3.0 min				
Max. Velocity=	-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs 2.6 fps, Min. Travel Time= 4.2 min 2.0 fps, Avg. Travel Time= 5.5 min				
Capacity at bar Inlet Invert= 504	36' @ 14.12 hrs k full= 191.76 cfs 4.00', Outlet Invert= 494.00' deep Parabolic Channel, n= 0.027 Length= 648.0' Slope= 0.0154 '/'				
	Reach r21-1b:				
Overland Flow	Reach				
Inflow Area = Inflow = Outflow =	29.123 ac, Inflow Depth = 0.76" for 2-yr event 14.23 cfs @ 12.79 hrs, Volume= 1.848 af 14.05 cfs @ 12.81 hrs, Volume= 1.848 af, Atten= 1%, Lag= 1.1 min				
Routina by Dyn	-Stor-Ind method. Time Span= 0.00-48.00 hrs. dt= 0.01 hrs				

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 2.9 fps, Min. Travel Time= 0.9 min Avg. Velocity = 1.2 fps, Avg. Travel Time= 2.2 min Proposed Conditions\_10454-01Type III 24-hPrepared by The Chazen CompaniesHydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems

Peak Depth= 0.28' @ 12.81 hrs Capacity at bank full= 227.81 cfs Inlet Invert= 506.70', Outlet Invert= 485.75' 50.00' x 1.00' deep Parabolic Channel, n= 0.060 Length= 160.0' Slope= 0.1309 '/'

# Reach r22-2:

**Overland Flow Reach** 

Inflow Area = Inflow = Outflow =	15.520 ac, Inflow Depth =0.49"for 2-yr event1.34 cfs @13.55 hrs, Volume=0.631 af1.34 cfs @13.61 hrs, Volume=0.631 af, Atten= 0%, Lag= 3.6 min
Max. Velocity= 2	Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs .3 fps, Min. Travel Time= 4.5 min .1 fps, Avg. Travel Time= 9.8 min

# Reach r25-0a:

Ditch Pipe inverts need to be surveyed

Inflow Area = Inflow = Outflow =	67.391 ac, Inflow Depth = 0.89" 15.05 cfs @ 12.48 hrs, Volume= 14.99 cfs @ 12.51 hrs, Volume=	= 4.992 af	nin
Max. Velocity= 7	Stor-Ind method, Time Span= 0.00-4 7.1 fps, Min. Travel Time= 2.6 min 2.7 fps, Avg. Travel Time= 6.7 min		
Peak Depth= 0.5 Capacity at bank Inlet Invert= 570.			

# 10.00' x 2.00' deep Parabolic Channel, n= 0.027 Length= 1,090.0' Slope= 0.0606 '/'

# Reach r25-0b: Wetland Reach

Wetland Reach Has wetland vegetation within reach

Inflow Area =	9.435 ac, Inflow Depth = $0.66$ "	for 2-yr event
Inflow =	1.10 cfs @ 12.98 hrs, Volume=	0.522 af
Outflow =	1.04 cfs @ 13.23 hrs, Volume=	0.521 af, Atten= 6%, Lag= 15.3 min

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Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 0.9 fps, Min. Travel Time= 14.6 min Avg. Velocity = 0.4 fps, Avg. Travel Time= 34.1 min

Peak Depth= 0.29' @ 13.23 hrs Capacity at bank full= 156.51 cfs Inlet Invert= 504.00', Outlet Invert= 499.50' 20.00' x 3.00' deep Parabolic Channel, n= 0.045 Length= 750.0' Slope= 0.0060 '/'

# Reach r25-0c: Amenia Creek/Cascade Brook

(Steam) parallel to Route 22 Base Flow estimated from field observation (see field note 21)

138.083 ac, Inflow Depth = 28.33" for 2-yr event Inflow Area = Inflow 108.58 cfs @ 13.01 hrs, Volume= 325.976 af, Incl. 40.00 cfs Base Flow = 108.30 cfs @ 13.09 hrs, Volume= Outflow = 325.292 af, Atten= 0%, Lag= 5.0 min Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 2.8 fps, Min. Travel Time= 5.8 min Avg. Velocity = 2.6 fps, Avg. Travel Time= 6.2 min Peak Depth= 5.21' @ 13.09 hrs Capacity at bank full= 67.14 cfs Inlet Invert= 504.00', Outlet Invert= 500.00' 10.00' x 4.00' deep Parabolic Channel, n= 0.060 Length= 975.0' Slope= 0.0041 '/' Pond 8P:

No field note. Water spills over cart path; no storage.

Inflow Area =	41.049 ac, Inflow Depth = 0.94"	for 2-yr event
Inflow =	14.54 cfs @ 12.48 hrs, Volume=	3.209 af
Outflow =	14.54 cfs @ 12.48 hrs, Volume=	3.209 af, Atten= 0%, Lag= 0.0 min
Primary =	14.54 cfs @ 12.48 hrs, Volume=	3.209 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 575.17' @ 12.48 hrs Flood Elev= 574.70' Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= (not calculated)

#	Routing	Invert	Outlet Devices
1	Primary	574.70'	177.0 deg Sharp-Crested Vee/Trap Weir C= 2.46

Primary OutFlow Max=14.54 cfs @ 12.48 hrs HW=575.17' TW=570.58' (Dynamic Tailwater) ←1=Sharp-Crested Vee/Trap Weir (Weir Controls 14.54 cfs @ 1.7 fps)

# Pond p02-2:

Proposed culvert under proposed road at intersection with 44.

Inf	low Area =	7 776 2	ac, Inflow Depth = 0.75" for 2-yr event
	low Alea =		s @ 12.48 hrs, Volume = 0.484 af
	itflow =		s @ 12.48 hrs, Volume= 0.484 af, Atten= 0%, Lag= 0.0 min
Pri	imary =	3.29 cfs	s @ 12.48 hrs, Volume= 0.484 af
Pe Flo Plu Ce	ak Elev= 640. ood Elev= 645 ug-Flow deten	.76' @ 12.4 .00' tion time= (	ethod, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs 8 hrs (not calculated: outflow precedes inflow) (not calculated) Outlet Devices
	0		
	1 Primary	640.00'	<b>24.0</b> " <b>x 100.0</b> ' long Culvert CPP, end-section conforming to fill, Ke= 0.500

Outlet Invert= 638.00' S= 0.0200 '/' n= 0.012 Cc= 0.900

Primary OutFlow Max=3.29 cfs @ 12.48 hrs HW=640.76' TW=621.37' (Dynamic Tailwater) ↓ 1=Culvert (Inlet Controls 3.29 cfs @ 3.0 fps)

# Pond p02-3:

Simulates last DMH at bottom of small road, at intersection with 44. This culvert is only used to size the drain pipe under 44.

Inflow Inflow Outflo Prima	- wo	11.27 cfs 11.27 cfs	c, Inflow Depth = 2.01" for 2-yr event @ 12.03 hrs, Volume= 0.685 af @ 12.03 hrs, Volume= 0.685 af, Atten= 0%, Lag= 0.0 min @ 12.03 hrs, Volume= 0.685 af
Peak Flood Plug-l Cente	Elev= 636.8 Elev= 645. Flow detent er-of-Mass c	38' @ 12.03 00' ion time= (i det. time= (	not calculated: outflow precedes inflow) not calculated)
#	Routing	Invert	Outlet Devices
1	Primary	635.00'	24.0" x 100.0' long Culvert CPP, projecting, no headwall, Ke= 0.900

5	· · ·	<b>j</b> 0,	,
Outlet Invert= 634.00'	S= 0.0100 '/'	n= 0.012	Cc = 0.900

Primary OutFlow Max=11.23 cfs @ 12.03 hrs HW=636.87' TW=552.55' (Dynamic Tailwater) -1=Culvert (Inlet Controls 11.23 cfs @ 3.7 fps)

# Pond p03-2:

Inflow Area	a =	4.738 ac, Inflow Depth = 1.90"	for 2-yr event
Inflow	=	11.31 cfs @ 12.02 hrs, Volume=	= 0.749 af
Outflow	=	1.07 cfs @ 12.81 hrs, Volume=	= 0.746 af, Atten= 91%, Lag= 47.5 min
Primary	=	1.07 cfs @ 12.81 hrs, Volume=	= 0.746 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 774.00' Surf.Area= 2,315 sf Storage= 4,095 cf Peak Elev= 776.59' @ 12.81 hrs Surf.Area= 7,564 sf Storage= 20,242 cf (16,147 cf above start) Flood Elev= 779.00' Surf.Area= 9,991 sf Storage= 41,391 cf (37,296 cf above start) Plug-Flow detention time= 548.6 min calculated for 0.652 af (87% of inflow) Center-of-Mass det. time= 407.5 min (1,198.5 - 791.0)

#	Invert	Avail.St	orage	Storage De	scription		
1	768.00'	51,	363 cf	Custom Sta	age Data (Conic	Listed below	
Elevat (fe	ion et)	Surf.Area (sq-ft)	(0	Inc.Store cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
768.	.00	67		0	0	67	7
770.	.00	345		376	376	361	l
772.	.00	729		1,050	1,426	777	7
772.	.50	842		392	1,819	901	
774.	.00	2,315		2,277	4,095	2,388	3
774.	.50	5,704		1,942	6,037	5,779	)
776.	.00	6,996		9,509	15,546	7,138	3
778.	.00	8,917		15,874	31,420	9,160	)
780.	.00	11,064		19,942	51,363	11,421	
# F	Routing	Invert	Outlet	Devices			
1 F	Primary	774.00'	3.0" Ve	ert. Orifice/G	irate C= 0.600		
2 F	Primary	776.20'	6.0" Ve	ert. Orifice/G	irate X 2.00 C=	0.600	

3 Primary 778.50' 4.0' long x 6.0' high Sharp-Crested Rectangular Weir 2 End Contraction(s)

Primary OutFlow Max=1.07 cfs @ 12.81 hrs HW=776.59' TW=721.20' (Dynamic Tailwater)

-1=Orifice/Grate (Orifice Controls 0.37 cfs @ 7.6 fps)

-2=Orifice/Grate (Orifice Controls 0.70 cfs @ 2.1 fps)

-3=Sharp-Crested Rectangular Weir (Controls 0.00 cfs)

# Pond p04-1:

Storage, inverts and culvert length based on assumed grading, check when final grading becomes available

Inflow Are	a =	34.207 ac, Inflow Depth = 1.02" for 2-yr event
Inflow	=	12.81 cfs @ 12.49 hrs, Volume= 2.908 af
Outflow	=	12.80 cfs @ 12.50 hrs, Volume= 2.907 af, Atten= 0%, Lag= 0.8 min
Primary	=	12.80 cfs @ 12.50 hrs, Volume= 2.907 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

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Peak Elev= 639.72' @ 12.50 hrs Surf.Area= 1,116 sf Storage= 744 cf Flood Elev= 648.00' Surf.Area= 15,680 sf Storage= 66,062 cf Plug-Flow detention time= 2.3 min calculated for 2.907 af (100% of inflow) Center-of-Mass det. time= 2.2 min (979.1 - 977.0)

#	Invert	Avail.Sto	rage Storage Des	scription		
1	638.00'	66,06	62 cf Custom Sta	age Data (Conic) L	isted below	
	ation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft <u>)</u>	
63	38.00	0	0	0	0	
64	40.00	1,300	867	867	1,306	
64	42.00	6,180	6,876	7,743	6,203	
64	44.00	7,270	13,435	21,178	7,438	
64	46.00	11,100	18,235	39,414	11,327	
64	48.00	15,680	26,648	66,062	15,980	
#	Routing	Invert C	Dutlet Devices			
1	Primary	638.00' <b>2</b>	4.0" x 685.0' long	Culvert CPP, en	d-section conform	ming to fill, Ke= 0.500

Outlet Invert= 598.00' S= 0.0584 '/' n= 0.012 Cc= 0.900

Primary OutFlow Max=12.80 cfs @ 12.50 hrs HW=639.72' TW=575.17' (Dynamic Tailwater)

#### Pond p06-0:

Field Note #22 Geometry to be confirmed by survey.

Inflow Area	=	9.435 ac, Inflow Depth = 0.70"	for 2-yr event
Inflow =	=	3.56 cfs @ 12.29 hrs, Volume=	0.549 af
Outflow :	=	1.10 cfs @ 12.98 hrs, Volume=	0.522 af, Atten= 69%, Lag= 41.2 min
Primary =	=	1.10 cfs @ 12.98 hrs, Volume=	0.522 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 506.80' Surf.Area= 18,600 sf Storage= 42,160 cf Peak Elev= 507.21' @ 12.98 hrs Surf.Area= 20,460 sf Storage= 50,896 cf (8,736 cf above start) Flood Elev= 507.10' Surf.Area= 19,958 sf Storage= 48,537 cf (6,377 cf above start) Plug-Flow detention time= (not calculated) Center-of-Mass det. time= (not calculated)

#	Invert	Avail.Stora	age Storage Des	cription	
1	500.00'	67,669	Ocf Custom Sta	<b>ge Data (Conic)</b> Li	sted below
Eleva <sup>.</sup>	tion	Surf.Area	Inc.Store	Cum.Store	Wet.Area
(fe	eet)	(sq-ft)	(cubic-feet)	(cubic-feet)	(sq-ft)
506	).00	0	0	0	0
	5.80	18,600	42,160	42,160	18,672
	3.00	24,030	25,509	67,669	24,138

	Juseu Com			$\sim 112 + 112$ yr $1.011101-0.40$
Prep	ared by The	e Chazen	Companies	Page 84
Hydro	DCAD® 7.00 s	s/n 000927	© 1986-2003 Applied Microcomputer Systems	4/10/2006 3:15:07 PM
#	Routing	Invert	Outlet Devices	
1	Primary	506.80'	12.0" x 20.0' long Culvert CMP, projecting, r	no headwall, Ke= 0.900
			Outlet Invert= 506.00' S= 0.0400 '/' n= 0.024	4 Cc= 0.900
2	Primary	507.10'	178.0 deg Sharp-Crested Vee/Trap Weir C=	2.46
Prim	ary OutFlow	Max=1.1	0 cfs @ 12 98 hrs_HW=507 21'_TW=504 28'_(	(Dynamic Tailwater)

rimary OutFlow Max=1.10 cfs @ 12.98 hrs HW=507.21' TW=504.28' (Dynamic Tailwater) -1=Culvert (Inlet Controls 0.52 cfs @ 1.7 fps) -2=Sharp-Crested Vee/Trap Weir (Weir Controls 0.58 cfs @ 0.8 fps)

#### Pond p07-1:

Field Note # 29 Outlet geometry to be confirmed by survey.

Inflow Area =	26.342 ac, Inflow Depth = 0.87"	for 2-yr event
Inflow =	3.47 cfs @ 12.15 hrs, Volume=	1.920 af
Outflow =	1.40 cfs @ 17.02 hrs, Volume=	1.783 af, Atten= 60%, Lag= 292.1 min
Primary =	1.40 cfs @ 17.02 hrs, Volume=	1.783 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 572.80' Surf.Area= 21,640 sf Storage= 56,264 cf Peak Elev= 573.41' @ 17.02 hrs Surf.Area= 24,492 sf Storage= 71,048 cf (14,784 cf above start) Flood Elev= 573.50' Surf.Area= 24,936 sf Storage= 73,351 cf (17,087 cf above start) Plug-Flow detention time= 1,511.5 min calculated for 0.492 af (26% of inflow) Center-of-Mass det. time= 138.9 min (1,472.4 - 1,333.5)

#	Invert	Avail.St	orage	Storage Des	scription		
1	565.00'	85,	557 cf	Custom Sta	age Data (Conic) L	isted below	
`	eet)	Surf.Area (sq-ft)	(0	Inc.Store	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
565		0		0	0	0	
572		21,640		56,264	56,264	21,735	
574	.00	27,290		29,293	85,557	27,424	
# F	Routing	Invert	Outlet	Devices			
1 F	Primary	572.80'				jecting, no headwall n= 0.024   Cc= 0.90	
2 F	Primary	573.50'				<b>/eir X 2.00</b> C= 2.46	

Primary OutFlow Max=1.40 cfs @ 17.02 hrs HW=573.41' TW=570.29' (Dynamic Tailwater) -1=Culvert (Inlet Controls 1.40 cfs @ 2.1 fps) -2=Sharp-Crested Vee/Trap Weir (Controls 0.00 cfs)

#### Pond p08-2:

Inflow Are	a =	18.762 ac, Inflow Depth = 1.70"	for 2-yr event
Inflow	=	33.19 cfs @ 12.04 hrs, Volume=	= 2.653 af
Outflow	=	2.24 cfs @ 13.93 hrs, Volume=	2.007 af, Atten= 93%, Lag= 113.5 min
Primary	=	2.24 cfs @ 13.93 hrs, Volume=	2.007 af

Type III 24-hr 2-yr Rainfall=3.40"

Proposed Conditions_10454-01	Type III 24-hr 2-yi
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Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 550.00' Surf.Area= 8,558 sf Storage= 24,834 cf Peak Elev= 554.57' @ 13.93 hrs Surf.Area= 19,179 sf Storage= 95,803 cf (70,969 cf above start) Flood Elev= 557.00' Surf.Area= 23,344 sf Storage= 147,597 cf (122,763 cf above start) Plug-Flow detention time= 1,016.3 min calculated for 1.437 af (54% of inflow) Center-of-Mass det. time= 640.3 min (1,434.3 - 793.9)

#	Invert	Avail.Storage	e Storage Des	cription		
1	544.00'	170,918 c	of Custom Sta	ge Data (Conic) Lis	sted below	
	et)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
544.		1,962	0	0	1,962	
546.		3,155	5,070	5,070	3,207	
548.	.00	4,454	7,572	12,642	4,577	
548.	.50	4,796	2,312	14,954	4,940	
550.	.00	8,558	9,880	24,834	8,726	
550.	.50	12,948	5,339	30,173	13,120	
552.	.00	15,129	21,037	51,209	15,390	
554.	.00	18,234	33,315	84,524	18,627	
556.	.00	21,565	39,752	124,277	22,105	
558.	.00	25,122	46,642	170,918	25,823	

#	Routing	Invert	Outlet Devices
1	Primary	550.00'	3.0" Vert. Orifice/Grate C= 0.600
2	Primary	554.09'	12.0" Vert. Orifice/Grate X 2.00 C= 0.600
3	Primary	556.00'	11.0' long x 6.0' high Sharp-Crested Rectangular Weir

2 End Contraction(s)

Primary OutFlow Max=2.24 cfs @ 13.93 hrs HW=554.57' TW=514.91' (Dynamic Tailwater) -1=Orifice/Grate (Orifice Controls 0.50 cfs @ 10.1 fps) -2=Orifice/Grate (Orifice Controls 1.74 cfs @ 2.4 fps) -3=Sharp-Crested Rectangular Weir (Controls 0.00 cfs)

#### Pond p08-3:

Inflow Area =	2.828 ac, Inflow Depth = 1.58"	for 2-yr event
Inflow =	4.49 cfs @ 12.01 hrs, Volume=	0.373 af
Outflow =	0.82 cfs @ 12.54 hrs, Volume=	0.373 af, Atten= 82%, Lag= 31.7 min
Primary =	0.82 cfs @ 12.54 hrs, Volume=	0.373 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 528.00' Surf.Area= 1,849 sf Storage= 2,615 cf Peak Elev= 530.34' @ 12.54 hrs Surf.Area= 3,668 sf Storage= 9,190 cf (6,575 cf above start) Flood Elev= 533.00' Surf.Area= 6,389 sf Storage= 22,602 cf (19,987 cf above start) Plug-Flow detention time= 337.5 min calculated for 0.313 af (84% of inflow) Center-of-Mass det. time= 193.4 min (975.8 - 782.4)

4-hr 2-yr Rainfall=3.40"
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# Invert	Avail.Stora	age Storage Des	cription		
1 524.00'	28,956	6 cf Custom Sta	<b>ge Data (Conic)</b> Li	sted below	
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
524.00	178	0	0	178	
526.00	500	651	651	524	
526.50	548	262	913	587	
528.00	1,849	1,702	2,615	1,900	
530.00	3,344	5,120	7,734	3,437	
532.00	5,240	8,513	16,248	5,388	
534.00	7,538	12,709	28,956	7,755	
<u># Routing</u> 1 Primary	528.00' <b>3.</b>	utlet Devices 0" Vert. Orifice/Gr			
2 Primary	530.00' <b>12</b>	2.0" Vert. Orifice/G	irate C= 0.600		

Primary OutFlow Max=0.82 cfs @ 12.54 hrs HW=530.34' TW=514.89' (Dynamic Tailwater) 1=Orifice/Grate (Orifice Controls 0.35 cfs @ 7.2 fps) 2=Orifice/Grate (Orifice Controls 0.47 cfs @ 2.0 fps)

# Pond p09-2:

Inflow Area =		21.180 ac, Inflow Depth = $1.09$ "	for 2-yr event
Inflow =	=	12.66 cfs @ 12.29 hrs, Volume=	1.915 af
Outflow =	=	1.29 cfs @ 15.71 hrs, Volume=	1.523 af, Atten= 90%, Lag= 205.2 min
Primary =	=	1.29 cfs @ 15.71 hrs, Volume=	1.523 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 586.00' Surf.Area= 10,285 sf Storage= 36,340 cf Peak Elev= 588.83' @ 15.71 hrs Surf.Area= 20,467 sf Storage= 87,194 cf (50,854 cf above start) Flood Elev= 593.00' Surf.Area= 27,610 sf Storage= 187,200 cf (150,860 cf above start) Plug-Flow detention time= 1,329.2 min calculated for 0.688 af (36% of inflow) Center-of-Mass det. time= 621.0 min (1,464.1 - 843.1)

#	Invert	Avail.Storag	ge Storage Des	cription		
1	580.00'	214,790	cf Custom Sta	<b>ge Data (Conic)</b> Li	sted below	
Elevati (fe	ion et)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft <u>)</u>	
580.	.00	3,968	0	0	3,968	
582.	.00	5,102	9,046	9,046	5,198	
584.	.00	6,343	11,423	20,469	6,550	
584.	.50	6,670	3,253	23,722	6,907	
586.	.00	10,285	12,619	36,340	10,554	
586.	.50	16,887	6,725	43,066	17,159	
588.	.00	19,143	27,005	70,070	19,525	
590.	.00	22,349	41,451	111,521	22,890	
592.	.00	25,781	48,089	159,610	26,494	
594.	.00	29,439	55,180	214,790	30,336	

Proposed Conditions 10454-01 Type III 24-hr 2-yr Rainfall=3.40" Prepared by The Chazen Companies Page 87 HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems 4/10/2006 3:15:07 PM # Routing Invert Outlet Devices

- Primary 586.00' **3.0" Vert. Orifice/Grate** C= 0.600 1
- 2 Primary 588.21' 8.0" Vert. Orifice/Grate C= 0.600
- 2.0' long x 6.0' high Sharp-Crested Rectangular Weir 2 End Contraction(s) 3 Primary 592.00'

Primary OutFlow Max=1.29 cfs @ 15.71 hrs HW=588.83' TW=573.38' (Dynamic Tailwater)

-1=Orifice/Grate (Orifice Controls 0.39 cfs @ 7.9 fps)

-2=Orifice/Grate (Orifice Controls 0.90 cfs @ 2.7 fps)

-3=Sharp-Crested Rectangular Weir (Controls 0.00 cfs)

# Pond p10:

Field Note #25 Need to get full story on how this pond works

Inflow Area = Inflow = Outflow =	59.531 ac, Inflow Depth =0.48"for 2-yr event4.62 cfs @12.40 hrs, Volume=2.405 af0.00 cfs @0.00 hrs, Volume=0.000 af, Atten= 100%, Lag= 0.0 min
Starting Elev= 498 Peak Elev= 500.8 Plug-Flow detenti	Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs 8.40' Surf.Area= 36,110 sf Storage= 101,108 cf 87' @ 48.00 hrs Surf.Area= 47,807 sf Storage= 205,882 cf (104,774 cf above start) ion time= (not calculated) let. time= (not calculated)

# Invert	Avail.Storage	Storage De	escription	
1 490.00'	581,029 cf	Custom St	age Data (Conic) Li	sted below
Elevation	Surf.Area	Inc.Store	Cum.Store	Wet.Area
(feet)	(sq-ft) (	cubic-feet)	(cubic-feet)	(sq-ft <u>)</u>
490.00	0	0	0	0
498.40	36,110	101,108	101,108	36,221
500.00	42,400	62,741	163,849	42,610
502.00	54,880	97,012	260,861	55,187
504.00	78,730	132,895	393,755	79,107
506.00	109,382	187,274	581,029	109,836

#### Pond p13-1:

No Field Note Natural depression.

Inflow Are	a =	12.222 ac, Inflow Depth = 2.07	" for 2-yr event
Inflow	=	26.43 cfs @ 12.04 hrs, Volume	e= 2.107 af
Outflow	=	22.81 cfs @ 12.08 hrs, Volume	= 2.091 af, Atten= 14%, Lag= 2.4 min
Primary	=	22.81 cfs @ 12.08 hrs, Volume	e= 2.091 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 524.00' Surf.Area= 5,894 sf Storage= 16,480 cf Peak Elev= 526.48' @ 12.08 hrs Surf.Area= 9,268 sf Storage= 35,610 cf (19,131 cf above start) Flood Elev= 527.00' Surf.Area= 10,067 sf Storage= 40,862 cf (24,383 cf above start)

Plug-Flow detention time= 348.5 min calculated for 1.712 af (81% of inflow) Center-of-Mass det. time= 202.2 min (977.7 - 775.5)

#	Invert	Avail.S	torage	Storage Description				
1	518.00'	50,	891 cf	Custom Sta	custom Stage Data (Conic) Listed below			
	vation (feet)	Surf.Area (sq-ft)	(0	Inc.Store cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)		
5	18.00	1,331		0	0	1,331		
5	20.00	2,048		3,353	3,353	2,104		
5	22.00	2,912		4,935	8,288	3,037		
5	22.50	3,150		1,515	9,803	3,294		
5	24.00	5,894		6,676	16,480	6,061		
5	26.00	8,542		14,354	30,834	8,776		
5	28.00	11,592		20,057	50,891	11,908		
<u>#</u>	Routing Primary	Invert 524.00'		Devices	rate C= 0.600			
2	Primary	525.90'	15.0' lo	Vert. Orifice/Grate C= 0.600 long x 1.3' high Sharp-Crested Rectangular Weir d Contraction(s)				

Primary OutFlow Max=22.79 cfs @ 12.08 hrs HW=526.48' TW=499.38' (Dynamic Tailwater) -1=Orifice/Grate (Orifice Controls 0.36 cfs @ 7.4 fps) 2=Sharp Croated Bactongular Wair (Wair Controls 22.42 cfs @ 2.6 fps)

2=Sharp-Crested Rectangular Weir (Weir Controls 22.43 cfs @ 2.6 fps)

# Pond p14-1:

Field Note #26 Need to figure out how this pond works

Inflow Area = Inflow = Outflow = Primary =	50.663 ac, Inflow Depth =1.64"for 2-yr event62.90 cfs @12.08 hrs, Volume=6.905 af1.16 cfs @23.63 hrs, Volume=1.605 af, Atten= 98%, Lag= 692.9 min1.16 cfs @23.63 hrs, Volume=1.605 af					
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 497.40' Surf.Area= 22,200 sf Storage= 54,760 cf Peak Elev= 502.44' @ 23.63 hrs Surf.Area= 75,714 sf Storage= 305,667 cf (250,907 cf above start) Plug-Flow detention time= 1,945.2 min calculated for 0.348 af (5% of inflow) Center-of-Mass det. time= 815.0 min (1,742.6 - 927.5)						

#	Invert	Avail.Storage	Storage Description
1	490.00'	805,062 cf	Custom Stage Data (Conic) Listed below

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Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
490.00	0	0	0	0
497.40	22,200	54,760	54,760	22,286
498.00	25,330	14,249	69,009	25,433
500.00	52,810	76,476	145,485	52,948
502.00	73,360	125,608	271,093	73,574
504.00	84,070	157,308	428,402	84,467
506.00	92,130	176,139	604,540	92,797
508.00	108,618	200,522	805,062	109,437

# Routing Invert Outlet Devices

1 Primary 500.00' **24.0" x 80.0' long Culvert** CPP, end-section conforming to fill, Ke= 0.500 Outlet Invert= 502.00' S= -0.0250 '/' n= 0.012 Cc= 0.900

Primary OutFlow Max=1.16 cfs @ 23.63 hrs HW=502.44' TW=499.82' (Dynamic Tailwater)

## Pond p14-2:

Inflow Area	a =	15.934 ac, Inflow Depth = 2.12"	' for 2-yr event	
Inflow	=	35.40 cfs @ 12.05 hrs, Volume=	= 2.811 af	
Outflow	=	30.16 cfs @ 12.09 hrs, Volume=	= 2.783 af, Atten= 15%, Lag= 2.8	min
Primary	=	30.16 cfs @ 12.09 hrs, Volume=	= 2.783 af	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 532.00' Surf.Area= 7,681 sf Storage= 23,903 cf Peak Elev= 534.33' @ 12.09 hrs Surf.Area= 11,245 sf Storage= 46,238 cf (22,335 cf above start) Flood Elev= 535.00' Surf.Area= 12,390 sf Storage= 54,538 cf (30,635 cf above start) Plug-Flow detention time= 322.3 min calculated for 2.234 af (79% of inflow) Center-of-Mass det. time= 172.2 min (947.6 - 775.4)

#	Invert	Avail.St	torage Storage De	escription		
1	526.00'	66,	889 cf Custom S	tage Data (Conic)	Listed below	
Eleva	ation feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
52	6.00	2,239	0	0	2,239	
52	8.00	3,156	5,369	5,369	3,227	
53	0.00	4,207	7,338	12,707	4,362	
53	0.50	4,491	2,174	14,881	4,669	
53	2.00	7,681	9,023	23,903	7,885	
53	4.00	10,686	18,285	42,188	10,966	
53	6.00	14,093	24,701	66,889	14,463	
#	Routing	Invert	Outlet Devices			
1 2	Primary Primary	532.00' 533.60'	3.0" Vert. Orifice/0 14.0' long x 1.5' hi 2 End Contraction	igh Sharp-Crested	Rectangular We	ir

Primary OutFlow Max=30.13 cfs @ 12.09 hrs HW=534.33' TW=499.45' (Dynamic Tailwater) 1=Orifice/Grate (Orifice Controls 0.35 cfs @ 7.1 fps) 2=Sharp-Crested Rectangular Weir (Weir Controls 29.78 cfs @ 3.0 fps)

# Pond p16-1:

Field Note # 49 Large pond with man-made island. Geometry to be verified by survey. In particular, we are making big guesses about the outlets. Also need to find out about valves...

Inflow Area =	176.893 ac, Inflow Depth = 0.85"	for 2-yr event
Inflow =	29.36 cfs @ 12.30 hrs, Volume=	12.506 af
Outflow =	1.43 cfs @ 25.98 hrs, Volume=	2.318 af, Atten= 95%, Lag= 820.9 min
Primary =	1.43 cfs @ 25.98 hrs, Volume=	2.318 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 507.00' Surf.Area= 199,799 sf Storage= 878,320 cf Peak Elev= 509.61' @ 25.98 hrs Surf.Area= 241,196 sf Storage= 1,383,367 cf (505,047 cf above start) Flood Elev= 510.50' Surf.Area= 271,550 sf Storage= 1,623,217 cf (744,897 cf above start) Plug-Flow detention time= (not calculated) Center-of-Mass det. time= (not calculated)

#	Invert	Avail.S	torage	Storage D	escription		
1 4	500.00'	2,062,	087 cf	Custom S	tage Data (Conic)	Listed below	
El su seti su	_	Curf Anna			Ourse Otherse	Mat Area	
Elevatior	-	Surf.Area		Inc.Store	Cum.Store	Wet.Area	
(feet	:)	(sq-ft)	(0	cubic-feet)	(cubic-feet)	(sq-ft)	
500.00	0	0		0	0	0	
503.00	0	140,344		140,344	140,344	140,358	
509.20	0	232,500		1,143,862	1,284,206	232,994	
510.00	0	249,400		192,720	1,476,927	249,951	
512.00	0	338,000		585,160	2,062,087	338,634	
# Ro	uting	Invert	Outlet	Devices			
1 Pri	mary	509.00'	18.0"	x 110.0' lon	a Culvert CMP, p	projecting, no hea	dwall, Ke= 0.900
	,				.70' S= 0.0300 '/'		-
2 Pri	mary	500.00'	8.0" x	100.0' long	assumed equalization	ation pipe w/ val	ve X 0.00
				, .	.00' S= 0.0000 '/'		0.900
3 Pri	mary	510.50'	175.0 c	deg Sharp-O	Crested Vee/Trap \	Weir X 2.00 C=	2.46
	,			<b>U</b>	•		
503.00 509.20 510.00 512.00 <u># Ro</u> 1 Prin 2 Prin	0 0 0 0 <u>uting</u> mary mary	140,344 232,500 249,400 338,000 Invert 509.00' 500.00'	Outlet 18.0" x Outlet 8.0" x CMP, p Outlet	140,344 1,143,862 192,720 585,160 Devices x 110.0' lon Invert= 505. 100.0' long projecting, n Invert= 500.	140,344 1,284,206 1,476,927 2,062,087 <b>g Culvert</b> CMP, p .70' S= 0.0300 '/' a <b>ssumed equaliz</b> to headwall, Ke= 0	140,358 232,994 249,951 338,634 projecting, no hea n= 0.024 Cc= 0 ation pipe w/ val 0.900 n= 0.013 Cc= 0	0.900 I <b>ve X 0.00</b> 0.900

Primary OutFlow Max=1.42 cfs @ 25.98 hrs HW=509.61' TW=505.48' (Dynamic Tailwater)

-1=Culvert (Inlet Controls 1.42 cfs @ 2.1 fps)

-2=assumed equalization pipe w/ valve (Controls 0.00 cfs)

-3=Sharp-Crested Vee/Trap Weir (Controls 0.00 cfs)

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# Pond p17-1:

Field Note #45 Golf pond Geometry to be confirmed by surveyed

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 523.80' Surf.Area= 7,290 sf Storage= 9,234 cf Peak Elev= 525.24' @ 13.41 hrs Surf.Area= 10,496 sf Storage= 22,790 cf (13,556 cf above start) Flood Elev= 524.30' Surf.Area= 8,074 sf Storage= 13,623 cf (4,389 cf above start) Plug-Flow detention time= 43.7 min calculated for 6.761 af (97% of inflow) Center-of-Mass det. time= 19.4 min (972.2 - 952.8)

#	Invert	Avail.St	torage	Storage Des	scription		
1	520.00'	30,	224 cf	Custom Sta	age Data (Conic) L	isted below	
`	ition eet) 0.00	Surf.Area (sq-ft) 0		Inc.Store ubic-feet) 0	Cum.Store (cubic-feet) 0	Wet.Area (sq-ft) 0	
523 524	3.80 4.00 6.00	7,290 7,300 12,460		9,234 1,459 19,531	9,234 10,693 30,224	7,313 7,374 12,581	
#	Routing	Invert	Outlet D	Devices			
2	Primary Primary Primary	523.80' 524.30' 525.20'	Head (f Coef. (E <b>143.0 d</b>	eet) 0.20 0 English) 2.8 <b>eg Sharp-C</b> i	adth Broad-Crest .40 0.60 0.80 1.0 0 2.92 3.08 3.30 rested Vee/Trap W ong Sharp-Creste	00 3.32 Veir C= 2.47	

Primary OutFlow Max=20.35 cfs @ 13.41 hrs HW=525.24' TW=515.55' (Dynamic Tailwater) =Broad-Crested Rectangular Weir (Weir Controls 12.60 cfs @ 4.0 fps)

-2=Sharp-Crested Vee/Trap Weir (Weir Controls 6.30 cfs @ 2.4 fps)

**3=Sharp-Crested Vee/Trap Weir** (Weir Controls 1.45 cfs @ 0.6 fps)

# Pond p18-1:

Field Note #46 Golf pond Geometry to be confirmed by surveyed

Inflow Area =	131.862  ac,  Inflow Depth = 0.76"	for 2-yr event
Inflow =	22.44 cfs @ 13.37 hrs, Volume=	8.366 af
Outflow =	22.27 cfs @ 13.50 hrs, Volume=	8.362 af, Atten= 1%, Lag= 8.0 min
Primary =	22.27 cfs @ 13.50 hrs, Volume=	8.362 af

Proposed Conditions_10454-01	Type III 24-hr
Prepared by The Chazen Companies	
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e III 24-hr 2-yr Rainfall=3.40" Page 92 4/10/2006 3:15:08 PM

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 513.90' Surf.Area= 20,680 sf Storage= 26,884 cf Peak Elev= 515.55' @ 13.50 hrs Surf.Area= 26,574 sf Storage= 66,717 cf (39,833 cf above start) Flood Elev= 514.81' Surf.Area= 23,768 sf Storage= 48,709 cf (21,825 cf above start) Plug-Flow detention time= 115.8 min calculated for 7.745 af (93% of inflow) Center-of-Mass det. time= 55.6 min (1,011.7 - 956.1)

#	Invert	Avail.Storag	e Storage De	scription				
1	510.00'	148,288 0	of Custom Sta	Custom Stage Data (Conic) Listed below				
Elevat	ion eet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)			
510	.00	0	0	0	0			
513.	.90	20,680	26,884	26,884	20,704			
514.	.00	20,690	2,068	28,952	20,756			
516	.00	28,290	48,782	77,735	28,436			
518.	.00	42,760	70,554	148,288	42,967			

#	Routing	Invert	Outlet Devices
1	Primary	513.90'	2.0' long x 0.5' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00
			Coef. (English) 2.80 2.92 3.08 3.30 3.32
2	Primary	514.81'	143.0 deg Sharp-Crested Vee/Trap Weir C= 2.47
3	Primary	515.32'	175.0 deg x 10.0' long Sharp-Crested Vee/Trap Weir C= 2.46
	•		

Primary OutFlow Max=22.27 cfs @ 13.50 hrs HW=515.55' TW=507.93' (Dynamic Tailwater) -1=Broad-Crested Rectangular Weir (Weir Controls 14.05 cfs @ 4.3 fps) -2=Sharp-Crested Vee/Trap Weir (Weir Controls 3.46 cfs @ 2.1 fps)

-3=Sharp-Crested Vee/Trap Weir (Weir Controls 4.76 cfs @ 1.4 fps)

# Pond p19-0:

Wetland Geometry to be confirmed by survey Based off aerial topo, and assumed topo contour

Pond Unchanged from existing to proposed conditions

Inflow Area = Inflow = Outflow = Primary = Secondary =	15.520 ac, Inflow Depth = 0.49 3.05 cfs @ 12.70 hrs, Volume 1.34 cfs @ 13.55 hrs, Volume 1.34 cfs @ 13.55 hrs, Volume 0.00 cfs @ 0.00 hrs, Volume	= 0.633 af = 0.631 af, Atten= 56%, Lag= 50.5 min = 0.631 af			
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 972.00' Surf.Area= 86,000 sf Storage= 57,333 cf Peak Elev= 972.06' @ 13.55 hrs Surf.Area= 87,584 sf Storage= 63,780 cf (6,447 cf above start Plug-Flow detention time= (not calculated) Center-of-Mass det. time= (not calculated)					

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# Invert	Avail.Sto	orage Storage De	escription			
1 970.00'	282,3	329 cf Custom St	age Data (Conic)	Listed below		
Elevation (feet) 970.00 972.00 974.00	Surf.Area (sq-ft) 0 86,000 141,270	Inc.Store (cubic-feet) 0 57,333 224,996	Cum.Store (cubic-feet) 0 57,333 282,329	Wet.Area (sq-ft) 0 86,006 141,327		
<ul><li># Routing</li><li>1 Secondary</li><li>2 Primary</li></ul>	973.60' 972.00'	Outlet Devices <b>178.0 deg x 51.0'</b> <b>35.0' long x 0.5' b</b> Head (feet) 0.20 ( Coef. (English) 2.8	readth Broad-Cres	sted Rectangula 00		

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Primary OutFlow Max=1.34 cfs @ 13.55 hrs HW=972.06' TW=970.07' (Dynamic Tailwater) **2=Broad-Crested Rectangular Weir** (Weir Controls 1.34 cfs @ 0.7 fps)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=972.00' TW=973.60' (Dynamic Tailwater) —1=Sharp-Crested Vee/Trap Weir (Controls 0.00 cfs)

# Pond p20-1:

Field Note #50 Spring Fed Pond Geometry to be confirmed by surveyed

Inflow Area	=	207.817 ac, Inflow Depth = 0.31"	for 2-yr event
Inflow =	=	10.81 cfs @ 12.31 hrs, Volume=	5.310 af
Outflow =	=	2.15 cfs @ 14.07 hrs, Volume=	4.789 af, Atten= 80%, Lag= 105.7 min
Primary =	=	2.15 cfs @ 14.07 hrs, Volume=	4.789 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 505.10' Surf.Area= 89,370 sf Storage= 138,524 cf Peak Elev= 505.52' @ 14.07 hrs Surf.Area= 89,375 sf Storage= 175,981 cf (37,457 cf above start) Plug-Flow detention time= 1,635.2 min calculated for 1.609 af (30% of inflow) Center-of-Mass det. time= 177.5 min (1,695.9 - 1,518.5)

#	Invert	Avail.Storag	e Storage Des	scription	
1	502.00'	615,682 c	of Custom Sta	ige Data (Prismatio	c) Listed below
Elevatio (fee		Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
502.0	0	0	0	0	
505.1 506.0		89,370 89,380	138,524 80,437	138,524 218,961	
508.0 510.0		99,280 108,781	188,660 208,061	407,621 615,682	

Type III 24-hr 2-yr Rainfall=3.40" Page 93 s 4/10/2006 3:15:08 PM

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Type III 24-hr 2-yr Rainfall=3.40" Page 94 HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems 4/10/2006 3:15:08 PM

#	Routing	Invert	Outlet Devices
1	Primary	505.10'	<b>3.0' long x 1.5' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 Coef. (English) 2.62 2.64 2.64 2.68 2.75 2.86 2.92 3.07 3.07 3.03 3.28
2	Primary	506.20'	3.32 6.5' long x 1.5' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 Coef. (English) 2.62 2.64 2.64 2.68 2.75 2.86 2.92 3.07 3.07 3.03 3.28 3.32
3	Primary	506.00'	176.0 deg x 97.0' long Sharp-Crested Vee/Trap Weir C= 2.46

Primary OutFlow Max=2.15 cfs @ 14.07 hrs HW=505.52' TW=504.36' (Dynamic Tailwater)

-1=Broad-Crested Rectangular Weir (Weir Controls 2.15 cfs @ 1.7 fps)

-2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

-3=Sharp-Crested Vee/Trap Weir (Controls 0.00 cfs)

# Pond p20-2:

Inflow Are	ea =	13.511 ac, I	nflow Depth = 1.97"	for 2-yr event	
Inflow	=	26.49 cfs @	12.08 hrs, Volume=	2.214 af	
Outflow	=	0.48 cfs @	20.06 hrs, Volume=	1.359 af,	Atten= 98%, Lag= 478.9 min
Primary	=	0.48 cfs @	20.06 hrs, Volume=	1.359 af	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 552.00' Surf.Area= 10,535 sf Storage= 35,913 cf Peak Elev= 556.27' @ 20.06 hrs Surf.Area= 20,905 sf Storage= 110,186 cf (74,273 cf above start) Flood Elev= 559.00' Surf.Area= 25,653 sf Storage= 174,016 cf (138,102 cf above start) Plug-Flow detention time= 1,780.5 min calculated for 0.534 af (24% of inflow) Center-of-Mass det. time= 901.0 min (1,692.2 - 791.2)

# Inver	t Avail.S	torage Storage De	escription		
1 546.00	' 199,	647 cf Custom S	tage Data (Conic)	Listed below	
Elevation	Surf.Area	Inc.Store	Cum.Store	Wet.Area	
(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)	(sq-ft)	
546.00	3,714	0	0	3,714	
548.00	4,960	8,644	8,644	5,044	
550.00	6,308	11,241	19,885	6,493	
550.50	6,661	3,242	23,127	6,874	
552.00	10,535	12,786	35,913	10,779	
552.50	15,037	6,360	42,273	15,285	
554.00	17,268	24,209	66,483	17,616	
556.00	20,441	37,664	104,147	20,935	
558.00	23,840	44,237	148,384	24,494	
560.00	27,465	51,262	199,647	28,292	
# Routing	Invert	Outlet Devices			
1 Primary	552.00'	3.0" Vert. Orifice/	Grate C= 0.600		
2 Primary	558.20'	6.1' long x 6.2' hig	gh Sharp-Crested F	Rectangular Weir	2 End Contraction(s)

Primary OutFlow Max=0.48 cfs @ 20.06 hrs HW=556.27' TW=505.44' (Dynamic Tailwater) -1=Orifice/Grate (Orifice Controls 0.48 cfs @ 9.8 fps) -2=Sharp-Crested Rectangular Weir (Controls 0.00 cfs)

# Pond p21-1:

Inflow Area =	459.188 ac, Inflow Depth = 0.71"	for 2-yr event
Inflow =	104.52 cfs @ 12.32 hrs, Volume=	27.286 af
Outflow =	14.96 cfs @ 16.77 hrs, Volume=	25.820 af, Atten= 86%, Lag= 267.0 min
Primary =	14.96 cfs @ 16.77 hrs, Volume=	25.820 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 483.12' @ 16.77 hrs Surf.Area= 360,734 sf Storage= 481,539 cf Plug-Flow detention time= 468.6 min calculated for 25.820 af (95% of inflow) Center-of-Mass det. time= 386.2 min (1,496.1 - 1,109.9)

#	Invert	Avail.St	orage S	Storage D	escription		
1	480.40'	5,244,8	885 cf <b>(</b>	Custom S	tage Data (Conic)	Listed below	
Elevat (fe	tion eet)	Surf.Area (sq-ft)	•	nc.Store bic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
480 482	-	0 202,230		0 107,856	0 107,856	0 202,234	
484 486		485,198 1,275,481		667,114 698,237	774,970 2,473,208	485,231 1,275,541	
488	8.00	1,499,208	2,	771,678	5,244,885	1,499,423	
#_F	Routing	Invert	Outlet D	evices			
1 F	Primary	480.40'	30.0" x	70.0' long	g Culvert CMP, p	orojecting, no head	wall, Ke= 0.900

Outlet Invert= 480.40' S= 0.0000 '/' n= 0.024 Cc= 0.900

Primary OutFlow Max=14.96 cfs @ 16.77 hrs HW=483.12' TW=0.00' (Dynamic Tailwater) **1=Culvert** (Barrel Controls 14.96 cfs @ 3.5 fps)

#### Pond p21-4:

Inflow Area	=	5.152 ac, Inflow Depth = $1.61$ "	for 2-yr event
Inflow =	=	7.41 cfs @ 12.02 hrs, Volume=	0.691 af
Outflow :	=	1.24 cfs @ 12.73 hrs, Volume=	0.683 af, Atten= 83%, Lag= 42.4 min
Primary :	=	1.24 cfs @ 12.73 hrs, Volume=	0.683 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 496.00' Surf.Area= 5,112 sf Storage= 14,306 cf Peak Elev= 498.24' @ 12.73 hrs Surf.Area= 7,805 sf Storage= 28,965 cf (14,659 cf above start) Flood Elev= 499.00' Surf.Area= 8,847 sf Storage= 35,622 cf (21,317 cf above start) Plug-Flow detention time= 941.9 min calculated for 0.354 af (51% of inflow) Center-of-Mass det. time= 438.3 min (1,232.7 - 794.4)

1,784

2,530

2.733

5,112

7,468

10,226

Invert

490.00'

#

1

Elevation

(feet) 490.00

492.00

494.00

494.50

496.00

498.00

500.00

e Chazen Cor	mpanies	Page 96		
s/n 000927 © 1	986-2003 Applied	Microcomputer Syst	ems	4/10/2006 3:15:08 PM
Avail.Stora	ge Storage Des	cription		
44,433	cf Custom Sta	<b>ge Data (Conic)</b> Lis	sted below	
Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft <u>)</u>	
1,146	0	0	1,146	

2,907

7,199

8,514

14,306

26,812

44,433

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Type III 24-hr 2-yr Rainfall=3.40"

1,839

2,654

2.876

5,278

7,699

10,536

#	Routing	Invert	Outlet Devices	
1	Primary	496.00'	3.0" Vert. Orifice/Grate C= 0.600	
2	Primary	498.10'	5.0' long x 2.0' high Sharp-Crested Rectangular Weir 2 End Contraction	(s)

2,907

4,292

1,315

5,791

12,506

17,622

Primary OutFlow Max=1.24 cfs @ 12.73 hrs HW=498.24' TW=482.51' (Dynamic Tailwater) -1=Orifice/Grate (Orifice Controls 0.34 cfs @ 7.0 fps) -2=Sharp-Crested Rectangular Weir (Weir Controls 0.90 cfs @ 1.3 fps)

# Pond p21-5:

Inflow Area =	2.398 ac, Inflow Depth = 1	.23" for 2-yr event
Inflow =	2.59 cfs @ 12.20 hrs, Volu	me= 0.246 af
Primary =	2.59 cfs @ 12.20 hrs, Volu	me= 0.246 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

# Pond p21-6:

Inflow Area =	6.182  ac,  Inflow Depth = 1.51"	for 2-yr event
Inflow =	6.59 cfs @ 12.24 hrs, Volume=	0.780 af
Outflow =	0.97 cfs @ 13.43 hrs, Volume=	0.773 af, Atten= 85%, Lag= 71.3 min
Primary =	0.97 cfs @ 13.43 hrs, Volume=	0.773 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 492.00' Surf.Area= 3,323 sf Storage= 4,847 cf Peak Elev= 494.11' @ 13.43 hrs Surf.Area= 12,339 sf Storage= 22,242 cf (17,395 cf above start) Flood Elev= 495.00' Surf.Area= 13,824 sf Storage= 34,456 cf (29,609 cf above start) Plug-Flow detention time= 685.3 min calculated for 0.661 af (85% of inflow) Center-of-Mass det. time= 516.0 min (1,350.5 - 834.5)

#	Invert	Avail.Storage	Storage Description
1	488.00'	48,245 cf	Custom Stage Data (Conic) Listed below

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Elevation	Surf.Area	Inc.Store	Cum.Store	Wet.Area
(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)	(sq-ft)
488.00	296	0	0	296
490.00	924	1,162	1,162	946
490.50	1,110	508	1,670	1,141
492.00	3,323	3,177	4,847	3,367
492.50	6,166	2,336	7,182	6,212
494.00	12,147	13,484	20,666	12,214
496.00	15,500	27,579	48,245	15,669

Routing Invert Outlet Devices # 1

Primary 492.00' **3.0" Vert. Orifice/Grate** C= 0.600

5.0' long x 2.0' high Sharp-Crested Rectangular Weir 2 End Contraction(s) 2 Primary 494.00'

Primary OutFlow Max=0.97 cfs @ 13.43 hrs HW=494.11' TW=482.79' (Dynamic Tailwater) -1=Orifice/Grate (Orifice Controls 0.33 cfs @ 6.8 fps)

-2=Sharp-Crested Rectangular Weir (Weir Controls 0.63 cfs @ 1.1 fps)

# Pond p21-7:

Inflow Are	a =	8.355 ac, Inflow Depth = 1.85"	for 2-yr event
Inflow	=	15.45 cfs @ 12.04 hrs, Volume=	1.291 af
Outflow	=	1.42 cfs @ 13.09 hrs, Volume=	1.277 af, Atten= 91%, Lag= 63.1 min
Primary	=	1.42 cfs @ 13.09 hrs, Volume=	1.277 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 492.00' Surf.Area= 3,941 sf Storage= 8,984 cf Peak Elev= 496.54' @ 13.09 hrs Surf.Area= 9,503 sf Storage= 39,167 cf (30,183 cf above start) Flood Elev= 499.00' Surf.Area= 13,379 sf Storage= 67,369 cf (58,385 cf above start) Plug-Flow detention time= 748.9 min calculated for 1.071 af (83% of inflow) Center-of-Mass det. time= 548.4 min (1,327.1 - 778.7)

1 486.00'	80,	712 cf Custom S			
			tage Data (Conic)	Listed below	
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft <u>)</u>	
486.00	478	0	0	478	
488.00	964	1,414	1,414	999	
490.00	1,601	2,538	3,952	1,684	
490.50	1,782	845	4,797	1,879	
492.00	3,941	4,187	8,984	4,056	
494.00	6,120	9,981	18,965	6,292	
496.00	8,702	14,746	33,712	8,944	
498.00	11,686	20,315	54,027	12,012	
500.00	15,071	26,685	80,712	15,495	
<ul><li># Routing</li><li>1 Primary</li><li>2 Primary</li><li>3 Primary</li></ul>	Invert 492.00' 496.05' 498.00'	Outlet Devices 3.0" Vert. Orifice/0 6.0" Vert. Orifice/0 5.0' long x 6.0' bio	Grate X 2.00 C= 0		2 End Contraction(s)

Primary OutFlow Max=1.42 cfs @ 13.09 hrs HW=496.54' TW=482.69' (Dynamic Tailwater) -1=Orifice/Grate (Orifice Controls 0.50 cfs @ 10.1 fps) -2=Orifice/Grate (Orifice Controls 0.93 cfs @ 2.4 fps) -3=Sharp-Crested Rectangular Weir (Controls 0.00 cfs)

# Pond p22-1:

Field Note #54 Golf Pond Geometry to be confirmed by survey

Pond unchanged from existing to proposed conditions

Inflow Area =	78.382  ac,  Inflow Depth = 0.83"	for 2-yr event
Inflow =	37.07 cfs @ 12.32 hrs, Volume=	5.397 af
Outflow =	36.21 cfs @ 12.39 hrs, Volume=	5.099 af, Atten= 2%, Lag= 3.9 min
Primary =	36.21 cfs @ 12.39 hrs, Volume=	5.099 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 498.10' Surf.Area= 6,520 sf Storage= 10,106 cf Peak Elev= 501.19' @ 12.39 hrs Surf.Area= 10,261 sf Storage= 36,138 cf (26,032 cf above start) Plug-Flow detention time= 103.7 min calculated for 4.866 af (90% of inflow) Center-of-Mass det. time= 42.4 min (950.0 - 907.7)

#	Invert	Avail.St	torage Storage D	escription	
1	495.00'	143,	770 cf Custom S	tage Data (Prism	atic) Listed below
	ation	Surf.Area	Inc.Store	Cum.Store	
(	(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)	
49	95.00	0	0	0	
49	98.10	6,520	10,106	10,106	
50	00.00	8,390	14,164	24,270	
50	)2.00	11,530	19,920	44,190	
50	04.00	14,530	26,060	70,250	
50	06.00	18,340	32,870	103,120	
50	00.80	22,310	40,650	143,770	
#	Routing	Invert	Outlet Devices		
1	Primary	499.75'	18.0" x 21.0' long	g Culvert CMP, p	projecting, no headwall, Ke= 0.900
	-		Outlet Invert= 499	.75' S= 0.0000 '/	n= 0.024 Cc= 0.900
2	Primary	500.50'	1.0' long x 15.0' l	preadth Broad-Cr	ested Rectangular Weir
	-				1.00 1.20 1.40 1.60
			Coef. (English) 2.	68 2.70 2.70 2.6	64 2.63 2.64 2.64 2.63
3	Primary	500.50'	20.0' long x 13.5'	breadth Broad-C	Frested Rectangular Weir
	-		Head (feet) 0.20	0.40 0.60 0.80	1.00 1.20 1.40 1.60
			Coef. (English) 2.	62 2.66 2.70 2.6	66 2.65 2.66 2.65 2.63

Primary OutFlow Max=36.21 cfs @ 12.39 hrs HW=501.19' TW=482.25' (Dynamic Tailwater) 1=Culvert (Barrel Controls 3.84 cfs @ 2.8 fps) -2=Broad-Crested Rectangular Weir (Weir Controls 1.54 cfs @ 2.2 fps) -3=Broad-Crested Rectangular Weir (Weir Controls 30.83 cfs @ 2.2 fps)

# Pond p23-1:

Inflow Area	a =	29.123 ac, Inflow Depth = 1.06"	for 2-yr event	
Inflow	=	16.95 cfs @ 12.58 hrs, Volume=	= 2.563 af	
Outflow	=	14.23 cfs @ 12.79 hrs, Volume=	= 1.848 af, Atten= 16%, Lag= 12.7 r	nin
Primary	=	14.23 cfs @ 12.79 hrs, Volume=	= 1.848 af	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 507.79' @ 12.79 hrs Surf.Area= 14,065 sf Storage= 32,176 cf Plug-Flow detention time= 161.1 min calculated for 1.848 af (72% of inflow) Center-of-Mass det. time= 60.0 min (954.9 - 894.8)

#	Invert	Avail.Stora	ge Storage Des	cription	
1	503.50'	68,915	cf Custom Sta	<b>ge Data (Conic)</b> Li	sted below
Elevat (fe	ion eet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
503	.50	0	0	0	0
504	.00	2,390	398	398	2,390
506	.00	9,090	10,761	11,159	9,110
508	.00	14,660	23,529	34,688	14,732
510	.00	19,690	34,227	68,915	19,847
# F	Routing	Invert Ou	tlet Devices		

1 Primary 507.70' **178.0 deg x 178.0' long Sharp-Crested Vee/Trap Weir** C= 2.46

Primary OutFlow Max=14.22 cfs @ 12.79 hrs HW=507.79' TW=506.97' (Dynamic Tailwater) ←1=Sharp-Crested Vee/Trap Weir (Weir Controls 14.22 cfs @ 0.9 fps)

# Pond p23-2:

Inflow Are	a =	16.094 ac, Inflow Depth = 2.19"	for 2-yr event
Inflow	=	40.54 cfs @ 12.06 hrs, Volume=	2.932 af
Outflow	=	2.33 cfs @ 13.94 hrs, Volume=	2.128 af, Atten= 94%, Lag= 113.1 min
Primary	=	2.33 cfs @ 13.94 hrs, Volume=	2.128 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 508.00' Surf.Area= 7,318 sf Storage= 15,927 cf Peak Elev= 513.04' @ 13.94 hrs Surf.Area= 20,869 sf Storage= 97,221 cf (81,294 cf above start) Flood Elev= 515.00' Surf.Area= 24,788 sf Storage= 141,986 cf (126,059 cf above start) Plug-Flow detention time= 917.1 min calculated for 1.762 af (60% of inflow) Center-of-Mass det. time= 657.6 min (1,442.0 - 784.4)

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 Type III 24-hr 2-yr
 Rainfall=3.40"

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# Invert	Avail.Storag	e Storage Des	cription			
1 502.00'	U	¥	ge Data (Conic) Lis	sted below		
Elevation	Surf.Area	Inc.Store	Cum.Store	Wet.Area		
(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)	(sq-ft)		
502.00	826	0	0	826		
504.00	1,667	2,444	2,444	1,702		
506.00	2,788	4,407	6,852	2,872		
506.50	3,112	1,474	8,326	3,210		
508.00	7,318	7,601	15,927	7,432		
508.50	12,618	4,924	20,851	12,735		
510.00	15,208	20,839	41,690	15,400		
512.00	18,859	34,002	75,692	19,166		
514.00	22,736	41,535	117,227	23,175		
516.00	26,840	49,519	166,746	27,428		
# Routing	Invert Out	let Devices				
1 Primary	508.00' <b>3.0</b> "	Vert. Orifice/Gr	ate C= 0.600			
2 Primary	512.55' <b>12.0</b>	" Vert. Orifice/G	Grate X 2.00 C= 0.0	600		
3 Primary	514.00' <b>20.0</b>	)' long x 6.0' hig	h Sharp-Crested R	ectangular Weir		
,		nd Contraction(s)		5		
				2.90' (Dynamic Tailwater)		
	ate (Orifice Cont					
	ate (Orifice Cont					
└─3=Sharp-Cre	sted Rectangula	ar Weir (Control	ls 0.00 cfs)			
		Pond 7DP	1: Design Point	1		
			1. Design i onit	1		
Field note #10.						
	ons to be confirm	ed by survey				
Inflow Area =	26.658 ac. Inf	flow Depth = 1.0	07" for 2-yr even	t		
Inflow =		12.56 hrs, Volun				
Outflow =		12.56 hrs, Volun		af, Atten= 0%, Lag= 0.0 min		
Primary =						
	Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs					
	Peak Elev= 721.30' @ 12.56 hrs Surf.Area= 57 sf Storage= 36 cf					
	.00' Surf.Area=					
Plug-Flow detention time= 0.2 min calculated for 2.377 af (100% of inflow)						

Plug-Flow detention time= 0.2 min calculated for 2.377 af (100% of inflow) Center-of-Mass det. time= 0.1 min (997.2 - 997.1)

 #	Invert	Avail.Storage	Storage Description
1	720.10'	3,706 cf	Custom Stage Data (Conic) Listed below

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Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft <u>)</u>
720.10	0	0	0	0
722.00	90	57	57	96
724.00	340	403	460	364
726.00	760	1,072	1,533	815
728.00	1,450	2,173	3,706	1,543

#	Routing	Invert	Outlet Devices
1	Primary	720.10'	<b>42.0" x 120.0' long Culvert</b> CMP, square edge headwall, Ke= 0.500
	-		Outlet Invert= 700.00' S= 0.1675 '/' n= 0.024 Cc= 0.900
2	Primary	727.00'	155.0 deg Sharp-Crested Vee/Trap Weir C= 2.47

Primary OutFlow Max=10.95 cfs @ 12.56 hrs HW=721.30' TW=686.13' (Dynamic Tailwater) 1=Culvert (Inlet Controls 10.95 cfs @ 3.7 fps) 2=Sharp Crosted Vac/Trap Weir (Controls 0.00 cfs)

-2=Sharp-Crested Vee/Trap Weir (Controls 0.00 cfs)

# Pond zDP2: Design Point 2

Field Note #15 Culvert dimensions to be confirmed by survey Overflow to ditch is currently discarded... We may have to model that area...

Inflow Area =	93.367 ac, Inflow Depth = 0.70"	for 2-yr event
Inflow =	24.04 cfs @ 12.94 hrs, Volume=	5.478 af
Outflow =	24.00 cfs @ 12.95 hrs, Volume=	5.478 af, Atten= 0%, Lag= 1.0 min
Discarded =	0.00 cfs @ 0.00 hrs, Volume=	0.000 af
Primary =	24.00 cfs @ 12.95 hrs, Volume=	5.478 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 623.12' @ 12.95 hrs Surf.Area= 539 sf Storage= 760 cf Flood Elev= 624.50' Surf.Area= 925 sf Storage= 1,728 cf Plug-Flow detention time= 0.4 min calculated for 5.478 af (100% of inflow) Center-of-Mass det. time= 0.2 min (938.2 - 937.9)

#	Invert	Avail.St	torage	Storage Des	scription		
1	619.60'	7,280 cf		Custom Stage Data (Conic) Listed below			
<b>F</b> lav	ation	Current Arrage		la o Ctoro	Curra Starra		
-	ation	Surf.Area		Inc.Store	Cum.Store	Wet.Area	
	(feet)	(sq-ft)	()	cubic-feet)	(cubic-feet)	(sq-ft)	
61	19.60	0		0	0	0	
62	20.00	10		1	1	10	
62	22.00	260		214	215	269	
62	24.00	760		976	1,192	793	
62	26.00	1,420		2,146	3,338	1,492	
62	28.00	2,580		3,943	7,280	2,694	
#	Routing	Invert	Outlet	Devices			
1	Primary	619.60'			<b>Culvert</b> RCP, er 0' S= 0.0773 '/'		ming to fill, Ke= 0.500
2	Discarded	624.50'			rested Vee/Trap W		

**Discarded OutFlow** Max=0.00 cfs @ 0.00 hrs HW=619.60' (Free Discharge) **1**–2=Sharp-Crested Vee/Trap Weir (Controls 0.00 cfs)

Primary OutFlow Max=24.00 cfs @ 12.95 hrs HW=623.12' TW=607.57' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 24.00 cfs @ 7.6 fps)

# Pond zDP3: Design Point 3

Inflow Area =		228.471 ac, Inflow Depth = 17.42"	" for 2-yr event	
Inflow	=	121.12 cfs @ 12.92 hrs, Volume=	= 331.646 af	
Primary	=	121.12 cfs @ 12.92 hrs, Volume=	= 331.646 af, Atten= 0%, Lag= 0.0 m	in

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

# Pond zDP4: Design Point 4

Inflow Area =		459.188 ac, Inflow Depth = 0.67'	for 2-yr event	
Inflow	=	14.96 cfs @ 16.77 hrs, Volume=	= 25.820 af	
Primary	=	14.96 cfs @ 16.77 hrs, Volume=	= 25.820 af, Atten= 0%, Lag= 0.0 min	i

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

# Pond zDP5: Design Point 5

Inflow Area =	28.325 ac, Inflow Depth = 0.95"	for 2-yr event
Inflow =	16.05 cfs @ 12.48 hrs, Volume=	2.235 af
Primary =	16.05 cfs @ 12.48 hrs, Volume=	2.235 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

# Post-Development Conditions 10 year 24 hour Storm Event Model Computations

Proposed Conditions_10454-01Type III 24-hr 10-yrRainfall=5.00"Prepared by The Chazen CompaniesPage 103HydroCAD® 7.00s/n 000927© 1986-2003 Applied Microcomputer Systems4/10/20063:15:22 PM			
Subcatchment s01-0:			
Runoff = 11.66 cfs @ 12.61 hrs, Volume= 1.799 af, Depth= 1.88"			
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=5.00"			
Area (ac) CN Description 11.485 68			
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)			
42.8 Direct Entry,			
Subcatchment s02-1:			
Runoff = 60.88 cfs @ 12.87 hrs, Volume= 11.794 af, Depth= 1.65"			
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=5.00"			
Area (ac) CN Description			
85.591 65			
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)			
61.3Direct Entry,			
Subcatchment s02-2:			
Runoff = 8.59 cfs @ 12.44 hrs, Volume= 1.120 af, Depth= 1.73"			
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=5.00"			
Area (ac) CN Description			
7.776 66			
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)			
29.4Direct Entry,			
Subcatchment s02-3:			
Runoff = 19.21 cfs @ 12.02 hrs, Volume= 1.181 af, Depth= 3.47"			
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=5.00"			

Proposed Conditions_10454-01	Type III 24-hr 10-yr Rainfall=5.00"
Prepared by The Chazen Companies HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Syste	Page 104 ems 4/10/2006 3:15:22 PM
	4/10/2000 3.13.22 111
Area (ac) CN Description	
4.088 86	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
1.6 Direct Entry,	
Subcatchment s03-1:	
Runoff = 14.04 cfs @ 12.42 hrs, Volume= 1.771 a	f, Depth= 2.04"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs Type III 24-hr 10-yr Rainfall=5.00"	s, dt= 0.01 hrs
Area (ac) CN Description	
10.435 70	
Tc Length Slope Velocity Capacity Description	
(min) (feet) (ft/ft) (ft/sec) (cfs)	
28.8 Direct Entry,	
Subcatchment s03-2:	
Runoff = 9.82 cfs @ 12.02 hrs, Volume= 0.595 a	f, Depth= 2.36"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=5.00"	
Area (ac) CN Description	
3.021 74	
Tc Length Slope Velocity Capacity Description	
(min) (feet) (ft/ft) (ft/sec) (cfs) 1.5 Direct Entry,	
1.5 Direct Lifti y,	
Subcatchment s03-2(IC): s03-2 Impervious Cover	
Runoff = 9.55 cfs @ 12.02 hrs, Volume= 0.660 a	f, Depth= 4.76"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=5.00"	
Area (ac) CN Description	
1.663 98	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
1.5 Direct Entry,	

Proposed Conditions_10454-01Type III 24-hr 10-yrRainfall=5.00"Prepared by The Chazen CompaniesPage 105HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems4/10/2006 3:15:22 PM		
Subcatchment s03-2(OW): s03-2 Open Water		
Runoff = 0.32 cfs @ 12.00 hrs, Volume= 0.022 af, Depth= 5.00"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=5.00"		
Area (ac) CN Description		
0.054 100		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
0.0 Direct Entry,		
Subcatchment s04-1:		
Runoff = 16.12 cfs @ 12.10 hrs, Volume= 1.182 af, Depth= 1.88"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=5.00"		
Area (ac) CN Description		
7.549 68		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
6.1 Direct Entry,		
Subcatchment s05-1:		
Runoff = 7.60 cfs @ 12.21 hrs, Volume= 0.781 af, Depth= 1.37"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=5.00"		
Area (ac) CN Description		
6.842 61		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
14.4Direct Entry,		
Subcatchment s06-0:		
Runoff = 9.91 cfs @ 12.26 hrs, Volume= 1.080 af, Depth= 1.44"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs		

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=5.00"

Prepared by The Chazen Companies	ype III 24-hr 10-yr Rainfall=5.00" Page 106	
HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems	<u>4/10/2006 3:15:23 PM</u>	
Area (ac) CN Description		
9.007 62		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
17.3 Direct Entry,		
Subcatchment s06-0(OW): s06 Open	Water	
Runoff = 2.57 cfs @ 12.00 hrs, Volume= 0.178 af,	Depth= 5.00"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=5.00"		
Area (ac) CN Description 0.428 100		
Subcatchment s07-1:		
Runoff = 7.24 cfs @ 12.14 hrs, Volume= 0.613 af,	Depth= 1.58"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=5.00"		
Area (ac) CN Description 4.656 64		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
9.3 Direct Entry,		
Subcatchment s07-1(OW): s07 Open Water		
Runoff = 3.04 cfs @ 12.00 hrs, Volume= 0.211 af,	Depth= 5.00"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=5.00"		
Area (ac) CN Description		
0.506 100		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
0.0 Direct Entry,		

Proposed Conditions_10454-01Type III 24-hr 10-yrRainfall=5.00"Prepared by The Chazen CompaniesPage 107HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems4/10/2006 3:15:23 PM		
Subcatchment s08-1:		
Runoff = 19.51 cfs @ 12.39 hrs, Volume= 2.507 af, Depth= 1.30"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=5.00"		
Area (ac) CN Description 23.126 60		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
24.5 Direct Entry,		
Subcatchment s08-2:		
Runoff = 12.28 cfs @ 12.17 hrs, Volume= 1.126 af, Depth= 1.51"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=5.00"		
Area (ac) CN Description		
8.958 63		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
11.4 Direct Entry,		
Subcatchment s08-2(IC):		
Runoff = 30.19 cfs @ 12.04 hrs, Volume= 2.193 af, Depth= 4.76"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=5.00"		
Area (ac) CN Description		
5.524 98		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
2.9Direct Entry,		
Subcatchment s08-2(OW):		
Runoff = 1.15 cfs @ 12.00 hrs, Volume= 0.080 af, Depth= 5.00"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=5.00"		

Proposed Conditions_10454-01	Type III 24-hr 10-yr Rainfall=5.00"	
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Area (ac) CN Description		
0.192 100		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
0.0 Direct Entry,		
Subcatchment s08-3:		
Runoff = 1.93 cfs @ 12.20 hrs, Volume= 0.194 af	f, Depth= 1.37"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=5.00"		
Area (ac) CN Description		
1.700 61		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
13.6 <b>Direct Entry</b> ,		
Subcatchment s08-3(IC): s08-3 Imperv	vious Cover	
Runoff = 6.33 cfs @ 12.01 hrs, Volume= 0.431 af	f, Depth= 4.76"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs Type III 24-hr 10-yr Rainfall=5.00"	, dt= 0.01 hrs	
Area (ac) CN Description		
1.086 98		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
1.0 Direct Entry,		
Subcatchment s08-3(OW): s08-3 Open Water		
Runoff = 0.25 cfs @ 12.00 hrs, Volume= 0.018 at	f, Depth= 5.00"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=5.00"		
Area (ac) CN Description		
0.042 100		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
0.0 Direct Entry,		

Proposed Conditions_10454-01Type III 24-hr 10-yrRainfall=5.00"Prepared by The Chazen CompaniesPage 109HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems4/10/2006 3:15:23 PM		
Subcatchment s09-1:		
Runoff = 3.19 cfs @ 12.14 hrs, Volume= 0.282 af, Depth= 1.30"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=5.00"		
Area (ac) CN Description 2.604 60		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
9.0 Direct Entry,		
Subcatchment s09-2:		
Runoff = 25.15 cfs @ 12.30 hrs, Volume= 2.796 af, Depth= 1.80"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=5.00"		
Area (ac) CN Description		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
20.6 Direct Entry,		
Subcatchment s09-2(IC): s09-2 Impervious Cover		
Runoff = 12.95 cfs @ 12.04 hrs, Volume= 0.927 af, Depth= 4.76"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=5.00"		
Area (ac) CN Description 2.336 98		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
2.5 Direct Entry,		
Subcatchment s09-2(OW): s09-2 Open Water		
Runoff = 1.42 cfs @ 12.00 hrs, Volume= 0.098 af, Depth= 5.00"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=5.00"		

Proposed Conditions_10454-01Type III 24-hr 10-yrRainfall=5.00"Prepared by The Chazen CompaniesPage 110HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems4/10/2006 3:15:23 PM		
Area (ac) CN Description		
0.236 100		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
0.0 Direct Entry,		
Subcatchment s09-3:		
Runoff = 6.91 cfs @ 12.16 hrs, Volume= 0.598 af, Depth= 1.88"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=5.00"		
Area (ac) CN Description		
3.818 68		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
10.9 Direct Entry,		
Subcatchment s10-1:		
Runoff = 10.01 cfs @ 12.42 hrs, Volume= 1.259 af, Depth= 1.88"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=5.00"		
Area (ac) CN Description		
8.038 68		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
27.9 Direct Entry,		
Subcatchment s10-1(OW): s10 Open Water		
Runoff = 4.98 cfs @ 12.00 hrs, Volume= 0.346 af, Depth= 5.00"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=5.00"		
Area (ac) CN Description 0.830 100		

Proposed Conditions_10454-01Type III 24-hr 10-yrRainfall=5.00"Prepared by The Chazen CompaniesPage 111HydroCAD® 7.00s/n 000927© 1986-2003 Applied Microcomputer Systems4/10/20063:15:23 PM		
Subcatchment s13-1:		
Runoff = 5.83 cfs @ 12.05 hrs, Volume= 0.406 af, Depth= 1.37"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=5.00"		
Area (ac) CN Description		
3.555 61		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
2.8Direct Entry,		
Subcatchment s13-1(IC): s13-1 Impervious Cover		
Runoff = 34.89 cfs @ 12.04 hrs, Volume= 2.524 af, Depth= 4.76"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=5.00"		
Area (ac) CN Description		
6.360 98		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
2.8 Direct Entry,		
Subcatchment s13-1(OW): s13-1 Open Water		
Runoff = 0.79 cfs @ 12.00 hrs, Volume= 0.055 af, Depth= 5.00"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=5.00"		
Area (ac) CN Description		
0.131 100		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
0.0 Direct Entry,		
Subcatchment s14-1:		
Runoff = 17.88 cfs @ 12.40 hrs, Volume= 2.239 af, Depth= 1.96"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=5.00"		

Proposed Conditions_10454-01Type III 24-hr 10-yrRainfall=5.00'Prepared by The Chazen CompaniesPage 112HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems4/10/2006 3:15:23 PM		
Area (ac) CN Description 13.727 69		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
28.1 Direct Entry,		
Subcatchment s14-1(IC): s14-1 Impervious Cover		
Runoff = 10.28 cfs @ 12.03 hrs, Volume= 0.730 af, Depth= 4.76"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=5.00"		
Area (ac) CN Description		
1.840 98		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
2.3 Direct Entry,		
Subcatchment s14-1(OW): s14 Open Water		
Runoff = 3.11 cfs @ 12.00 hrs, Volume= 0.216 af, Depth= 5.00"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=5.00"		
Area (ac) CN Description		
0.518 100		
Subcatchment s14-2:		
Runoff = 0.73 cfs @ 12.10 hrs, Volume= 0.058 af, Depth= 1.37"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=5.00"		
Area (ac) CN Description		
0.504 61		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
6.4 Direct Entry,		

Proposed Conditions_10454-01Type III 24-hr 10-yrRainfall=5.00"Prepared by The Chazen CompaniesPage 113HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems4/10/2006 3:15:23 PM		
Subcatchment s14-2(OW): s14-2 Open Water		
Runoff = 1.06 cfs @ 12.00 hrs, Volume= 0.073 af, Depth= 5.00"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=5.00"		
Area (ac) CN Description		
0.176 100		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
0.0 Direct Entry,		
Subcatchment s14-3:		
Runoff = 13.95 cfs @ 12.13 hrs, Volume= 1.108 af, Depth= 1.96"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=5.00"		
Area (ac) CN Description		
6.794 69		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
8.5 Direct Entry,		
Subcatchment s14-3(IC): s14-3 Impervious Cover		
Runoff = 46.24 cfs @ 12.04 hrs, Volume= 3.358 af, Depth= 4.76"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=5.00"		
Area (ac) CN Description		
8.460 98		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
2.9Direct Entry,		
Subcatchment s16-1:		
Runoff = 55.45 cfs @ 12.27 hrs, Volume= 5.962 af, Depth= 1.80"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs		

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=5.00"

Proposed Conditions_10454-01	Type III 24-hr 10-yr Rainfall=5.00"
Prepared by The Chazen Companies HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Syst	Page 114 ems 4/10/2006 3:15:23 PM
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Area (ac) CN Description	
39.680 67	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
19.0 Direct Entry,	
Subcatchment s16-1(OW): s16-1 O	pen Water
Runoff = 32.12 cfs @ 12.00 hrs, Volume= 2.230 a	af, Depth= 5.00"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hr Type III 24-hr 10-yr Rainfall=5.00"	s, dt= 0.01 hrs
Area (ac) CN Description	
5.351 100	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
0.0 Direct Entry,	
Subcatchment s16-2:	
Runoff = $4.78 \text{ cfs} @ 12.22 \text{ hrs}, \text{ Volume} = 0.460 \text{ a}$	af, Depth= 2.54"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hr Type III 24-hr 10-yr Rainfall=5.00"	s, dt= 0.01 hrs
Area (ac) CN Description	
2.176 76	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
15.8 Direct Entry,	
Subcatchment s17-1:	
Runoff = 5.70 cfs @ 12.51 hrs, Volume= 0.805 a	af, Depth= 1.58"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=5.00"	
Area (ac) CN Description	
6.110 64	
Tc Length Slope Velocity Capacity Description	
(min)(feet)(ft/ft)(ft/sec)(cfs)33.8Direct Entry,	

Proposed Conditions_10454-01	Type III 24-hr 10-yr Rainfall=5.00"	
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Subcatchment s17-1(OW): s17-1 Op	en Water	
Runoff = 0.98 cfs @ 12.00 hrs, Volume= 0.068 af	, Depth= 5.00"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, Type III 24-hr 10-yr Rainfall=5.00"	, dt= 0.01 hrs	
Area (ac) CN Description		
0.164 100		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
0.0 Direct Entry,		
Subcatchment s17-2:		
Runoff = 42.48 cfs @ 13.30 hrs, Volume= 10.954 af	, Depth= 1.73"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, Type III 24-hr 10-yr Rainfall=5.00"	, dt= 0.01 hrs	
Area (ac) CN Description		
76.086 66		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
95.7 Direct Entry,		
Subcatchment s17-3:		
Runoff = 32.40 cfs @ 12.45 hrs, Volume= 4.302 af	, Depth= 1.73"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, Type III 24-hr 10-yr Rainfall=5.00"	dt= 0.01 hrs	
Area (ac) CN Description		
29.880 66		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
30.7 Direct Entry,		
Subcatchment s18-1:		
Runoff = 10.66 cfs @ 12.24 hrs, Volume= 1.110 af	, Depth= 1.58"	

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=5.00"

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Area (ac) CN Description		
8.429 64		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
16.4 Direct Entry,		
Subcatchment s18-1(OW): s18-1 Open Water		
Runoff = 2.83 cfs @ 12.00 hrs, Volume= 0.197 af, Depth= 5.00"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=5.00"		
Area (ac) CN Description 0.472 100		
Subcatchment s18-2:		
Runoff = 16.85 cfs @ 12.26 hrs, Volume= 1.749 af, Depth= 1.96"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=5.00"		
Area (ac) CN Description 10.721 69		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
18.0 Direct Entry,		
Subcatchment s19-0:		
Runoff = 10.40 cfs @ 12.62 hrs, Volume= 1.683 af, Depth= 1.30"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=5.00"		
Area (ac) CN Description 15.520 60		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
40.4Direct Entry,		

Proposed Conditions_10454-01 Prepared by The Chazen Companies HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Syste	Type III 24-hr 10-yr         Rainfall=5.00"           Page 117           ems         4/10/2006         3:15:24 PM	
Subcatchment s20-1:		
Runoff = 10.81 cfs @ 12.31 hrs, Volume= 1.232 a	f, Depth= 1.73"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=5.00"		
Area (ac) CN Description		
8.559 66		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
21.5 Direct Entry,		
Subcatchment s20-1(OW): s20-1 Open Water		
Runoff = 11.81 cfs @ 12.00 hrs, Volume= 0.820 a	f, Depth= 5.00"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs Type III 24-hr 10-yr Rainfall=5.00"	, dt= 0.01 hrs	
Area (ac) CN Description		
1.968 100		
Subcatchment s20-2:		
Runoff = 20.70 cfs @ 12.12 hrs, Volume= 1.607 a	f, Depth= 2.36"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=5.00"		
Area (ac) CN Description		
8.157 74		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
8.4 Direct Entry,		
Subcatchment s20-2(IC): s20-2 Impervious Cover		
Runoff = 26.18 cfs @ 12.07 hrs, Volume= 2.029 a	f, Depth= 4.76"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=5.00"		
Area (ac) CN Description		
5.112 98		

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Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
4.7 Direct Entry,		
Subcatchment s20-2(OW): s20-2 Op	ben Water	
Runoff = 1.45 cfs @ 12.00 hrs, Volume= 0.101 at	f, Depth= 5.00"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs Type III 24-hr 10-yr Rainfall=5.00"	, dt= 0.01 hrs	
Area (ac) CN Description		
0.242 100		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
0.0 Direct Entry,		
Subcatchment s20-3:		
Runoff = 10.41 cfs @ 12.32 hrs, Volume= 1.169 at	f, Depth= 2.04"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=5.00"		
Area (ac) CN Description		
6.886 70		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
22.0 Direct Entry,		
Subcatchment s21-1:		
Runoff = 99.04 cfs @ 12.23 hrs, Volume= 10.015 at	f, Depth= 1.88"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=5.00"		
Area (ac) CN Description		
63.942 68		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
16.6 Direct Entry,		

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Subcatchment s21-1(OW):			
Runoff = 73.44 cfs @ 12.00 hrs, Volume= 5.098 af, Depth= 5.00"			
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=5.00"			
Area (ac) CN Description 12.235 100			
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)			
0.0 Direct Entry,			
Subcatchment s21-2:			
Runoff = 27.14 cfs @ 12.46 hrs, Volume= 3.554 af, Depth= 2.04"			
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=5.00"			
Area (ac) CN Description			
20.941 70			
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)			
31.4 Direct Entry,			
Subcatchment s21-3:			
Runoff = 19.87 cfs @ 12.16 hrs, Volume= 1.688 af, Depth= 2.36"			
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=5.00"			
Area (ac) CN Description			
8.567 74			
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)			
11.2 Direct Entry,			
Subcatchment s21-4:			
Runoff = 5.37 cfs @ 12.20 hrs, Volume= 0.510 af, Depth= 1.80"			
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=5.00"			

Proposed Conditions_10454-01Type III 24-hr 10-yr Rainfall=5.00"Prepared by The Chazen CompaniesPage 120HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems4/10/2006 3:15:24 PM		
Area (ac) CN Description 3.392 67		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
13.8Direct Entry,		
Subcatchment s21-4(IC): s21-4 Impervious Cover		
Runoff = 9.40 cfs @ 12.02 hrs, Volume= 0.652 af, Depth= 4.76"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=5.00"		
Area (ac) CN Description		
1.643 98		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
1.6 Direct Entry,		
Subcatchment s21-4(OW): s21-4 Open Water		
Runoff = 0.70 cfs @ 12.00 hrs, Volume= 0.049 af, Depth= 5.00"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=5.00"		
Area (ac) CN Description		
0.117 100		
Subcatchment s21-5:		
Runoff = 5.34 cfs @ 12.19 hrs, Volume= 0.489 af, Depth= 2.45"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=5.00"		
Area (ac) CN Description		
2.398 75		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
13.9 Direct Entry,		

Proposed Conditions_10454-01 Prepared by The Chazen Companies HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Syste	<i>Type III 24-hr 10-yr Rainfall=5.00"</i> Page 121 ms 4/10/2006 3:15:24 PM	
Subcatchment s21-6:		
Runoff = 11.52 cfs @ 12.24 hrs, Volume= 1.154 a	f, Depth= 2.54"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs Type III 24-hr 10-yr Rainfall=5.00"	, dt= 0.01 hrs	
Area (ac) CN Description		
5.463 76		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
17.5 Direct Entry,		
Subcatchment s21-6(IC): s21-6 Imper	vious Cover	
Runoff = 3.69 cfs @ 12.02 hrs, Volume= 0.255 a	f, Depth= 4.76"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs Type III 24-hr 10-yr Rainfall=5.00"	, dt= 0.01 hrs	
Area (ac) CN Description		
0.643 98		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
1.5 Direct Entry,		
Subcatchment s21-6(OW): s21-6 Op	ben Water	
Runoff = 0.46 cfs @ 12.00 hrs, Volume= 0.032 a	f, Depth= 5.00"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=5.00"		
Area (ac) CN Description		
0.076 100		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
0.0 Direct Entry,		
Subcatchment s21-7:		
Runoff = 6.03 cfs @ 12.19 hrs, Volume= 0.576 a	f, Depth= 1.58"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=5.00"		

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Area (ac) CN Description		
4.375 64		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
13.1Direct Entry,		
Subcatchment s21-7(IC): s21-7 Imperv	vious Cover	
Runoff = 21.56 cfs @ 12.04 hrs, Volume= 1.544 at	f, Depth= 4.76"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs Type III 24-hr 10-yr Rainfall=5.00"	, dt= 0.01 hrs	
Area (ac) CN Description		
3.890 98		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
2.5 Direct Entry,		
Subcatchment s21-7(OW): s21-7 Open Water		
Runoff = 0.54 cfs @ 12.00 hrs, Volume= 0.037 at	f, Depth= 5.00"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=5.00"		
Area (ac) CN Description		
0.090 100		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
0.0 Direct Entry,		
Subcatchment s22-1:		
Runoff = 34.67 cfs @ 12.20 hrs, Volume= 3.274 at	f, Depth= 2.20"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=5.00"		
Area (ac) CN Description		
Area (ac) CN Description 17.878 72		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
14.7 Direct Entry,		

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Subcatchment s22-1(OW): s22-1 Open Water		
Runoff = 0.82 cfs @ 12.00 hrs, Volume= 0.057 af, Depth= 5.00"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=5.00"		
Area (ac) CN Description 0.136 100		
Subcatchment s22-2:		
Runoff = 59.74 cfs @ 12.35 hrs, Volume= 7.025 af, Depth= 1.88"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=5.00"		
Area (ac) CN Description		
44.848 68		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
24.0Direct Entry,		
Subcatchment s23-1:		
Runoff = 37.22 cfs @ 12.54 hrs, Volume= 5.334 af, Depth= 2.20"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=5.00"		
Area (ac) CN Description		
29.123 72		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
38.5 Direct Entry,		
Subcatchment s23-2:		
Runoff = 28.90 cfs @ 12.06 hrs, Volume= 1.911 af, Depth= 2.62"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=5.00"		
Area (ac) CN Description 8.741 77		

Proposed Conditions_10454-01Type III 24-hr 10-yrRainfall=5.00"Prepared by The Chazen CompaniesPage 124HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems4/10/2006 3:15:25 PM		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
4.0 Direct Entry,		
Subcatchment s23-2(IC): s23-2 Impervious Cover		
Runoff = 37.73 cfs @ 12.06 hrs, Volume= 2.852 af, Depth= 4.76"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=5.00"		
Area (ac) CN Description		
7.185 98		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
4.0Direct Entry,		
Subcatchment s23-2(OW): s23-2 Open Water		
Runoff = 1.01 cfs @ 12.00 hrs, Volume= 0.070 af, Depth= 5.00"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=5.00"		
Area (ac) CN Description		
0.168 100		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
0.0 Direct Entry,		
Subcatchment s24-0:		
Runoff = 37.09 cfs @ 12.45 hrs, Volume= 4.807 af, Depth= 2.04"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=5.00"		
Area (ac) CN Description		
28.325 70		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
30.7 Direct Entry,		

Proposed Conditions_10454-01Type III 24-hr 10-yrRainfall=5.00"Prepared by The Chazen CompaniesPage 125HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems4/10/2006 3:15:25 PM		
Subcatchment s25-0:		
Runoff = 17.56 cfs @ 12.30 hrs, Volume= 1.953 af, Depth= 1.73"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=5.00"		
Area (ac) CN Description 13.562 66		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
20.2 Direct Entry,		
Reach 25R:		
Overland Flow Reach		
Inflow Area =       15.520 ac, Inflow Depth =       1.30" for 10-yr event         Inflow =       5.84 cfs @       13.14 hrs, Volume=       1.681 af         Outflow =       5.79 cfs @       13.21 hrs, Volume=       1.681 af, Atten= 1%, Lag= 4.5 min		
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 1.8 fps, Min. Travel Time= 5.6 min Avg. Velocity = 0.6 fps, Avg. Travel Time= 16.9 min		
Peak Depth= 0.21' @ 13.21 hrs Capacity at bank full= 175.17 cfs Inlet Invert= 560.00', Outlet Invert= 512.00' 50.00' x 1.00' deep Parabolic Channel, n= 0.060 Length= 620.0' Slope= 0.0774 '/'		
Reach r03-1:		
Overland Flow Reach Requires more survey		
Inflow Area =       11.485 ac, Inflow Depth =       1.88" for 10-yr event         Inflow =       11.66 cfs @       12.61 hrs, Volume=       1.799 af         Outflow =       11.61 cfs @       12.65 hrs, Volume=       1.799 af, Atten= 0%, Lag= 2.7 min		
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 4.9 fps, Min. Travel Time= 2.6 min Avg. Velocity = 1.9 fps, Avg. Travel Time= 6.7 min		
Peak Depth= 0.57' @ 12.65 hrs Capacity at bank full= 92.14 cfs Inlet Invert= 845.00', Outlet Invert= 728.00' 10.00' x 1.50' deep Parabolic Channel, n= 0.060 Length= 785.0' Slope= 0.1490 '/'		

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# Reach r04-1:

Channel

Inflow Area = Inflow = Outflow =	26.658 ac, Inflow Depth = 2.18" 26.58 cfs @ 12.52 hrs, Volume= 26.56 cfs @ 12.52 hrs, Volume=	4.843 af	Atten= 0%, Lag= 0.5 min
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 7.1 fps, Min. Travel Time= 0.8 min Avg. Velocity = 2.0 fps, Avg. Travel Time= 2.8 min			
Peak Depth= 0.96' @ 12.52 hrs Capacity at bank full= 530.15 cfs Inlet Invert= 685.50', Outlet Invert= 632.00' 12.00' x 4.00' deep Parabolic Channel, n= 0.060 Length= 330.0' Slope= 0.1621 '/'			

#### Reach r08-1a:

Man Made Ditch Inverts of pipe to be surveyed

Inflow Area =	93.367 ac, Inflow Depth = 1.42"	for 10-yr event
Inflow =	34.13 cfs @ 12.87 hrs, Volume=	11.019 af
Outflow =	34.13 cfs @ 12.88 hrs, Volume=	11.019 af, Atten= 0%, Lag= 0.3 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 9.4 fps, Min. Travel Time= 0.4 min Avg. Velocity = 5.0 fps, Avg. Travel Time= 0.8 min

Peak Depth= 0.67' @ 12.88 hrs Capacity at bank full= 81.88 cfs Inlet Invert= 607.00', Outlet Invert= 587.00' 10.00' x 1.00' deep Parabolic Channel, n= 0.027 Length= 226.0' Slope= 0.0885 '/'

#### Reach r08-1b:

24" HDPE Inverts to be surveyed

Inflow Area = 93.367 ac, Inflow Depth = 1.42" for 10-yr event Inflow = 34.13 cfs @ 12.88 hrs, Volume= 11.019 af Outflow = 34.13 cfs @ 12.88 hrs, Volume= 11.019 af, Atten= 0%, Lag= 0.1 min Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 23.8 fps, Min. Travel Time= 0.2 min Avg. Velocity = 13.1 fps, Avg. Travel Time= 0.4 min

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Peak Depth= 0.93' @ 12.88 hrs Capacity at bank full= 77.17 cfs Inlet Invert= 587.00', Outlet Invert= 557.75' 24.0" Diameter Pipe n= 0.012 Length= 295.0' Slope= 0.0992 '/'

# Reach r08-1c:

Ditch Pipe inverts to be surveyed

Inflow Area =		93.367 ac, Inflow Dept	h = 1.42"	for 10-yr event	
Inflow	=	34.13 cfs @ 12.88 hrs	, Volume=	11.019 af	
Outflow	=	34.13 cfs @ 12.89 hrs	, Volume=	11.019 af,	Atten= 0%, Lag= 0.6 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 9.0 fps, Min. Travel Time= 1.1 min Avg. Velocity = 4.5 fps, Avg. Travel Time= 2.2 min

Peak Depth= 0.69' @ 12.89 hrs Capacity at bank full= 76.65 cfs Inlet Invert= 557.75', Outlet Invert= 512.00' 10.00' x 1.00' deep Parabolic Channel, n= 0.027 Length= 590.0' Slope= 0.0775 '/'

## Reach r08-1d: Amenia Creek/Cascade Brook

(Steam) parallel to Route 22 Base Flow estimated from field observation (see field note 21)

Inflow Area = 114.957 ac, Inflow Depth = 18.19" for 10-yr event Inflow = 85.81 cfs @ 12.62 hrs, Volume= 174.279 af, Incl. 40.00 cfs Base Flow Outflow = 85.72 cfs @ 12.68 hrs, Volume= 174.034 af, Atten= 0%, Lag= 3.6 min Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 3.7 fps, Min. Travel Time= 3.6 min Avg. Velocity = 3.1 fps, Avg. Travel Time= 4.3 min

Peak Depth= 3.63' @ 12.68 hrs Capacity at bank full= 104.49 cfs Inlet Invert= 512.00', Outlet Invert= 504.00' 10.00' x 4.00' deep Parabolic Channel, n= 0.060 Length= 805.0' Slope= 0.0099 '/'

# Reach r13-1:

 Inflow Area =
 2.176 ac, Inflow Depth =
 2.54" for 10-yr event

 Inflow =
 4.78 cfs @
 12.22 hrs, Volume=
 0.460 af

 Outflow =
 4.73 cfs @
 12.24 hrs, Volume=
 0.460 af, Atten= 1%, Lag= 1.3 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 8.5 fps, Min. Travel Time= 1.8 min Avg. Velocity = 3.2 fps, Avg. Travel Time= 4.7 min

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Peak Depth= 0.53' @ 12.24 hrs Capacity at bank full= 17.79 cfs Inlet Invert= 546.00', Outlet Invert= 524.00' 18.0" Diameter Pipe n= 0.012 Length= 900.0' Slope= 0.0244 '/'

# Reach r14-3a:

30" HDPE Under Main Entrance Road

Inflow Area = Inflow = Outflow =	10.01 cfs @	nflow Depth = 1.6 12.17 hrs, Volum 12.17 hrs, Volum	= 0	0.880 af	Atten= 0%,	Lag= 0.4 min	
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 12.6 fps, Min. Travel Time= 0.6 min Avg. Velocity = 4.9 fps, Avg. Travel Time= 1.5 min							

Peak Depth= 0.55' @ 12.17 hrs Capacity at bank full= 94.91 cfs Inlet Invert= 526.00', Outlet Invert= 505.70' 30.0" Diameter Pipe n= 0.012 Length= 445.0' Slope= 0.0456 '/'

## Reach r14-3b:

Grass lined channel

Inflow Area = Inflow = Outflow =	6.422 ac, Inflow Depth = 1.65" 10.08 cfs @ 12.15 hrs, Volume= 10.01 cfs @ 12.17 hrs, Volume=	0.880 af				
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 6.0 fps, Min. Travel Time= 1.0 min Avg. Velocity = 2.3 fps, Avg. Travel Time= 2.6 min						
Peak Depth= 0.58' @ 12.17 hrs Capacity at bank full= 325.42 cfs						

Inlet Invert= 542.00', Outlet Invert= 526.00' 10.00' x 3.00' deep Parabolic Channel, n= 0.027 Length= 360.0' Slope= 0.0444 '/'

# Reach r17-1:

Inflow Are	a =	76.086 ac, 1	nflow Depth	i = 1.73"	for 10-yr even	t
Inflow	=	42.48 cfs @	13.30 hrs,	Volume=	10.954 af	
Outflow	=	42.38 cfs @	13.39 hrs,	Volume=	10.954 af	, Atten= 0%, Lag= 5.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 7.4 fps, Min. Travel Time= 3.2 min Avg. Velocity = 3.0 fps, Avg. Travel Time= 7.7 min

Proposed Conditions_10454-01 Prepared by The Chazen Companies HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Sys	Type III 24-hr 10-yr         Rainfall=5.00"           Page 129           stems         4/10/2006         3:15:25 PM								
Peak Depth= 1.01' @ 13.39 hrs Capacity at bank full= 181.28 cfs Inlet Invert= 646.00', Outlet Invert= 524.00' 12.00' x 2.00' deep Parabolic Channel, n= 0.045 Length= 1,390	Capacity at bank full= 181.28 cfs								
Reach r18-2:									
Overland Flow Reach									
Inflow         =         0.00 cfs @         0.00 hrs, Volume=         0.000           Outflow         =         0.00 cfs @         0.00 hrs, Volume=         0.000	af af, Atten= 0%, Lag= 0.0 min								
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0 Max. Velocity= 0.0 fps, Min. Travel Time= 0.0 min Avg. Velocity = 0.0 fps, Avg. Travel Time= 0.0 min	).01 hrs								
Peak Depth= 0.00' @ 0.00 hrs Capacity at bank full= 434.91 cfs Inlet Invert= 973.60', Outlet Invert= 630.00' 50.00' x 1.00' deep Parabolic Channel, n= 0.060 Length= 720.0	)' Slope= 0.4772 '/'								
Reach r21-1a:									
Man Made Ditch									
Inflow Area =         207.817 ac, Inflow Depth =         1.16"         for 10-yr ev           Inflow =         15.24 cfs @         18.09 hrs, Volume=         20.073           Outflow =         15.24 cfs @         18.12 hrs, Volume=         20.059									
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0 Max. Velocity= 4.6 fps, Min. Travel Time= 2.3 min Avg. Velocity = 2.9 fps, Avg. Travel Time= 3.8 min	).01 hrs								
Peak Depth= 0.90' @ 18.12 hrs Capacity at bank full= 191.76 cfs Inlet Invert= 504.00', Outlet Invert= 494.00' 10.00' x 3.00' deep Parabolic Channel, n= 0.027 Length= 648.0	0' Slope= 0.0154 '/'								
Reach r21-1b:									
Overland Flow Reach									
Inflow Area =         29.123 ac, Inflow Depth =         1.90" for 10-yr ev           Inflow =         37.19 cfs @         12.55 hrs, Volume=         4.619           Outflow =         37.17 cfs @         12.56 hrs, Volume=         4.619									

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 3.9 fps, Min. Travel Time= 0.7 min Avg. Velocity = 1.5 fps, Avg. Travel Time= 1.8 min Proposed Conditions\_10454-01Type III 24-hr 10-yr Rainfall=5.00"Prepared by The Chazen CompaniesPage 130HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems4/10/2006 3:15:25 PM

Peak Depth= 0.43' @ 12.56 hrs Capacity at bank full= 227.81 cfs Inlet Invert= 506.70', Outlet Invert= 485.75' 50.00' x 1.00' deep Parabolic Channel, n= 0.060 Length= 160.0' Slope= 0.1309 '/'

#### Reach r22-2:

**Overland Flow Reach** 

Inflow Area = Inflow = Outflow =	15.520 ac, Inflow Depth =1.30"for10-yr event5.85 cfs @13.10 hrs, Volume=1.681 af5.84 cfs @13.14 hrs, Volume=1.681 af, Atten= 0%, Lag= 2.1 min						
Max. Velocity= 3.	Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 3.7 fps, Min. Travel Time= 2.9 min Avg. Velocity = 1.3 fps, Avg. Travel Time= 8.2 min						

#### Reach r25-0a:

Ditch Pipe inverts need to be surveyed

Inflow Area = Inflow = Outflow =	67.391 ac, Inflow Depth = 1.93" 33.69 cfs @ 12.53 hrs, Volume= 33.68 cfs @ 12.56 hrs, Volume=	10.850 af	Atten= 0%, Lag= 1.6 min				
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 9.0 fps, Min. Travel Time= 2.0 min Avg. Velocity = 3.2 fps, Avg. Travel Time= 5.7 min							

Peak Depth= 0.85' @ 12.56 hrs Capacity at bank full= 205.50 cfs Inlet Invert= 570.00', Outlet Invert= 504.00' 10.00' x 2.00' deep Parabolic Channel, n= 0.027 Length= 1,090.0' Slope= 0.0606 '/'

# Reach r25-0b: Wetland Reach

Wetland Reach Has wetland vegetation within reach

Inflow Area =	9.435 ac, Inflow Depth = 1.56"	for 10-yr event
Inflow =	8.28 cfs @ 12.43 hrs, Volume=	1.229 af
Outflow =	7.38 cfs @ 12.56 hrs, Volume=	1.228 af, Atten= 11%, Lag= 7.7 min

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Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 1.6 fps, Min. Travel Time= 8.0 min Avg. Velocity = 0.4 fps, Avg. Travel Time= 29.5 min

Peak Depth= 0.72' @ 12.56 hrs Capacity at bank full= 156.51 cfs Inlet Invert= 504.00', Outlet Invert= 499.50' 20.00' x 3.00' deep Parabolic Channel, n= 0.045 Length= 750.0' Slope= 0.0060 '/'

## Reach r25-0c: Amenia Creek/Cascade Brook

(Steam) parallel to Route 22 Base Flow estimated from field observation (see field note 21)

138.083 ac, Inflow Depth = 29.13" for 10-yr event Inflow Area = Inflow 141.71 cfs @ 12.52 hrs, Volume= 335.252 af, Incl. 40.00 cfs Base Flow = 140.23 cfs @ 12.62 hrs, Volume= Outflow 334.568 af, Atten= 1%, Lag= 6.3 min = Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 2.9 fps, Min. Travel Time= 5.6 min Avg. Velocity = 2.7 fps, Avg. Travel Time= 6.1 min Peak Depth= 6.16' @ 12.62 hrs Capacity at bank full= 67.14 cfs Inlet Invert= 504.00', Outlet Invert= 500.00' 10.00' x 4.00' deep Parabolic Channel, n= 0.060 Length= 975.0' Slope= 0.0041 '/' Pond 8P:

No field note. Water spills over cart path; no storage.

Inflow Are	a =	41.049 ac, Inflow Depth = 1.99" for 10-yr event	
Inflow	=	31.62 cfs @ 12.43 hrs, Volume= 6.806 af	
Outflow	=	31.62 cfs @ 12.43 hrs, Volume= 6.806 af, Atten= 0%, Lag= 0.0 min	n
Primary	=	31.62 cfs @ 12.43 hrs, Volume= 6.806 af	
	_		

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 575.35' @ 12.43 hrs Flood Elev= 574.70' Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= (not calculated)

#	Routing	Invert	Outlet Devices
1	Primary	574.70'	177.0 deg Sharp-Crested Vee/Trap Weir C= 2.46

Primary OutFlow Max=31.62 cfs @ 12.43 hrs HW=575.35' TW=570.85' (Dynamic Tailwater) ←1=Sharp-Crested Vee/Trap Weir (Weir Controls 31.62 cfs @ 2.0 fps)

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# Pond p02-2:

Proposed culvert under proposed road at intersection with 44.

Inflow Inflow Outfle Prima	ow =	8.59 cfs 8.59 cfs	c, Inflow Depth = 1.73" for 10-yr event @ 12.44 hrs, Volume= 1.120 af @ 12.44 hrs, Volume= 1.120 af, Atten= 0%, Lag= 0.0 min @ 12.44 hrs, Volume= 1.120 af				
Peak Flood Plug-	Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 641.32' @ 12.44 hrs Flood Elev= 645.00' Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= (not calculated)						
#	Routing	Invert	Outlet Devices				
1	Primary	640.00'	<b>24.0"</b> x <b>100.0' long Culvert</b> CPP, end-section conforming to fill, Ke= 0.500 Outlet Invert= 638.00' S= 0.0200 '/' n= 0.012 Cc= 0.900				

Primary OutFlow Max=8.59 cfs @ 12.44 hrs HW=641.32' TW=625.16' (Dynamic Tailwater) ↓ 1=Culvert (Inlet Controls 8.59 cfs @ 3.9 fps)

## Pond p02-3:

Simulates last DMH at bottom of small road, at intersection with 44. This culvert is only used to size the drain pipe under 44.

Inflow Inflow Outflo Prima	= w	19.21 cfs 19.21 cfs	c, Inflow Depth = 3.47" for 10-yr event @ 12.02 hrs, Volume= 1.181 af @ 12.02 hrs, Volume= 1.181 af, Atten= 0%, Lag= 0.0 min @ 12.02 hrs, Volume= 1.181 af				
Peak Flood Plug-f Cente	Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 638.59' @ 12.02 hrs Flood Elev= 645.00' Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= (not calculated)						
#	Routing	Invert	Outlet Devices				
1	Primary	635.00'	24.0" x 100.0' long Culvert CPP, projecting, no headwall, Ke= 0.900				

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		Outlet Invert= 634.00'	S= 0.0100 '/'	n= 0.012	Cc= 0.900	

**Primary OutFlow** Max=19.10 cfs @ 12.02 hrs HW=638.56' TW=554.10' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 19.10 cfs @ 6.1 fps)

#### Pond p03-2:

Inflow Area	ι =	4.738 ac, 1	nflow Depth = 3.24"	for 10-yr event	
Inflow	=	19.64 cfs @	12.02 hrs, Volume=	1.278 af	
Outflow	=	2.69 cfs @	12.50 hrs, Volume=	1.274 af,	Atten= 86%, Lag= 28.5 min
Primary	=	2.69 cfs @	12.50 hrs, Volume=	1.274 af	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 774.00' Surf.Area= 2,315 sf Storage= 4,095 cf Peak Elev= 777.84' @ 12.50 hrs Surf.Area= 8,765 sf Storage= 30,160 cf (26,065 cf above start) Flood Elev= 779.00' Surf.Area= 9,991 sf Storage= 41,391 cf (37,296 cf above start) Plug-Flow detention time= 387.9 min calculated for 1.180 af (92% of inflow) Center-of-Mass det. time= 305.5 min (1,090.6 - 785.1)

#	Invert	Avail.St	orage	Storage Des	cription		
1	768.00'	51,	363 cf	Custom Stag	ge Data (Conic) L	isted below	
Elevat (fe	ion et)	Surf.Area (sq-ft)	(0	Inc.Store cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
768.	.00	67		0	0	67	
770.	.00	345		376	376	361	
772	.00	729		1,050	1,426	777	
772	.50	842		392	1,819	901	
774.	.00	2,315		2,277	4,095	2,388	
774.	.50	5,704		1,942	6,037	5,779	
776.	.00	6,996		9,509	15,546	7,138	
778.	.00	8,917		15,874	31,420	9,160	
780.	.00	11,064		19,942	51,363	11,421	
# F	Routing	Invert	Outlet	Devices			
1 F	Primary	774.00'	3.0" Ve	ert. Orifice/Gr	ate C= 0.600		

2 Primary 776.20' **6.0" Vert. Orifice/Grate X 2.00** C= 0.600

3 Primary 778.50' 4.0' long x 6.0' high Sharp-Crested Rectangular Weir 2 End Contraction(s)

Primary OutFlow Max=2.69 cfs @ 12.50 hrs HW=777.84' TW=722.06' (Dynamic Tailwater)

-1=Orifice/Grate (Orifice Controls 0.46 cfs @ 9.3 fps)

-2=Orifice/Grate (Orifice Controls 2.23 cfs @ 5.7 fps)

-3=Sharp-Crested Rectangular Weir (Controls 0.00 cfs)

#### Pond p04-1:

Storage, inverts and culvert length based on assumed grading, check when final grading becomes available

Inflow Are	a =	34.207 ac, Inflow Depth = 2.11"	for 10-yr event
Inflow	=	31.17 cfs @ 12.46 hrs, Volume=	6.026 af
Outflow	=	27.68 cfs @ 12.65 hrs, Volume=	6.026 af, Atten= 11%, Lag= 12.0 min
Primary	=	27.68 cfs @ 12.65 hrs, Volume=	6.026 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

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Peak Elev= 642.35' @ 12.65 hrs Surf.Area= 6,369 sf Storage= 10,078 cf Flood Elev= 648.00' Surf.Area= 15,680 sf Storage= 66,062 cf Plug-Flow detention time= 3.0 min calculated for 6.024 af (100% of inflow) Center-of-Mass det. time= 2.9 min (923.4 - 920.4)

#	Invert	Avail.Ste	orage Storage De	escription		
1	638.00'	66,0	062 cf Custom St	age Data (Conic)	Listed below	
	ation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
63	38.00	0	0	0	0	
64	40.00	1,300	867	867	1,306	
64	42.00	6,180	6,876	7,743	6,203	
64	14.00	7,270	13,435	21,178	7,438	
64	46.00	11,100	18,235	39,414	11,327	
64	48.00	15,680	26,648	66,062	15,980	
#	Routing	Invert	Outlet Devices			
1	Primary	638.00'		g Culvert CPP, e		rming to fill, Ke= 0.500

Outlet Invert= 598.00' S= 0.0584 '/' n= 0.012 Cc= 0.900

Primary OutFlow Max=27.68 cfs @ 12.65 hrs HW=642.35' TW=575.34' (Dynamic Tailwater) ←1=Culvert (Inlet Controls 27.68 cfs @ 8.8 fps)

#### Pond p06-0:

Field Note #22 Geometry to be confirmed by survey.

Inflow Area =	9.435 ac, Inflow Depth = $1.60$ "	for 10-yr event
Inflow =	10.64 cfs @ 12.25 hrs, Volume=	1.258 af
Outflow =	8.28 cfs @ 12.43 hrs, Volume=	1.229 af, Atten= 22%, Lag= 10.5 min
Primary =	8.28 cfs @ 12.43 hrs, Volume=	1.229 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 506.80' Surf.Area= 18,600 sf Storage= 42,160 cf Peak Elev= 507.40' @ 12.43 hrs Surf.Area= 21,338 sf Storage= 55,020 cf (12,860 cf above start) Flood Elev= 507.10' Surf.Area= 19,958 sf Storage= 48,537 cf (6,377 cf above start) Plug-Flow detention time= 875.3 min calculated for 0.262 af (21% of inflow) Center-of-Mass det. time= 143.4 min (1,000.6 - 857.2)

#	Invert	ert Avail.Stora	age Storage Des	cription		
1	500.00'	00' 67,669	9 cf Custom Sta	<b>ge Data (Conic)</b> Li	sted below	
Elevat (fe	tion eet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
500 506 508	.80	0 18,600 24,030	0 42,160 25,509	0 42,160 67,669	0 18,672 24,138	

110		unions_		+ III IO yi Kaiiliaii–0.00
Prep	ared by The	e Chazen	Companies	Page 135
Hydro	DCAD® 7.00 s	s/n 000927	© 1986-2003 Applied Microcomputer Systems	4/10/2006 3:15:26 PM
#	Routing	Invert	Outlet Devices	
1	Primary	506.80'	12.0" x 20.0' long Culvert CMP, projecting, no he	adwall, Ke= 0.900
	-		Outlet Invert= 506.00' S= 0.0400 '/' n= 0.024 Co	= 0.900
2	Primary	507.10'	178.0 deg Sharp-Crested Vee/Trap Weir C= 2.46	
Prim	ary OutFlow	May-8 2	R cfs @ 12.43 hrs HW = 507.40' TW = 504.66' (Dyna	amic Tailwater)

Primary OutFlow Max=8.28 cfs @ 12.43 hrs HW=507.40' TW=504.66' (Dynamic Tailwater) -1=Culvert (Inlet Controls 1.04 cfs @ 2.1 fps) -2=Sharp-Crested Vee/Trap Weir (Weir Controls 7.24 cfs @ 1.4 fps)

#### Pond p07-1:

Field Note # 29 Outlet geometry to be confirmed by survey.

Inflow Area =	=	26.342 ac, Inflow Depth = 1.91"	for 10-yr event	
Inflow =		8.68 cfs @ 12.14 hrs, Volume=	4.186 af	
Outflow =		3.92 cfs @ 13.44 hrs, Volume=	4.044 af,	Atten= 55%, Lag= 78.1 min
Primary =		3.92 cfs @ 13.44 hrs, Volume=	4.044 af	-

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 572.80' Surf.Area= 21,640 sf Storage= 56,264 cf Peak Elev= 573.64' @ 13.44 hrs Surf.Area= 25,600 sf Storage= 76,792 cf (20,528 cf above start) Flood Elev= 573.50' Surf.Area= 24,936 sf Storage= 73,351 cf (17,087 cf above start) Plug-Flow detention time= 562.2 min calculated for 2.752 af (66% of inflow) Center-of-Mass det. time= 95.5 min (1,285.1 - 1,189.6)

#	Invert	Avail.St	orage	Storage Description			
1	565.00'	85,	557 cf	Custom Sta	<b>ge Data (Conic)</b> L	isted below	
Eleva (†	ation feet)	Surf.Area (sq-ft)	-	nc.Store ibic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
56	5.00	0	•	0	0	0	
57	2.80	21,640		56,264	56,264	21,735	
57	4.00	27,290		29,293	85,557	27,424	
#	Routing	Invert	Outlet D	evices			
1	Primary	572.80'	<b>18.0" x 20.0' long Culvert</b> CMP, projecting, no headwall, Ke= 0.900				
2	Primary	573.50'	Outlet Invert= 572.00' S= 0.0400 '/' n= 0.024 Cc= 0.900 177.0 deg Sharp-Crested Vee/Trap Weir X 2.00 C= 2.46				

Primary OutFlow Max=3.92 cfs @ 13.44 hrs HW=573.64' TW=570.61' (Dynamic Tailwater) -1=Culvert (Inlet Controls 2.51 cfs @ 2.5 fps)

-2=Sharp-Crested Vee/Trap Weir (Weir Controls 1.40 cfs @ 0.9 fps)

#### Pond p08-2:

Inflow Are	a =	18.762 ac, Inflow Depth = 2.93"	" for 10-yr event	
Inflow	=	55.49 cfs @ 12.04 hrs, Volume=	= 4.580 af	
Outflow	=	9.93 cfs @ 12.58 hrs, Volume=	= 3.908 af, Atten= 82%, Lag= 32.3 m	nin
Primary	=	9.93 cfs @ 12.58 hrs, Volume=	= 3.908 af	

Proposed Conditions_10454-01	Type III 24-hr 10-yr Rainfall=5.00"
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Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 550.00' Surf.Area= 8,558 sf Storage= 24,834 cf Peak Elev= 556.04' @ 12.58 hrs Surf.Area= 21,631 sf Storage= 125,136 cf (100,302 cf above start) Flood Elev= 557.00' Surf.Area= 23,344 sf Storage= 147,597 cf (122,763 cf above start) Plug-Flow detention time= 544.0 min calculated for 3.337 af (73% of inflow) Center-of-Mass det. time= 370.3 min (1,160.3 - 790.0)

#	Invert	Avail.Storag	ge Storage Des	cription		
1	544.00'	170,918	cf Custom Sta	<b>ge Data (Conic)</b> Li	sted below	
Elevat (fe	ion eet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
544		1,962	0	0	1,962	
546	.00	3,155	5,070	5,070	3,207	
548		4,454	7,572	12,642	4,577	
548	.50	4,796	2,312	14,954	4,940	
550	.00	8,558	9,880	24,834	8,726	
550	.50	12,948	5,339	30,173	13,120	
552	.00	15,129	21,037	51,209	15,390	
554	.00	18,234	33,315	84,524	18,627	
556	.00	21,565	39,752	124,277	22,105	
558	.00	25,122	46,642	170,918	25,823	

#	Routing	Invert	Outlet Devices
1	Primary	550.00'	3.0" Vert. Orifice/Grate C= 0.600
2	Primary	554.09'	12.0" Vert. Orifice/Grate X 2.00 C= 0.600
3	Primary	556.00'	11.0' long x 6.0' high Sharp-Crested Rectangular Weir

2 End Contraction(s)

Primary OutFlow Max=9.93 cfs @ 12.58 hrs HW=556.04' TW=515.62' (Dynamic Tailwater) -1=Orifice/Grate (Orifice Controls 0.57 cfs @ 11.7 fps) -2=Orifice/Grate (Orifice Controls 9.10 cfs @ 5.8 fps)

-3=Sharp-Crested Rectangular Weir (Weir Controls 0.25 cfs @ 0.6 fps)

#### Pond p08-3:

Inflow Area =	2.828 ac, Inflow Depth = 2.73"	for 10-yr event	
Inflow =	7.20 cfs @ 12.02 hrs, Volume=	0.643 af	
Outflow =	2.90 cfs @ 12.36 hrs, Volume=	0.642 af, Atten= 60%, Lag= 20.9 mir	۱
Primary =	2.90 cfs @ 12.36 hrs, Volume=	0.642 af	
Primary =	2.90 cfs @ 12.36 hrs, Volume=	0.642 af	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 528.00' Surf.Area= 1,849 sf Storage= 2,615 cf Peak Elev= 530.93' @ 12.36 hrs Surf.Area= 4,227 sf Storage= 11,701 cf (9,086 cf above start) Flood Elev= 533.00' Surf.Area= 6,389 sf Storage= 22,602 cf (19,987 cf above start) Plug-Flow detention time= 244.6 min calculated for 0.582 af (91% of inflow) Center-of-Mass det. time= 152.5 min (936.3 - 783.8)

# Proposed Conditions\_10454-01

Type III 2	24-hr 10-yr Rainfall=5.00"
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#	Invert	Avail.St	orage S	Storage Des	scription		
1	524.00'	28,	956 cf 🛛 🕻	Custom Sta	age Data (Conic)	Listed below	
Elevat	tion eet)	Surf.Area (sq-ft)		nc.Store lbic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
524	1.00	178		0	0	178	
526	6.00	500		651	651	524	
526	6.50	548		262	913	587	
528	3.00	1,849		1,702	2,615	1,900	
530	0.00	3,344		5,120	7,734	3,437	
532	2.00	5,240		8,513	16,248	5,388	
534	1.00	7,538		12,709	28,956	7,755	
1	Routing Primary Primary	Invert 528.00' 530.00'		t. Orifice/G	rate C= 0.600 Grate C= 0.600		

Primary OutFlow Max=2.90 cfs @ 12.36 hrs HW=530.93' TW=515.39' (Dynamic Tailwater) -1=Orifice/Grate (Orifice Controls 0.40 cfs @ 8.1 fps) -2=Orifice/Grate (Orifice Controls 2.50 cfs @ 3.3 fps)

## Pond p09-2:

Inflow Area	a =	21.180 ac, Inflow Depth = $2.17$ "	for 10-yr event
Inflow	=	29.47 cfs @ 12.29 hrs, Volume=	3.821 af
Outflow	=	3.02 cfs @ 14.90 hrs, Volume=	3.362 af, Atten= 90%, Lag= 156.8 min
Primary	=	3.02 cfs @ 14.90 hrs, Volume=	3.362 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 586.00' Surf.Area= 10,285 sf Storage= 36,340 cf Peak Elev= 590.77' @ 14.90 hrs Surf.Area= 23,669 sf Storage= 130,022 cf (93,682 cf above start) Flood Elev= 593.00' Surf.Area= 27,610 sf Storage= 187,200 cf (150,860 cf above start) Plug-Flow detention time= 705.1 min calculated for 2.528 af (66% of inflow) Center-of-Mass det. time= 442.9 min (1,277.9 - 835.0)

#	Invert	Avail.Storag	e Storage Des	scription		
1	580.00'	214,790 c	of Custom Sta	ge Data (Conic) Li	sted below	
Elevat (fe	ion et)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
580	.00	3,968	0	0	3,968	
582	.00	5,102	9,046	9,046	5,198	
584	.00	6,343	11,423	20,469	6,550	
584	.50	6,670	3,253	23,722	6,907	
586	.00	10,285	12,619	36,340	10,554	
586	.50	16,887	6,725	43,066	17,159	
588	.00	19,143	27,005	70,070	19,525	
590	.00	22,349	41,451	111,521	22,890	
592	.00	25,781	48,089	159,610	26,494	
594	.00	29,439	55,180	214,790	30,336	

Proposed Conditions\_10454-01Type III 24-hr 10-yrRainfall=5.00"Prepared by The Chazen CompaniesPage 138HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems4/10/2006 3:15:26 PM

#	Routing	Invert	Outlet Devices
1	Primary	586.00'	3.0" Vert. Orifice/Grate C= 0.600
2	Primary	588.21'	8.0" Vert. Orifice/Grate C= 0.600

3 Primary 592.00' 2.0' long x 6.0' high Sharp-Crested Rectangular Weir 2 End Contraction(s)

Primary OutFlow Max=3.02 cfs @ 14.90 hrs HW=590.77' TW=573.63' (Dynamic Tailwater)

-1=Orifice/Grate (Orifice Controls 0.51 cfs @ 10.4 fps)

-2=Orifice/Grate (Orifice Controls 2.51 cfs @ 7.2 fps)

-3=Sharp-Crested Rectangular Weir (Controls 0.00 cfs)

#### Pond p10:

Field Note #25 Need to get full story on how this pond works

Inflow Area = Inflow = Outflow =	59.531 ac, Inflow Depth =1.20"for10-yr event11.29 cfs @12.46 hrs, Volume=5.973 af0.00 cfs @0.00 hrs, Volume=0.000 af, Atten=100%, Lag=
Starting Elev= 49 Peak Elev= 503.5 Plug-Flow detent	Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs 08.40' Surf.Area= 36,110 sf Storage= 101,108 cf 51' @ 48.00 hrs Surf.Area= 72,902 sf Storage= 361,278 cf (260,170 cf above start) cion time= (not calculated) det. time= (not calculated)

#	Invert	Avail.Stora	ge Storage Des	cription		
1 4	90.00'	581,029	cf Custom Stag	ge Data (Conic) Li	sted below	
Elevation (feet)	-	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
490.00 498.40		0 36,110	0 101,108	0 101,108	0 36,221	
500.00 502.00	)	42,400 54,880	62,741 97,012	163,849 260,861	42,610 55,187	
504.00 506.00		78,730 109,382	132,895 187,274	393,755 581,029	79,107 109,836	

#### Pond p13-1:

No Field Note Natural depression.

Inflow Area	a =	12.222 ac, Inflow Depth = 3.38"	for 10-yr event	
Inflow	=	43.29 cfs @ 12.04 hrs, Volume=	3.444 af	
Outflow	=	38.66 cfs @ 12.08 hrs, Volume=	3.429 af, At	tten= 11%, Lag= 2.1 min
Primary	=	38.66 cfs @ 12.08 hrs, Volume=	3.429 af	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 524.00' Surf.Area= 5,894 sf Storage= 16,480 cf Peak Elev= 526.71' @ 12.08 hrs Surf.Area= 9,629 sf Storage= 37,983 cf (21,503 cf above start) Flood Elev= 527.00' Surf.Area= 10,067 sf Storage= 40,862 cf (24,383 cf above start) Plug-Flow detention time= 227.9 min calculated for 3.050 af (89% of inflow) Center-of-Mass det. time= 134.2 min (906.8 - 772.6)

#	Invert	Invert Avail.Storage		Storage Description			
1	518.00'	50,891 cf		Custom Stage Data (Conic) Listed below			
Elevation (feet)		Surf.Area (sq-ft)		Inc.Store cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
518.00		1,331		0	0	1,331	
520.00		2,048		3,353	3,353	2,104	
522.00 2,9		2,912		4,935	8,288	3,037	
522.50		3,150		1,515	9,803	3,294	
524.00		5,894		6,676	16,480	6,061	
526.00		8,542		14,354	30,834	8,776	
528.00		11,592		20,057	50,891	11,908	
 1 2	Routing Primary Primary	Invert 524.00' 525.90'	3.0" V 15.0' k	ong x 1.3' hig	rate C= 0.600 h Sharp-Crested I	Rectangular Weir	
			2 End	Contraction(s	)		

**Primary OutFlow** Max=38.60 cfs @ 12.08 hrs HW=526.71' TW=500.68' (Dynamic Tailwater) -1=Orifice/Grate (Orifice Controls 0.38 cfs @ 7.7 fps)

-2=Sharp-Crested Rectangular Weir (Weir Controls 38.22 cfs @ 3.2 fps)

# Pond p14-1:

Field Note #26 Need to figure out how this pond works

Inflow Area = Inflow = Outflow = Primary =	50.663 ac, Inflow Depth = 2 111.48 cfs @ 12.08 hrs, Volu 8.78 cfs @ 14.58 hrs, Volu 8.78 cfs @ 14.58 hrs, Volu	Ime= 12.063 af Ime= 4.368 af, Atten= 92%, Lag= 149.6 min				
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 497.40' Surf.Area= 22,200 sf Storage= 54,760 cf Peak Elev= 503.51' @ 48.00 hrs Surf.Area= 81,453 sf Storage= 389,958 cf (335,198 cf above start) Plug-Flow detention time= 452.0 min calculated for 3.111 af (26% of inflow) Center-of-Mass det. time= 155.9 min (1,032.2 - 876.3)						

 4	Invert	Avail.Storage	Storage Description
1	490.00'	805,062 cf	Custom Stage Data (Conic) Listed below

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Elevation	Surf.Area	Inc.Store	Cum.Store	Wet.Area
(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)	(sq-ft)
490.00	0	0	0	0
497.40	22,200	54,760	54,760	22,286
498.00	25,330	14,249	69,009	25,433
500.00	52,810	76,476	145,485	52,948
502.00	73,360	125,608	271,093	73,574
504.00	84,070	157,308	428,402	84,467
506.00	92,130	176,139	604,540	92,797
	,	,	,	,

# Routing Invert Outlet Devices

1 Primary 500.00' **24.0" x 80.0' long Culvert** CPP, end-section conforming to fill, Ke= 0.500 Outlet Invert= 502.00' S= -0.0250 '/' n= 0.012 Cc= 0.900

Primary OutFlow Max=8.77 cfs @ 14.58 hrs HW=503.34' TW=500.81' (Dynamic Tailwater) ←1=Culvert (Inlet Controls 8.77 cfs @ 3.9 fps)

#### Pond p14-2:

Inflow Area	a =	15.934 ac, Inflow Depth = 3.46"	for 10-yr event	
Inflow	=	56.94 cfs @ 12.05 hrs, Volume=	4.597 af	
Outflow	=	50.62 cfs @ 12.09 hrs, Volume=	4.569 af,	Atten= 11%, Lag= 2.6 min
Primary	=	50.62 cfs @ 12.09 hrs, Volume=	4.569 af	-

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 532.00' Surf.Area= 7,681 sf Storage= 23,903 cf Peak Elev= 534.62' @ 12.09 hrs Surf.Area= 11,740 sf Storage= 49,830 cf (25,926 cf above start) Flood Elev= 535.00' Surf.Area= 12,390 sf Storage= 54,538 cf (30,635 cf above start) Plug-Flow detention time= 212.2 min calculated for 4.020 af (87% of inflow) Center-of-Mass det. time= 114.1 min (886.3 - 772.2)

#	Invert	Avail.St	torage Storage D	escription		
1	526.00'	66,	889 cf Custom S	Stage Data (Conic)	Listed below	
-	ation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
52	26.00	2,239	0	0	2,239	
52	28.00	3,156	5,369	5,369	3,227	
53	30.00	4,207	7,338	12,707	4,362	
53	80.50	4,491	2,174	14,881	4,669	
53	32.00	7,681	9,023	23,903	7,885	
53	84.00	10,686	18,285	42,188	10,966	
53	36.00	14,093	24,701	66,889	14,463	
	Routing	Invert	Outlet Devices			
1 2	Primary Primary	532.00' 533.60'	3.0" Vert. Orifice/ 14.0' long x 1.5' h 2 End Contraction	igh Sharp-Crested	d Rectangular We	ir

Primary OutFlow Max=50.59 cfs @ 12.09 hrs HW=534.62' TW=500.78' (Dynamic Tailwater) -1=Orifice/Grate (Orifice Controls 0.37 cfs @ 7.6 fps) -2=Sharp-Crested Rectangular Weir (Weir Controls 50.22 cfs @ 3.6 fps)

# Pond p16-1:

Field Note # 49 Large pond with man-made island. Geometry to be verified by survey. In particular, we are making big guesses about the outlets. Also need to find out about valves...

Inflow Area =	176.893 ac, Inflow Depth = 1.86"	for 10-yr event
Inflow =	110.76 cfs @ 12.43 hrs, Volume=	27.372 af
Outflow =	13.02 cfs @ 18.15 hrs, Volume=	15.691 af, Atten= 88%, Lag= 343.1 min
Primary =	13.02 cfs @ 18.15 hrs, Volume=	15.691 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 507.00' Surf.Area= 199,799 sf Storage= 878,320 cf Peak Elev= 510.81' @ 18.15 hrs Surf.Area= 285,346 sf Storage= 1,714,330 cf (836,010 cf above start) Flood Elev= 510.50' Surf.Area= 271,550 sf Storage= 1,623,217 cf (744,897 cf above start) Plug-Flow detention time= (not calculated) Center-of-Mass det. time= (not calculated)

#	Invert	Avail.S	torage Storage	e Descript	ion		
1	500.00'	2,062,	087 cf Custor	n Stage D	ata (Conic)	Listed below	
Eleva	ation feet)	Surf.Area (sq-ft)			Cum.Store	Wet.Area (sq-ft)	
50	0.00	0		0	0	0	
50	3.00	140,344	140,34	4	140,344	140,358	
50	9.20	232,500	1,143,86	62	1,284,206	232,994	
51	0.00	249,400	192,72	20	1,476,927	249,951	
51	2.00	338,000	585,16	60	2,062,087	338,634	
#	Routing	Invert	Outlet Devices				
1	Primary	509.00'	18.0" x 110.0'	long Culv	vert CMP, p	projecting, no hea	adwall, Ke= 0.900
			Outlet Invert= 5	505.70' S	= 0.0300 '/'	n= 0.024 Cc=	0.900
2	Primary	500.00'				ation pipe w/ va	lve X 0.00
			CMP, projecting	g, no head	dwall, Ke= (	0.900	
						n= 0.013 Cc=	
3	Primary	510.50'	175.0 deg Sha	rp-Creste	d Vee/Trap	Weir X 2.00 C=	2.46

Primary OutFlow Max=13.02 cfs @ 18.15 hrs HW=510.81' TW=506.08' (Dynamic Tailwater) -1=Culvert (Inlet Controls 6.92 cfs @ 3.9 fps)

-2=assumed equalization pipe w/ valve (Controls 0.00 cfs)

-3=Sharp-Crested Vee/Trap Weir (Weir Controls 6.10 cfs @ 1.4 fps)

# Pond p17-1:

Field Note #45 Golf pond Geometry to be confirmed by surveyed

Inflow Area Inflow Outflow Primary	= =	112.240 ac, Inflow Depth =       1.72" for 10-yr event         52.43 cfs @       13.28 hrs, Volume=       16.129 af         52.43 cfs @       13.29 hrs, Volume=       16.129 af, Atten= 0%, Lag= 0.5 m         52.43 cfs @       13.29 hrs, Volume=       16.129 af	in

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 523.80' Surf.Area= 7,290 sf Storage= 9,234 cf Peak Elev= 525.45' @ 13.29 hrs Surf.Area= 11,029 sf Storage= 24,809 cf (15,575 cf above start) Flood Elev= 524.30' Surf.Area= 8,074 sf Storage= 13,623 cf (4,389 cf above start) Plug-Flow detention time= 22.8 min calculated for 15.917 af (99% of inflow) Center-of-Mass det. time= 11.7 min (936.5 - 924.8)

#	Invert	Avail.St	orage	Storage Des	scription		
1	520.00'	30,	224 cf	Custom Sta	age Data (Conic) L	isted below	
520 523 524	tion eet) 0.00 3.80 4.00 5.00	Surf.Area (sq-ft) 0 7,290 7,300 12,460	(0	Inc.Store cubic-feet) 0 9,234 1,459 19,531	Cum.Store (cubic-feet) 0 9,234 10,693 30,224	Wet.Area (sq-ft) 0 7,313 7,374 12,581	
#	Routing	Invert	Outlet	Devices			
2	Primary Primary Primary	523.80' 524.30' 525.20'	Head ( Coef. ( <b>143.0 c</b>	feet) 0.20 0 English) 2.80 <b>leg Sharp-C</b> i	adth Broad-Crest .40 0.60 0.80 1.0 0 2.92 3.08 3.30 rested Vee/Trap W ong Sharp-Creste	00 3.32 Veir C= 2.47	

Primary OutFlow Max=52.43 cfs @ 13.29 hrs HW=525.45' TW=515.91' (Dynamic Tailwater) =Broad-Crested Rectangular Weir (Weir Controls 15.42 cfs @ 4.3 fps)

-2=Sharp-Crested Vee/Trap Weir (Weir Controls 10.37 cfs @ 2.6 fps)

-3=Sharp-Crested Vee/Trap Weir (Weir Controls 26.65 cfs @ 1.5 fps)

# Pond p18-1:

Field Note #46 Golf pond Geometry to be confirmed by surveyed

Inflow Area =	131.862 ac, Inflow Depth = 1.75"	for 10-yr event
Inflow =	66.53 cfs @ 12.46 hrs, Volume=	19.185 af
Outflow =	65.48 cfs @ 12.54 hrs, Volume=	19.181 af, Atten= 2%, Lag= 4.6 min
Primary =	65.48 cfs @ 12.54 hrs, Volume=	19.181 af

Proposed Conditions_10454-01	Type III 24-hr 10-yr Rainfall=5.00"
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Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 513.90' Surf.Area= 20,680 sf Storage= 26,884 cf Peak Elev= 515.97' @ 12.54 hrs Surf.Area= 28,168 sf Storage= 76,953 cf (50,069 cf above start) Flood Elev= 514.81' Surf.Area= 23,768 sf Storage= 48,709 cf (21,825 cf above start) Plug-Flow detention time= 60.0 min calculated for 18.560 af (97% of inflow) Center-of-Mass det. time= 33.3 min (957.1 - 923.8)

#	Invert	Avail.Stora	ige Storage Des	cription		
1	510.00'	148,288	B cf Custom Sta	ge Data (Conic) Li	sted below	
Elevat	tion eet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
510	0.00	0	0	0	0	
513	8.90	20,680	26,884	26,884	20,704	
514	.00	20,690	2,068	28,952	20,756	
516	6.00	28,290	48,782	77,735	28,436	
518	3.00	42,760	70,554	148,288	42,967	
#	Routing	Invert Ou	utlet Devices			

$\pi$	Routing	mven	Odilet Devices
1	Primary	513.90'	2.0' long x 0.5' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00
			Coef. (English) 2.80 2.92 3.08 3.30 3.32
2	Primary	514.81'	143.0 deg Sharp-Crested Vee/Trap Weir C= 2.47
3	Primary	515.32'	175.0 deg x 10.0' long Sharp-Crested Vee/Trap Weir C= 2.46

Primary OutFlow Max=65.47 cfs @ 12.54 hrs HW=515.97' TW=508.20' (Dynamic Tailwater) -1=Broad-Crested Rectangular Weir (Weir Controls 19.75 cfs @ 4.8 fps) -2=Sharp-Crested Vee/Trap Weir (Weir Controls 10.65 cfs @ 2.7 fps)

-3=Sharp-Crested Vee/Trap Weir (Weir Controls 35.07 cfs @ 2.2 fps)

# Pond p19-0:

Wetland Geometry to be confirmed by survey Based off aerial topo, and assumed topo contour

Pond Unchanged from existing to proposed conditions

Inflow Area = Inflow = Outflow = Primary = Secondary =	10.40 cfs @ 1 5.85 cfs @ 1 5.85 cfs @ 1	flow Depth = 1.30 12.62 hrs, Volume 13.10 hrs, Volume 13.10 hrs, Volume 0.00 hrs, Volume	= 1.683 af = 1.681 af, = 1.681 af	Atten= 44%, Lag= 29.1 min
Starting Elev= 97 Peak Elev= 972.1 Plug-Flow detenti	2.00' Surf.Area I5' @ 13.10 hrs ion time= 607.9 r	Time Span= 0.00- a= 86,000 sf Stora Surf.Area= 90,212 min calculated for hin ( 981.7 - 907.3	ige= 57,333 cf 9 sf Storage= 74, 0.365 af (22% of in	507 cf (17,174 cf above start)

Hydro	0CAD® 7.00	00 s/n 000927 © 1986-2003 Applied Microcomputer Systems 4/10/2006 3:15:26 PM					
#_	Invert	Avail.S	U	Description			
1	970.00'	282,	329 cf Custom	Stage Data (Conic	) Listed below		
	ation (feet)	Surf.Area (sq-ft)			Wet.Area (sq-ft)		
97	0.00	0	C	) 0	0		
97	2.00	86,000	57,333	57,333	86,006		
97	74.00	141,270	224,996	5 282,329	141,327		
#	Routing	Invert	Outlet Devices				
1	Secondary	973.60'		0' long Sharp-Cres			
2	Primary	972.00'	Head (feet) 0.20	breadth Broad-Cr 0 0.40 0.60 0.80 2.80 2.92 3.08 3.3	1.00	r Weir	

Primary OutFlow Max=5.85 cfs @ 13.10 hrs HW=972.15' TW=970.13' (Dynamic Tailwater) ←2=Broad-Crested Rectangular Weir (Weir Controls 5.85 cfs @ 1.1 fps)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=972.00' TW=973.60' (Dynamic Tailwater) —1=Sharp-Crested Vee/Trap Weir (Controls 0.00 cfs)

#### Pond p20-1:

Field Note #50 Spring Fed Pond Geometry to be confirmed by surveyed

Inflow Area =	207.817 ac, Inflow Depth = 1.21"	for 10-yr event
Inflow =	24.59 cfs @ 12.30 hrs, Volume=	20.990 af
Outflow =	15.24 cfs @ 18.09 hrs, Volume=	20.073 af, Atten= 38%, Lag= 347.4 min
Primary =	15.24 cfs @ 18.09 hrs, Volume=	20.073 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 505.10' Surf.Area= 89,370 sf Storage= 138,524 cf Peak Elev= 506.08' @ 18.09 hrs Surf.Area= 89,789 sf Storage= 226,753 cf (88,230 cf above start) Plug-Flow detention time= 449.0 min calculated for 16.893 af (80% of inflow) Center-of-Mass det. time= 107.5 min (1,553.3 - 1,445.7)

#	Invert	Avail.Storag	e Storage De	scription	
1	502.00'	615,682 0	of Custom Sta	age Data (Prisma	tic) Listed below
Elevatio (fee		Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
502.0		0	0	0	
505.1	-	89,370	138,524	138,524	
506.0 508.0		89,380 99,280	80,437 188,660	218,961 407,621	
510.0		108,781	208,061	615,682	

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Type III 24-hr 10-yr Rainfall=5.00" Page 145 HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems 4/10/2006 3:15:26 PM

#	Routing	Invert	Outlet Devices
1	Primary	505.10'	3.0' long x 1.5' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00
			Coef. (English) 2.62 2.64 2.64 2.68 2.75 2.86 2.92 3.07 3.07 3.03 3.28
			3.32
2	Primary	506.20'	6.5' long x 1.5' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50
			3.00 Coef. (English) 2.62 2.64 2.64 2.68 2.75 2.86 2.92 3.07 3.07 3.03 3.28
			3.32
3	Primary	506.00'	176.0 deg x 97.0' long Sharp-Crested Vee/Trap Weir C= 2.46

Primary OutFlow Max=15.24 cfs @ 18.09 hrs HW=506.08' TW=504.90' (Dynamic Tailwater)

-1=Broad-Crested Rectangular Weir (Weir Controls 8.02 cfs @ 2.7 fps)

-2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

-3=Sharp-Crested Vee/Trap Weir (Weir Controls 7.22 cfs @ 0.9 fps)

# Pond p20-2:

Inflow Are	a =	13.511 ac, Inflow Depth = 3.32"	for 10-yr event
Inflow	=	45.11 cfs @ 12.08 hrs, Volume=	3.737 af
Outflow	=	1.72 cfs @ 15.80 hrs, Volume=	2.078 af, Atten= 96%, Lag= 222.7 min
Primary	=	1.72 cfs @ 15.80 hrs, Volume=	2.078 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 552.00' Surf.Area= 10,535 sf Storage= 35,913 cf Peak Elev= 558.35' @ 15.80 hrs Surf.Area= 24,470 sf Storage= 157,296 cf (121,383 cf above start) Flood Elev= 559.00' Surf.Area= 25,653 sf Storage= 174,016 cf (138,102 cf above start) Plug-Flow detention time= 1,381.8 min calculated for 1.254 af (34% of inflow) Center-of-Mass det. time= 798.8 min (1,584.8 - 786.0)

#	Invert	Avail.S	torage S	Storage De	scription		
1	546.00'	199,	647 cf 🛛 🕻	Custom Sta	age Data (Conic)	Listed below	
-		o ( )		0			
Elev	ation	Surf.Area		nc.Store	Cum.Store	Wet.Area	
(	(feet)	(sq-ft)	(cu	ibic-feet)	(cubic-feet)	<u>(sq-ft)</u>	
54	46.00	3,714		0	0	3,714	
54	18.00	4,960		8,644	8,644	5,044	
55	50.00	6,308		11,241	19,885	6,493	
55	50.50	6,661		3,242	23,127	6,874	
55	52.00	10,535		12,786	35,913	10,779	
55	52.50	15,037		6,360	42,273	15,285	
55	54.00	17,268		24,209	66,483	17,616	
55	56.00	20,441		37,664	104,147	20,935	
55	58.00	23,840		44,237	148,384	24,494	
56	60.00	27,465		51,262	199,647	28,292	
#	Routing	Invert	Outlet D	evices			
1	Primary	552.00'	3.0" Ver	t. Orifice/G	irate C= 0.600		
2	Primary	558.20'				Rectangular Weir	2 End Contraction(s)

Primary OutFlow Max=1.72 cfs @ 15.80 hrs HW=558.35' TW=506.03' (Dynamic Tailwater) -1=Orifice/Grate (Orifice Controls 0.59 cfs @ 12.0 fps) -2=Sharp-Crested Rectangular Weir (Weir Controls 1.13 cfs @ 1.3 fps)

#### Pond p21-1:

Inflow Area =	459.188 ac, Inflow Depth = 1.73"	for 10-yr event
Inflow =	283.00 cfs @ 12.39 hrs, Volume=	66.032 af
Outflow =	25.67 cfs @ 20.43 hrs, Volume=	60.140 af, Atten= 91%, Lag= 482.6 min
Primary =	25.67 cfs @ 20.43 hrs, Volume=	60.140 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 484.64' @ 20.43 hrs Surf.Area= 739,721 sf Storage= 1,321,913 cf Plug-Flow detention time= 667.4 min calculated for 60.127 af (91% of inflow) Center-of-Mass det. time= 546.0 min (1,656.8 - 1,110.7)

#	Invert	Avail.St	orage Storage D	escription		
1	480.40'	5,244,8	B85 cf Custom S	tage Data (Conic)	Listed below	
Eleva (fe	tion eet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
	).40 2.00	0 202,230	0 107,856	0 107,856	0 202,234	
	4.00	485,198 1,275,481	667,114	774,970	485,231	
	6.00 3.00	1,499,208	1,698,237 2,771,678	2,473,208 5,244,885	1,275,541 1,499,423	
#	Routing	Invert	Outlet Devices			
1	Primary	480.40'	30.0" x 70.0' long	Culvert CMP, p	rojecting, no head	wall, Ke= 0.900

1 Primary 480.40' **30.0" x 70.0' long Culvert** CMP, projecting, no headwall, Ke= 0.900 Outlet Invert= 480.40' S= 0.0000 '/' n= 0.024 Cc= 0.900

Primary OutFlow Max=25.67 cfs @ 20.43 hrs HW=484.64' TW=0.00' (Dynamic Tailwater) -1=Culvert (Barrel Controls 25.67 cfs @ 5.2 fps)

#### Pond p21-4:

Inflow Are	ea =	5.152 ac, Inflow Depth = 2.82"	for 10-yr event	
Inflow	=	12.22 cfs @ 12.03 hrs, Volume=	= 1.211 af	
Outflow	=	7.07 cfs @ 12.31 hrs, Volume=	= 1.201 af, Atten= 42%, Lag= 17.3 min	1
Primary	=	7.07 cfs @ 12.31 hrs, Volume=	= 1.201 af	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 496.00' Surf.Area= 5,112 sf Storage= 14,306 cf Peak Elev= 498.65' @ 12.31 hrs Surf.Area= 8,361 sf Storage= 32,515 cf (18,209 cf above start) Flood Elev= 499.00' Surf.Area= 8,847 sf Storage= 35,622 cf (21,317 cf above start) Plug-Flow detention time= 544.2 min calculated for 0.872 af (72% of inflow) Center-of-Mass det. time= 298.6 min (1,091.5 - 793.0)

Type III 2₄	4-hr 10-yr Rainfall=5.00"
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#	Invert	Avail.St	orage St	orage De	scription			
1	490.00'	44,4	433 cf <b>C</b> u	Custom Stage Data (Conic) Listed below				
Elevat	tion eet)	Surf.Area (sq-ft)		c.Store ic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)		
490	0.00	1,146		0	0	1,146		
492	2.00	1,784		2,907	2,907	1,839		
494	.00	2,530		4,292	7,199	2,654		
494	.50	2,733		1,315	8,514	2,876		
496	6.00	5,112		5,791	14,306	5,278		
498	8.00	7,468		12,506	26,812	7,699		
500	0.00	10,226		17,622	44,433	10,536		
1 1	Routing Primary Primary	496.00'		Orifice/G	rate C= 0.600 Sharp-Crested	Rectangular Weir	2 End Contraction(s)	

Primary OutFlow Max=7.06 cfs @ 12.31 hrs HW=498.65' TW=482.87' (Dynamic Tailwater) -1=Orifice/Grate (Orifice Controls 0.38 cfs @ 7.6 fps) -2=Sharp-Crested Rectangular Weir (Weir Controls 6.69 cfs @ 2.5 fps)

# Pond p21-5:

Inflow Area	=	2.398 ac, 1	nflow Depth	= 2.45"	for 10-y	r event		
Inflow	=	5.34 cfs @	12.19 hrs,	Volume=	0.	.489 af		
Primary	=	5.34 cfs @	12.19 hrs,	Volume=	0.	.489 af,	Atten= 0%,	Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

#### Pond p21-6:

Inflow Area	a =	6.182 ac, Inflow Depth = 2.80"	for 10-yr event	
Inflow	=	12.87 cfs @ 12.23 hrs, Volume=	1.441 af	
Outflow	=	7.13 cfs @ 12.52 hrs, Volume=	1.432 af,	Atten= 45%, Lag= 17.3 min
Primary	=	7.13 cfs @ 12.52 hrs, Volume=	1.432 af	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 492.00' Surf.Area= 3,323 sf Storage= 4,847 cf Peak Elev= 494.55' @ 12.52 hrs Surf.Area= 13,071 sf Storage= 28,263 cf (23,416 cf above start) Flood Elev= 495.00' Surf.Area= 13,824 sf Storage= 34,456 cf (29,609 cf above start) Plug-Flow detention time= 408.3 min calculated for 1.320 af (92% of inflow) Center-of-Mass det. time= 326.0 min (1,148.7 - 822.7)

#	Invert	Avail.Storage	Storage Description
1	488.00'	48,245 cf	Custom Stage Data (Conic) Listed below

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Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
488.00	296	0	0	296
490.00	924	1,162	1,162	946
490.50	1,110	508	1,670	1,141
492.00	3,323	3,177	4,847	3,367
492.50	6,166	2,336	7,182	6,212
494.00	12,147	13,484	20,666	12,214
496.00	15,500	27,579	48,245	15,669

4	<b>D</b> :	100.00			-
Ħ	Routing	Invert	Outlet Devi	ices	

1 Primary 492.00' **3.0" Vert. Orifice/Grate** C= 0.600

2 Primary 494.00' 5.0' long x 2.0' high Sharp-Crested Rectangular Weir 2 End Contraction(s)

Primary OutFlow Max=7.13 cfs @ 12.52 hrs HW=494.55' TW=483.44' (Dynamic Tailwater)

-1=Orifice/Grate (Orifice Controls 0.37 cfs @ 7.5 fps)

-2=Sharp-Crested Rectangular Weir (Weir Controls 6.76 cfs @ 2.5 fps)

# Pond p21-7:

Inflow Are	a =	8.355 ac, Inflow Depth =	3.10" for 10-yr event
Inflow	=	24.66 cfs @ 12.04 hrs, Volu	lume= 2.158 af
Outflow	=	3.91 cfs @ 12.67 hrs, Volu	lume= 2.138 af, Atten= 84%, Lag= 38.1 min
Primary	=	3.91 cfs @ 12.67 hrs, Volu	lume= 2.138 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 492.00' Surf.Area= 3,941 sf Storage= 8,984 cf Peak Elev= 498.13' @ 12.67 hrs Surf.Area= 11,908 sf Storage= 55,776 cf (46,793 cf above start) Flood Elev= 499.00' Surf.Area= 13,379 sf Storage= 67,369 cf (58,385 cf above start) Plug-Flow detention time= 522.8 min calculated for 1.932 af (90% of inflow) Center-of-Mass det. time= 407.4 min (1,185.2 - 777.8)

#	Invert	Avail.St	orage Storag	ge De	escription				
1	486.00'	80,	712 cf Custo	om St	age Data (Conic)	Listed below			
Eleva (f	ition eet)	Surf.Area (sq-ft)	Inc.St (cubic-fe		Cum.Store (cubic-feet)	Wet.Area (sq-ft <u>)</u>			
486	5.00	478		0	0	478			
488	3.00	964	1,4	414	1,414	999			
490	0.00	1,601	2,5	538	3,952	1,684			
490	0.50	1,782	8	345	4,797	1,879			
492	2.00	3,941	4,1	187	8,984	4,056			
494	4.00	6,120	9,9	981	18,965	6,292			
496	5.00	8,702	14,7	746	33,712	8,944			
498	3.00	11,686	20,3	315	54,027	12,012			
500	0.00	15,071	26,6	685	80,712	15,495			
1	Routing Primary Primary	Invert 492.00' 496.05'		fice/G	Grate C= 0.600 Grate X 2.00 C= (	0.600			
	Primary			6.0" Vert. Orifice/Grate X 2.00 C= 0.600 5.0' long x 6.0' high Sharp-Crested Rectangular Weir 2 End Contraction(s)					

Primary OutFlow Max=3.91 cfs @ 12.67 hrs HW=498.13' TW=483.77' (Dynamic Tailwater) -1=Orifice/Grate (Orifice Controls 0.58 cfs @ 11.8 fps) -2=Orifice/Grate (Orifice Controls 2.56 cfs @ 6.5 fps) -3=Sharp-Crested Rectangular Weir (Weir Controls 0.77 cfs @ 1.2 fps)

# Pond p22-1:

Field Note #54 Golf Pond Geometry to be confirmed by survey

Pond unchanged from existing to proposed conditions

Inflow Area	a =	78.382 ac, Inflow Depth = 1.84" for 10-yr event	
Inflow	=	88.26 cfs @ 12.29 hrs, Volume= 12.037 af	
Outflow	=	87.47 cfs @ 12.33 hrs, Volume= 11.738 af, Atten= 1%, Lag= 2.0 min	1
Primary	=	87.47 cfs @ 12.33 hrs, Volume= 11.738 af	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 498.10' Surf.Area= 6,520 sf Storage= 10,106 cf Peak Elev= 501.79' @ 12.33 hrs Surf.Area= 11,200 sf Storage= 42,096 cf (31,990 cf above start) Plug-Flow detention time= 50.8 min calculated for 11.504 af (96% of inflow) Center-of-Mass det. time= 21.5 min (902.6 - 881.1)

#	Invert	Avail.St	orage Storage Description			
1	495.00'	143,	770 cf Custom	Stage Data (Prism	atic) Listed below	
Flov	vation	Surf.Area	Inc.Store	Cum.Store		
	(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)		
-	. ,	· · · · ·	· · · · ·			
	95.00	0	0	0		
49	98.10	6,520	10,106	10,106		
50	00.00	8,390	14,164	24,270		
50	02.00	11,530	19,920	44,190		
50	04.00	14,530	26,060	70,250		
50	06.00	18,340	32,870	103,120		
50	08.00	22,310	40,650	143,770		
#	Routing	Invert	Outlet Devices			
1	Primary	499.75'	18.0" x 21.0' lon	g Culvert CMP, p	projecting, no headwall, Ke= 0.900	
	2		Outlet Invert= 499	9.75' S= 0.0000 '/	n= 0.024 Cc= 0.900	
2	Primary	500.50'	1.0' long x 15.0'	breadth Broad-Cr	ested Rectangular Weir	
	,				1.00 1.20 1.40 1.60	
					64 2.63 2.64 2.64 2.63	
3	Primary	500.50'	( <b>U</b>		Crested Rectangular Weir	
5	тппату	500.50	•		0	
			· · · ·		1.00 1.20 1.40 1.60	
			Coet. (English) 2	.62 2.66 2.70 2.6	66 2.65 2.66 2.65 2.63	

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Primary OutFlow Max=87.46 cfs @ 12.33 hrs HW=501.79' TW=482.91' (Dynamic Tailwater) -1=Culvert (Barrel Controls 5.82 cfs @ 3.3 fps) -2=Broad-Crested Rectangular Weir (Weir Controls 3.87 cfs @ 3.0 fps) -3=Broad-Crested Rectangular Weir (Weir Controls 77.78 cfs @ 3.0 fps)

#### Pond p23-1:

Inflow Area	a =	29.123 ac, Inflow Dept	h = 2.20"	for 10-yr event	
Inflow	=	37.22 cfs @ 12.54 hrs	, Volume=	5.334 af	
Outflow	=	37.19 cfs @ 12.55 hrs	, Volume=	4.619 af,	Atten= 0%, Lag= 0.7 min
Primary	=	37.19 cfs @ 12.55 hrs	, Volume=	4.619 af	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 507.86' @ 12.55 hrs Surf.Area= 14,276 sf Storage= 33,065 cf Plug-Flow detention time= 86.8 min calculated for 4.619 af (87% of inflow) Center-of-Mass det. time= 25.9 min (898.6 - 872.7)

#	Invert	Avail.Stora	ige Storage Des	scription			
1	503.50'	68,915	ocf Custom Sta	Custom Stage Data (Conic) Listed below			
Elevat (fe	ion eet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft <u>)</u>		
503.50		0	0	0	0		
504	.00	2,390	398	398	2,390		
506	.00	9,090	10,761	11,159	9,110		
508.00		14,660	23,529	34,688	14,732		
510.00		19,690	34,227	68,915	19,847		
#_F	Routing	Invert Ou	utlet Devices				

507.70' **178.0 deg x 178.0' long Sharp-Crested Vee/Trap Weir** C= 2.46 1 Primary

Primary OutFlow Max=37.18 cfs @ 12.55 hrs HW=507.86' TW=507.13' (Dynamic Tailwater) **1=Sharp-Crested Vee/Trap Weir** (Weir Controls 37.18 cfs @ 1.2 fps)

#### Pond p23-2:

Inflow Area =	16.094 ac, Inflow Depth = 3.60"	for 10-yr event
Inflow =	67.24 cfs @ 12.06 hrs, Volume=	4.833 af
Outflow =	17.07 cfs @ 12.42 hrs, Volume=	4.001 af, Atten= 75%, Lag= 21.5 min
Primary =	17.07 cfs @ 12.42 hrs, Volume=	4.001 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 508.00' Surf.Area= 7,318 sf Storage= 15,927 cf Peak Elev= 514.25' @ 12.42 hrs Surf.Area= 23,249 sf Storage= 123,418 cf (107,492 cf above start) Flood Elev= 515.00' Surf.Area= 24,788 sf Storage= 141,986 cf (126,059 cf above start) Plug-Flow detention time= 528.1 min calculated for 3.636 af (75% of inflow) Center-of-Mass det. time= 388.3 min (1,166.4 - 778.1)

1

720.10'

 Type III 24-hr 10-yr
 Rainfall=5.00"

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#         Invert         Avail.Storage         Storage Description           1         502.00'         166,746 cf         Custom Stage Data (Conic)         Listed below           Elevation         Surf.Area         Inc.Store         Cum.Store         Wet.Area           (feet)         (sq-ft)         (cubic-feet)         (sq-ft)           502.00         826         0         0         826						
(feet) (sq-ft) (cubic-feet) (sq-ft)						
504.00 1,667 2,444 2,444 1,702						
506.00 2,788 4,407 6,852 2,872						
506.50         3,112         1,474         8,326         3,210           508.00         7,218         7,601         15,027         7,422						
508.007,3187,60115,9277,432508.5012,6184,92420,85112,735						
510.00 15,208 20,839 41,690 15,400						
512.00 18,859 34,002 75,692 19,166						
514.00 22,736 41,535 117,227 23,175						
516.00 26,840 49,519 166,746 27,428						
# Routing Invert Outlet Devices						
1 Primary 508.00' <b>3.0" Vert. Orifice/Grate</b> C= 0.600						
2 Primary 512.55' <b>12.0" Vert. Orifice/Grate X 2.00</b> C= 0.600						
3 Primary 514.00' 20.0' long x 6.0' high Sharp-Crested Rectangular Weir						
2 End Contraction(s)						
Primary OutFlow Max=17.07 cfs @ 12.42 hrs HW=514.25' TW=483.16' (Dynamic Tailwater) -1=Orifice/Grate (Orifice Controls 0.58 cfs @ 11.9 fps) -2=Orifice/Grate (Orifice Controls 8.29 cfs @ 5.3 fps) -3=Sharp-Crested Rectangular Weir (Weir Controls 8.19 cfs @ 1.6 fps)						
Pond zDP1: Design Point 1						
Field note #10. Culvert dimensions to be confirmed by survey.						
Inflow Area =       26.658 ac, Inflow Depth =       2.18"       for 10-yr event         Inflow =       26.58 cfs @       12.51 hrs, Volume=       4.843 af         Outflow =       26.58 cfs @       12.52 hrs, Volume=       4.843 af, Atten= 0%, Lag= 0.2 min         Primary =       26.58 cfs @       12.52 hrs, Volume=       4.843 af						
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 722.07' @ 12.52 hrs Surf.Area= 98 sf Storage= 70 cf Flood Elev= 727.00' Surf.Area= 1,105 sf Storage= 2,619 cf Plug-Flow detention time= 0.1 min calculated for 4.842 af (100% of inflow) Center-of-Mass det. time= 0.1 min ( 935.6 - 935.5 ) # Invert Avail.Storage Storage Description						

3,706 cf Custom Stage Data (Conic) Listed below

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Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft <u>)</u>
720.10	0	0	0	0
722.00	90	57	57	96
724.00	340	403	460	364
726.00	760	1,072	1,533	815
728.00	1,450	2,173	3,706	1,543

#	Routing	Invert	Outlet Devices
1	Primary	720.10'	<b>42.0" x 120.0' long Culvert</b> CMP, square edge headwall, Ke= 0.500
	-		Outlet Invert= 700.00' S= 0.1675 '/' n= 0.024 Cc= 0.900
2	Primary	727.00'	155.0 deg Sharp-Crested Vee/Trap Weir C= 2.47

Primary OutFlow Max=26.57 cfs @ 12.52 hrs HW=722.07' TW=686.46' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 26.57 cfs @ 4.8 fps)

-2=Sharp-Crested Vee/Trap Weir (Controls 0.00 cfs)

#### Pond zDP2: Design Point 2

Field Note #15 Culvert dimensions to be confirmed by survey Overflow to ditch is currently discarded... We may have to model that area...

Inflow Area =	93.367 ac, Inflow Depth = 1.66"	for 10-yr event
Inflow =	65.20 cfs @ 12.87 hrs, Volume=	12.913 af
Outflow =	65.17 cfs @ 12.87 hrs, Volume=	12.913 af, Atten= 0%, Lag= 0.2 min
Discarded =	31.04 cfs @ 12.87 hrs, Volume=	1.895 af
Primary =	34.13 cfs @ 12.87 hrs, Volume=	11.019 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 625.69' @ 12.87 hrs Surf.Area= 1,318 sf Storage= 3,006 cf Flood Elev= 624.50' Surf.Area= 925 sf Storage= 1,728 cf Plug-Flow detention time= 0.7 min calculated for 12.913 af (100% of inflow) Center-of-Mass det. time= 0.5 min (909.9 - 909.4)

#	Invert	Avail.S	torage	Storage Description				
1	619.60'	7,	280 cf	Custom Stage Data (Conic) Listed below				
	ation (feet)	Surf.Area (sq-ft)		Inc.Store Jbic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)		
61	19.60	0		0	0	0		
62	620.00 10		1		1	10		
62	22.00	260		214	215	269		
62	24.00	760		976	1,192	793		
62	26.00	1,420		2,146	3,338	1,492		
62	28.00	2,580		3,943	7,280	2,694		
#	Routing	Invert	Outlet D	evices				
1	Primary	619.60'		<b>x 150.0' long Culvert</b> RCP, end-section conforming to fill, Ke= 0.500 t Invert= $608.00'$ S= $0.0773'/$ n= $0.012$ Cc= $0.900$				
2	Discarded	624.50'	166.0 de	0 deg Sharp-Crested Vee/Trap Weir C= 2.46				

**Discarded OutFlow** Max=31.03 cfs @ 12.87 hrs HW=625.69' (Free Discharge) **2=Sharp-Crested Vee/Trap Weir** (Weir Controls 31.03 cfs @ 2.7 fps)

Primary OutFlow Max=34.13 cfs @ 12.87 hrs HW=625.69' TW=607.67' (Dynamic Tailwater)

#### Pond zDP3: Design Point 3

Inflow Are	a =	228.471 ac, Inflow Depth = 18.31"	for	10-yr event	
Inflow	=	192.28 cfs @ 12.55 hrs, Volume=		348.596 af	
Primary	=	192.28 cfs @ 12.55 hrs, Volume=		348.596 af,	Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

#### Pond zDP4: Design Point 4

Inflow Area =	459.188 ac, Inflow Depth = 1.57"	for 10-yr event
Inflow =	25.67 cfs @ 20.43 hrs, Volume=	60.140 af
Primary =	25.67 cfs @ 20.43 hrs, Volume=	60.140 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

#### Pond zDP5: Design Point 5

Inflow Area	1 =	28.325 ac, I	nflow Depth = 2.04"	for 10-yr event	
Inflow	=	37.09 cfs @	12.45 hrs, Volume=	= 4.807 af	
Primary	=	37.09 cfs @	12.45 hrs, Volume=	= 4.807 af,	Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

# Post-Development Conditions 25 year 24 hour Storm Event Model Computations

Proposed Conditions_10454-01Type III 24-hr 25-yrRainfall=5.90"Prepared by The Chazen CompaniesPage 154HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems4/10/2006 3:15:41 PM		
Subcatchment s01-0:		
Runoff = 16.08 cfs @ 12.60 hrs, Volume= 2.435 af, Depth= 2.54"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90"		
Area (ac) CN Description		
11.485 68		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
42.8 Direct Entry,		
Subcatchment s02-1:		
Runoff = 86.23 cfs @ 12.87 hrs, Volume= 16.254 af, Depth= 2.28"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90"		
Area (ac) CN Description		
85.591 65		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
61.3Direct Entry,		
Subcatchment s02-2:		
Runoff = 12.02 cfs @ 12.44 hrs, Volume= 1.533 af, Depth= 2.37"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90"		
Area (ac) CN Description		
7.776 66		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
29.4Direct Entry,		
Subcatchment s02-3:		
Runoff = 23.69 cfs @ 12.02 hrs, Volume= 1.470 af, Depth= 4.31"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs		

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90"

Proposed Conditions_10454-017Prepared by The Chazen Companies7HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer System	Type III 24-hr 25-yr         Rainfall=5.90"           Page 155         s         4/10/2006         3:15:41 PM	
Area (ac) CN Description		
4.088 86		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
1.6Direct Entry,		
Subcatchment s03-1:		
Runoff = 19.03 cfs @ 12.41 hrs, Volume= 2.371 af,	Depth= 2.73"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, o Type III 24-hr 25-yr Rainfall=5.90"	dt= 0.01 hrs	
Area (ac) CN Description		
10.435 70		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
28.8 Direct Entry,		
Subcatchment s03-2:		
Runoff = 12.93 cfs @ 12.02 hrs, Volume= 0.781 af,	Depth= 3.10"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90"		
Area (ac) CN Description		
3.021 74		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
1.5 Direct Entry,		
Subcatchment s03-2(IC): s03-2 Impervious Cover		
Runoff = 11.29 cfs @ 12.02 hrs, Volume= 0.785 af,	Depth= 5.66"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90"		
Area (ac) CN Description		
1.663 98		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
1.5 Direct Entry,		

Proposed Conditions_10454-01Type III 24-hr 25-yrRainfall=5.90"Prepared by The Chazen CompaniesPage 156HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems4/10/2006 3:15:41 PM		
Subcatchment s03-2(OW): s03-2 Open Water		
Runoff = 0.38 cfs @ 12.00 hrs, Volume= 0.027 af, Depth= 5.90"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90"		
Area (ac) CN Description		
0.054 100		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
0.0 Direct Entry,		
Subcatchment s04-1:		
Runoff = 22.18 cfs @ 12.09 hrs, Volume= 1.601 af, Depth= 2.54"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90"		
Area (ac) CN Description		
7.549 68		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
6.1 Direct Entry,		
Subcatchment s05-1:		
Runoff = 11.28 cfs @ 12.21 hrs, Volume= 1.105 af, Depth= 1.94"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90"		
Area (ac) CN Description 6.842 61		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
14.4 Direct Entry,		
Subcatchment s06-0:		
Runoff = 14.52 cfs @ 12.25 hrs, Volume= 1.518 af, Depth= 2.02"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs		

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90"

Proposed Conditions_10454-01 Prepared by The Chazen Companies	<i>Type III 24-hr 25-yr Rainfall=5.90"</i> Page 157	
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Area (ac) CN Description		
9.007 62		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
17.3 Direct Entry,		
Subcatchment s06-0(OW): s06 Ope	n Water	
Runoff = 3.03 cfs @ 12.00 hrs, Volume= 0.210 af	, Depth= 5.90"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90"		
Area (ac) CN Description 0.428 100		
Subcatchment s07-1:		
Subcatchment S07-1.		
Runoff = 10.35 cfs @ 12.14 hrs, Volume= 0.851 af	, Depth= 2.19"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90"		
Area (ac) CN Description		
4.656 64		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
9.3 Direct Entry,		
Subcatchment s07-1(OW): s07 Open Water		
Runoff = 3.58 cfs @ 12.00 hrs, Volume= 0.249 af	, Depth= 5.90"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90"		
Area (ac) CN Description		
0.506 100		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
0.0 Direct Entry,		

Proposed Conditions_10454-01Type III 24-hr 25-yrRainfall=5.90"Prepared by The Chazen CompaniesPage 158HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems4/10/2006 3:15:41 PM		
Subcatchment s08-1:		
Runoff = 29.17 cfs @ 12.38 hrs, Volume= 3.578 af, Depth= 1.86"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90"		
Area (ac) CN Description		
23.126 60		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
24.5     Direct Entry,		
Subcatchment s08-2:		
Runoff = 17.76 cfs @ 12.17 hrs, Volume= 1.573 af, Depth= 2.11"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90"		
Area (ac) CN Description		
8.958 63		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
11.4   Direct Entry,		
Subcatchment s08-2(IC):		
Runoff = 35.69 cfs @ 12.04 hrs, Volume= 2.606 af, Depth= 5.66"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90"		
Area (ac) CN Description		
5.524 98		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
2.9Direct Entry,		
Subcatchment s08-2(OW):		
Runoff = 1.36 cfs @ 12.00 hrs, Volume= 0.094 af, Depth= 5.90"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs		

Type III 24-hr 25-yr Rainfall=5.90"

Proposed Conditions_10454-01	Type III 24-hr 25-yr Rainfall=5.90"	
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Area (ac) CN Description		
0.192 100		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
0.0 Direct Entry,		
Subcatchment s08-3:		
Runoff = 2.86 cfs @ 12.20 hrs, Volume= 0.275 af	, Depth= 1.94"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90"		
Area (ac) CN Description		
1.700 61		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
13.6 <b>Direct Entry</b> ,		
Subsetshment s00 2/IC), s00 2 lmpsm		
Subcatchment s08-3(IC): s08-3 Imperv	lous cover	
Runoff = 7.48 cfs @ 12.01 hrs, Volume= 0.512 af	, Depth= 5.66"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, Type III 24-hr 25-yr Rainfall=5.90"	dt= 0.01 hrs	
Area (ac) CN Description		
1.086 98		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
1.0 Direct Entry,		
Subcatchment s08-3(OW): s08-3 Open Water		
Runoff = 0.30 cfs @ 12.00 hrs, Volume= 0.021 af	, Depth= 5.90"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90"		
Area (ac) CN Description		
0.042 100		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
0.0 Direct Entry,		

Proposed Conditions_10454-01Type III 24-hr 25-yrRainfall=5.90"Prepared by The Chazen CompaniesPage 160HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems4/10/2006 3:15:41 PM		
Subcatchment s09-1:		
Runoff = 4.79 cfs @ 12.14 hrs, Volume= 0.403 af, Depth= 1.86"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90"		
Area (ac) CN Description 2.604 60		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
9.0 Direct Entry,		
Subcatchment s09-2:		
Runoff = 35.01 cfs @ 12.29 hrs, Volume= 3.807 af, Depth= 2.45"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90"		
Area (ac) CN Description		
18.608 67		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
20.6 Direct Entry,		
Subcatchment s09-2(IC): s09-2 Impervious Cover		
Runoff = 15.30 cfs @ 12.04 hrs, Volume= 1.102 af, Depth= 5.66"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90"		
Area (ac) CN Description 2.336 98		
Tc       Length       Slope       Velocity       Capacity       Description         (min)       (feet)       (ft/ft)       (ft/sec)       (cfs)		
2.5 Direct Entry,		
Subcatchment s09-2(OW): s09-2 Open Water		
Runoff = 1.67 cfs @ 12.00 hrs, Volume= 0.116 af, Depth= 5.90"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90"		

Proposed Conditions_10454-01Type III 24-hr 25-yrRainfall=5.90"Prepared by The Chazen CompaniesPage 161HydroCAD® 7.00s/n 000927© 1986-2003 Applied Microcomputer Systems4/10/20063:15:42 PM		
Area (ac) CN Description		
0.236 100		
TcLengthSlopeVelocityCapacityDescription(min)(ft/ft)(ft/sec)(cfs)		
0.0 Direct Entry,		
Subcatchment s09-3:		
Runoff = 9.53 cfs @ 12.15 hrs, Volume= 0.810 af, Depth= 2.54"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90"		
Area (ac) CN Description		
3.818 68		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
10.9     Direct Entry,		
Subcatchment s10-1:		
Runoff = 13.79 cfs @ 12.40 hrs, Volume= 1.704 af, Depth= 2.54"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90"		
Area (ac) CN Description		
8.038 68		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
27.9Direct Entry,		
Subcatchment s10-1(OW): s10 Open Water		
Runoff = 5.88 cfs @ 12.00 hrs, Volume= 0.408 af, Depth= 5.90"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90"		
Area (ac) CN Description 0.830 100		

Proposed Conditions_10454-01Type III 24-hr 25-yrRainfall=5.90"Prepared by The Chazen CompaniesPage 162HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems4/10/2006 3:15:42 PM		
Subcatchment s13-1:		
Runoff = 8.63 cfs @ 12.05 hrs, Volume= 0.574 af, Depth= 1.94"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90"		
Area (ac) CN Description		
3.555 61		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
2.8     Direct Entry,		
Subcatchment s13-1(IC): s13-1 Impervious Cover		
Runoff = 41.24 cfs @ 12.04 hrs, Volume= 3.001 af, Depth= 5.66"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90"		
Area (ac) CN Description		
6.360 98		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
2.8 Direct Entry,		
Subcatchment s13-1(OW): s13-1 Open Water		
Runoff = 0.93 cfs @ 12.00 hrs, Volume= 0.064 af, Depth= 5.90"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90"		
Area (ac) CN Description		
0.131 100		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
0.0 Direct Entry,		
Subcatchment s14-1:		
Runoff = 24.44 cfs @ 12.40 hrs, Volume= 3.014 af, Depth= 2.63"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90"		

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Area (ac) CN Description		
13.727 69		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
28.1 Direct Entry,		
Subcatchment s14-1(IC): s14-1 Impervious Cover		
Runoff = 12.15 cfs @ 12.03 hrs, Volume= 0.868 af, Depth= 5.66"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90"		
Area (ac) CN Description 1.840 98		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
2.3 Direct Entry,		
Subcatchment s14-1(OW): s14 Open Water		
Runoff = 3.67 cfs @ 12.00 hrs, Volume= 0.255 af, Depth= 5.90"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90"		
Area (ac) CN Description		
0.518 100		
Subcatchment s14-2:		
Runoff = 1.07 cfs @ 12.10 hrs, Volume= 0.081 af, Depth= 1.94"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90"		
Area (ac) CN Description		
0.504 61		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
6.4     Direct Entry,		

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Subcatchment s14-2(OW): s14-2 Open Water		
Runoff = 1.25 cfs @ 12.00 hrs, Volume= 0.087 af, Depth= 5.90"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90"		
Area (ac) CN Description		
0.176 100		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
0.0 Direct Entry,		
Subcatchment s14-3:		
Runoff = 19.07 cfs @ 12.12 hrs, Volume= 1.492 af, Depth= 2.63"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90"		
Area (ac) CN Description		
6.794 69		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
8.5 Direct Entry,		
Subcatchment s14-3(IC): s14-3 Impervious Cover		
Runoff = 54.65 cfs @ 12.04 hrs, Volume= 3.992 af, Depth= 5.66"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90"		
Area (ac) CN Description		
8.460 98		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
2.9Direct Entry,		
Subcatchment s16-1:		
Runoff = 77.13 cfs @ 12.27 hrs, Volume= 8.117 af, Depth= 2.45"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs		

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90"

Proposed Conditions_10454-01 Prepared by The Chazen Companies HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Syste	Type III 24-hr 25-yr         Rainfall=5.90"           Page 165           sms         4/10/2006         3:15:42 PM	
	".e. <u>2000 e.ro.i2 r m</u>	
Area (ac) CN Description		
39.680 67		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
19.0 Direct Entry,		
Subcatchment s16-1(OW): s16-1 Op	ben Water	
Runoff = 37.90 cfs @ 12.00 hrs, Volume= 2.631 at	f, Depth= 5.90"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs Type III 24-hr 25-yr Rainfall=5.90"	, dt= 0.01 hrs	
Area (ac) CN Description		
5.351 100		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
0.0 Direct Entry,		
Subcatchment s16-2:		
Runoff = 6.23 cfs @ 12.22 hrs, Volume= 0.597 at	f, Depth= 3.29"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs Type III 24-hr 25-yr Rainfall=5.90"	, dt= 0.01 hrs	
Area (ac) CN Description		
2.176 76		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
15.8 Direct Entry,		
Subcatchment s17-1:		
Runoff = 8.13 cfs @ 12.50 hrs, Volume= 1.116 at	f, Depth= 2.19"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90"		
Area (ac) CN Description		
6.110 64		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
33.8 Direct Entry,		

Proposed Conditions_10454-01	Type III 24-hr 25-yr Rainfall=5.90"	
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Subcatchment s17-1(OW): s17-1 Op	en Water	
Runoff = 1.16 cfs @ 12.00 hrs, Volume= 0.081 af	, Depth= 5.90"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, Type III 24-hr 25-yr Rainfall=5.90"	dt= 0.01 hrs	
Area (ac) CN Description		
0.164 100		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
0.0 Direct Entry,		
Subcatchment s17-2:		
Runoff = 59.85 cfs @ 13.29 hrs, Volume= 15.004 af	, Depth= 2.37"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, Type III 24-hr 25-yr Rainfall=5.90"	dt= 0.01 hrs	
Area (ac) CN Description		
76.086 66		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
95.7 Direct Entry,		
Subcatchment s17-3:		
Runoff = 45.38 cfs @ 12.45 hrs, Volume= 5.892 af	, Depth= 2.37"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90"		
Area (ac) CN Description		
29.880 66		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
30.7 Direct Entry,		
Subcatchment s18-1:		
Runoff = 15.28 cfs @ 12.23 hrs, Volume= 1.540 af	, Depth= 2.19"	

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90"

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Area (ac) CN Description		
8.429 64		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
16.4 Direct Entry,		
Subcatchment s18-1(OW): s18-1 Op	en Water	
Runoff = 3.34 cfs @ 12.00 hrs, Volume= 0.232 af	, Depth= 5.90"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90"		
Area (ac) CN Description		
0.472 100		
Subcatchment s18-2:		
Runoff = 23.04 cfs @ 12.26 hrs, Volume= 2.354 af	, Depth= 2.63"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90"		
Area (ac) CN Description		
10.721 69		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
18.0 Direct Entry,		
Subcatchment s19-0:		
Runoff = 15.56 cfs @ 12.61 hrs, Volume= 2.401 af	, Depth= 1.86"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90"		
Area (ac) CN Description		
15.520 60		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
40.4 Direct Entry,		

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Subcatchment s20-1:		
Runoff = 15.16 cfs @ 12.31 hrs, Volume= 1.688 af,	Depth= 2.37"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, Type III 24-hr 25-yr Rainfall=5.90"	dt= 0.01 hrs	
Area (ac) CN Description		
8.559 66		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
21.5 Direct Entry,		
Subcatchment s20-1(OW): s20-1 Open Water		
Runoff = 13.94 cfs @ 12.00 hrs, Volume= 0.968 af,	Depth= 5.90"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90"		
Area (ac) CN Description		
1.968 100		
Subcatchment s20-2:		
Runoff = 27.29 cfs @ 12.12 hrs, Volume= 2.108 af,	Depth= 3.10"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, Type III 24-hr 25-yr Rainfall=5.90"	dt= 0.01 hrs	
Area (ac) CN Description		
8.157 74		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
8.4 Direct Entry,		
Subcatchment s20-2(IC): s20-2 Impervious Cover		
Runoff = 30.94 cfs @ 12.07 hrs, Volume= 2.412 af,	Depth= 5.66"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90"		
Area (ac) CN Description		
5.112 98		

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Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
4.7 Direct Entry,		
Subcatchment s20-2(OW): s20-2 Open Water		
Runoff = 1.71 cfs @ 12.00 hrs, Volume= 0.119 af, Depth= 5.90"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90"		
Area (ac) CN Description		
0.242 100		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
0.0 Direct Entry,		
Subcatchment s20-3:		
Runoff = 14.12 cfs @ 12.30 hrs, Volume= 1.564 af, Depth= 2.73"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90"		
Area (ac) CN Description		
6.886 70		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
22.0 Direct Entry,		
Subcatchment s21-1:		
Runoff = 136.65 cfs @ 12.23 hrs, Volume= 13.557 af, Depth= 2.54"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90"		
Area (ac) CN Description		
63.942 68		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
16.6 <b>Direct Entry</b> ,		

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Subcatchment s21-1(OW):		
Runoff = 86.66 cfs @ 12.00 hrs, Volume= 6.016 af, Depth= 5.90"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90"		
Area (ac) CN Description 12.235 100		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
0.0 Direct Entry,		
Subcatchment s21-2:		
Runoff = 36.78 cfs @ 12.45 hrs, Volume= 4.757 af, Depth= 2.73"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90"		
Area (ac) CN Description		
20.941 70		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
31.4 Direct Entry,		
Subcatchment s21-3:		
Runoff = 26.20 cfs @ 12.16 hrs, Volume= 2.214 af, Depth= 3.10"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90"		
Area (ac) CN Description		
8.567 74		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
11.2 Direct Entry,		
Subcatchment s21-4:		
Runoff = 7.48 cfs @ 12.19 hrs, Volume= 0.694 af, Depth= 2.45"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90"		

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Area (ac) CN Description		
3.392 67		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
13.8 Direct Entry,		
Subcatchment s21-4(IC): s21-4 Impervious Cover		
Subcatchment 321-4(IC). 321-4 Impervious Cover		
Runoff = 11.11 cfs @ 12.02 hrs, Volume= 0.775 af, Depth= 5.66"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90"		
Area (ac) CN Description		
1.643 98		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
1.6     Direct Entry,		
Subcatchment s21-4(OW): s21-4 Open Water		
Runoff = 0.83 cfs @ 12.00 hrs, Volume= 0.058 af, Depth= 5.90"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90"		
Area (ac) CN Description		
0.117 100		
Subcatchment s21-5:		
Runoff = 7.00 cfs @ 12.19 hrs, Volume= 0.639 af, Depth= 3.20"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90"		
Area (ac) CN Description		
2.398 75		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
13.9 Direct Entry,		

Proposed Conditions_10454-01 Prepared by The Chazen Companies HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Syste	Type III 24-hr 25-yr         Rainfall=5.90"           Page 172           rms         4/10/2006 3:15:43 PM	
Subcatchment s21-6:		
Runoff = 15.02 cfs @ 12.24 hrs, Volume= 1.500 a	f, Depth= 3.29"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs Type III 24-hr 25-yr Rainfall=5.90"	, dt= 0.01 hrs	
Area (ac) CN Description		
5.463 76		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
17.5 Direct Entry,		
Subcatchment s21-6(IC): s21-6 Impervious Cover		
Runoff = 4.36 cfs @ 12.02 hrs, Volume= 0.303 a	f, Depth= 5.66"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90"		
Area (ac) CN Description		
0.643 98		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
1.5Direct Entry,		
Subcatchment s21-6(OW): s21-6 Op	ben Water	
Runoff = 0.54 cfs @ 12.00 hrs, Volume= 0.037 a	f, Depth= 5.90"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90"		
Area (ac) CN Description		
0.076 100		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
0.0 Direct Entry,		
Subcatchment s21-7:		
Runoff = 8.64 cfs @ 12.19 hrs, Volume= 0.799 a	f, Depth= 2.19"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs		

Type III 24-hr 25-yr Rainfall=5.90"

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Area (ac) CN Description		
4.375 64		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
13.1 Direct Entry,		
Subcatchment s21-7(IC): s21-7 Imperv	vious Cover	
Runoff = 25.48 cfs @ 12.04 hrs, Volume= 1.835 at	f, Depth= 5.66"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs Type III 24-hr 25-yr Rainfall=5.90"	, dt= 0.01 hrs	
Area (ac) CN Description		
3.890 98		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
2.5 <b>Direct Entry</b> ,		
Subcatchment s21-7(OW): s21-7 Op	ben water	
Runoff = 0.64 cfs @ 12.00 hrs, Volume= 0.044 at	f, Depth= 5.90"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs Type III 24-hr 25-yr Rainfall=5.90"	, dt= 0.01 hrs	
Area (ac) CN Description		
0.090 100		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
0.0 Direct Entry,		
Subcatchment s22-1:		
Runoff = $46.39 \text{ cfs} @ 12.20 \text{ hrs}$ , Volume= $4.338 \text{ at}$	f, Depth= 2.91"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90"		
Area (ac) CN Description		
17.878 72		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
14.7 Direct Entry,		

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Subcatchment s22-1(OW): s22-1 Ope	en Water	
Runoff = 0.96 cfs @ 12.00 hrs, Volume= 0.067 af,	Depth= 5.90"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, o Type III 24-hr 25-yr Rainfall=5.90"	dt= 0.01 hrs	
Area (ac) CN Description 0.136 100		
Subcatchment s22-2:		
Runoff = 82.29 cfs @ 12.35 hrs, Volume= 9.509 af,	Depth= 2.54"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90"		
Area (ac) CN Description 44.848 68		
Tc Length Slope Velocity Capacity Description		
(min) (feet) (ft/ft) (ft/sec) (cfs) 24.0 Direct Entry,		
Subcatchment s23-1:		
Runoff = 49.78 cfs @ 12.54 hrs, Volume= 7.066 af, Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, o Type III 24-hr 25-yr Rainfall=5.90"	Depth= 2.91" dt= 0.01 hrs	
Area (ac) CN Description 29.123 72		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
38.5 Direct Entry,		
Subcatchment s23-2:		
Runoff = 37.37 cfs @ 12.06 hrs, Volume= 2.471 af,	Depth= 3.39"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90"		
Area (ac) CN Description 8.741 77		

Proposed Conditions_10454-01Type III 24-hr 25-yrRainfall=5.90"Prepared by The Chazen CompaniesPage 175HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems4/10/2006 3:15:43 PM					
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)					
4.0 Direct Entry,					
Subcatchment s23-2(IC): s23-2 Impervious Cover					
Runoff = 44.59 cfs @ 12.06 hrs, Volume= 3.390 af, Depth= 5.66"					
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90"					
Area (ac) CN Description					
7.185 98					
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)					
4.0 Direct Entry,					
Subcatchment s23-2(OW): s23-2 Open Water					
Runoff = 1.19 cfs @ 12.00 hrs, Volume= 0.083 af, Depth= 5.90"					
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90"					
Area (ac) CN Description					
0.168 100					
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)					
0.0 Direct Entry,					
Subcatchment s24-0:					
Runoff = 50.24 cfs @ 12.45 hrs, Volume= 6.435 af, Depth= 2.73"					
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90"					
Area (ac) CN Description					
28.325 70					
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)					
30.7 Direct Entry,					

Proposed Conditions_10454-01Type III 24-hr 25-yrRainfall=5.90"Prepared by The Chazen CompaniesPage 176HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems4/10/2006 3:15:43 PM				
Subcatchment s25-0:				
Runoff = 24.65 cfs @ 12.28 hrs, Volume= 2.674 af, Depth= 2.37"				
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.90"				
Area (ac) CN Description 13.562 66				
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)				
20.2 Direct Entry,				
Reach 25R:				
Overland Flow Reach				
Inflow Area =       15.520 ac, Inflow Depth =       1.86" for 25-yr event         Inflow =       9.61 cfs @       13.04 hrs, Volume=       2.400 af         Outflow =       9.55 cfs @       13.11 hrs, Volume=       2.399 af, Atten= 1%, Lag= 3.7 min				
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 2.1 fps, Min. Travel Time= 4.8 min Avg. Velocity = 0.7 fps, Avg. Travel Time= 15.7 min				
Peak Depth= 0.26' @ 13.11 hrs Capacity at bank full= 175.17 cfs Inlet Invert= 560.00', Outlet Invert= 512.00' 50.00' x 1.00' deep Parabolic Channel, n= 0.060 Length= 620.0' Slope= 0.0774 '/'				
Reach r03-1:				
Overland Flow Reach Requires more survey				
Inflow Area =       11.485 ac, Inflow Depth =       2.54" for 25-yr event         Inflow =       16.08 cfs @       12.60 hrs, Volume=       2.435 af         Outflow =       16.00 cfs @       12.64 hrs, Volume=       2.435 af, Atten= 1%, Lag= 2.0 min				
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 5.5 fps, Min. Travel Time= 2.4 min Avg. Velocity = 2.1 fps, Avg. Travel Time= 6.3 min				
Peak Depth= 0.66' @ 12.64 hrs Capacity at bank full= 92.14 cfs Inlet Invert= 845.00', Outlet Invert= 728.00' 10.00' x 1.50' deep Parabolic Channel, n= 0.060 Length= 785.0' Slope= 0.1490 '/'				

Proposed Conditions\_10454-01

# Reach r04-1:

Channel

Inflow Area = Inflow = Outflow =	26.658 ac, Inflow Depth = 2.88" 36.10 cfs @ 12.51 hrs, Volume= 36.09 cfs @ 12.52 hrs, Volume=	6.393 af	Atten= 0%, Lag= 0.4 min		
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 7.8 fps, Min. Travel Time= 0.7 min Avg. Velocity = 2.1 fps, Avg. Travel Time= 2.6 min					
Peak Depth= 1.11' @ 12.52 hrs Capacity at bank full= 530.15 cfs Inlet Invert= 685.50', Outlet Invert= 632.00' 12.00' x 4.00' deep Parabolic Channel, n= 0.060 Length= 330.0' Slope= 0.1621 '/'					

#### Reach r08-1a:

Man Made Ditch Inverts of pipe to be surveyed

Inflow Area =	93.367 ac, Inflow Depth = 1.74"	for 25-yr event
Inflow =	35.21 cfs @ 12.87 hrs, Volume=	13.568 af
Outflow =	35.21 cfs @ 12.87 hrs, Volume=	13.568 af, Atten= 0%, Lag= 0.2 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 9.5 fps, Min. Travel Time= 0.4 min Avg. Velocity = 5.3 fps, Avg. Travel Time= 0.7 min

Peak Depth= 0.68' @ 12.87 hrs Capacity at bank full= 81.88 cfs Inlet Invert= 607.00', Outlet Invert= 587.00' 10.00' x 1.00' deep Parabolic Channel, n= 0.027 Length= 226.0' Slope= 0.0885 '/'

#### Reach r08-1b:

24" HDPE Inverts to be surveyed

Inflow Area = 93.367 ac, Inflow Depth = 1.74" for 25-yr event Inflow = 35.21 cfs @ 12.87 hrs, Volume= 13.568 af Outflow = 35.21 cfs @ 12.87 hrs, Volume= 13.568 af, Atten= 0%, Lag= 0.2 min Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 24.0 fps, Min. Travel Time= 0.2 min Avg. Velocity = 13.9 fps, Avg. Travel Time= 0.4 min

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Peak Depth= 0.95' @ 12.87 hrs Capacity at bank full= 77.17 cfs Inlet Invert= 587.00', Outlet Invert= 557.75' 24.0" Diameter Pipe n= 0.012 Length= 295.0' Slope= 0.0992 '/'

### Reach r08-1c:

Ditch Pipe inverts to be surveyed

Inflow Area	a =	93.367 ac, Inflow Dep	th = 1.74"	for 25-yr event	
Inflow	=	35.21 cfs @ 12.87 hrs	, Volume=	13.568 af	
Outflow	=	35.21 cfs @ 12.88 hrs	s, Volume=	13.568 af,	Atten= 0%, Lag= 0.4 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 9.1 fps, Min. Travel Time= 1.1 min Avg. Velocity = 4.8 fps, Avg. Travel Time= 2.0 min

Peak Depth= 0.70' @ 12.88 hrs Capacity at bank full= 76.65 cfs Inlet Invert= 557.75', Outlet Invert= 512.00' 10.00' x 1.00' deep Parabolic Channel, n= 0.027 Length= 590.0' Slope= 0.0775 '/

#### Reach r08-1d: Amenia Creek/Cascade Brook

(Steam) parallel to Route 22 Base Flow estimated from field observation (see field note 21)

Inflow Area = 114.957 ac, Inflow Depth =  $18.60^{\circ}$  for 25-yr event Inflow = 102.65 cfs @ 12.39 hrs, Volume= 178.149 af, Incl. 40.00 cfs Base Flow Outflow = 101.99 cfs @ 12.45 hrs, Volume= 177.904 af, Atten= 1%, Lag= 3.4 min Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 3.9 fps, Min. Travel Time= 3.4 min Avg. Velocity = 3.1 fps, Avg. Travel Time= 4.3 min

Peak Depth= 3.95' @ 12.45 hrs Capacity at bank full= 104.49 cfs Inlet Invert= 512.00', Outlet Invert= 504.00' 10.00' x 4.00' deep Parabolic Channel, n= 0.060 Length= 805.0' Slope= 0.0099 '/'

### Reach r13-1:

 Inflow Area =
 2.176 ac, Inflow Depth =
 3.29" for 25-yr event

 Inflow =
 6.23 cfs @
 12.22 hrs, Volume=
 0.597 af

 Outflow =
 6.17 cfs @
 12.24 hrs, Volume=
 0.597 af, Atten= 1%, Lag= 1.3 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 9.2 fps, Min. Travel Time= 1.6 min Avg. Velocity = 3.4 fps, Avg. Travel Time= 4.4 min

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Peak Depth= 0.61' @ 12.24 hrs Capacity at bank full= 17.79 cfs Inlet Invert= 546.00', Outlet Invert= 524.00' 18.0" Diameter Pipe n= 0.012 Length= 900.0' Slope= 0.0244 '/'

#### Reach r14-3a:

30" HDPE Under Main Entrance Road

Inflow Are Inflow Outflow	a = = =	14.20 cfs @	nflow Depth = 2.27" 12.16 hrs, Volume= 12.17 hrs, Volume=	1.212 af	Atten= 0%, Lag= 0.4 min
Routing by Dyn-Stor-Ind method. Time Span= $0.00-48.00$ brs. dt= $0.01$ brs.					

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 13.9 fps, Min. Travel Time= 0.5 min Avg. Velocity = 5.2 fps, Avg. Travel Time= 1.4 min

Peak Depth= 0.65' @ 12.17 hrs Capacity at bank full= 94.91 cfs Inlet Invert= 526.00', Outlet Invert= 505.70' 30.0" Diameter Pipe n= 0.012 Length= 445.0' Slope= 0.0456 '/'

#### Reach r14-3b:

Grass lined channel

Inflow Area = 6.422 ac, Inflow Depth =  $2.27^{"}$  for 25-yr event Inflow = 14.28 cfs @ 12.15 hrs, Volume= 1.212 afOutflow = 14.20 cfs @ 12.16 hrs, Volume= 1.212 af, Atten= 1%, Lag= 0.7 min Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrsMax. Velocity= 6.6 fps, Min. Travel Time= 0.9 minAvg. Velocity = 2.5 fps, Avg. Travel Time= 2.4 minPeak Depth= 0.68' @ 12.16 hrsCapacity at bank full= 325.42 cfs

Capacity at bank full= 325.42 cfs Inlet Invert= 542.00', Outlet Invert= 526.00'  $10.00' \times 3.00'$  deep Parabolic Channel, n= 0.027 Length= 360.0' Slope= 0.0444 '/'

#### Reach r17-1:

 Inflow Area =
 76.086 ac, Inflow Depth =
 2.37"
 for 25-yr event

 Inflow =
 59.85 cfs @
 13.29 hrs, Volume=
 15.004 af

 Outflow =
 59.64 cfs @
 13.34 hrs, Volume=
 15.004 af, Atten= 0%, Lag= 2.7 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 8.1 fps, Min. Travel Time= 2.8 min Avg. Velocity = 3.2 fps, Avg. Travel Time= 7.2 min

Proposed Conditions_10454-01 Prepared by The Chazen Companies HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Syste	Type III 24-hr 25-yr         Rainfall=5.90"           Page 180           ems         4/10/2006 3:15:44 PM
Peak Depth= 1.19' @ 13.34 hrs Capacity at bank full= 181.28 cfs Inlet Invert= 646.00', Outlet Invert= 524.00' 12.00' x 2.00' deep Parabolic Channel, n= 0.045 Length= 1,390.0	)' Slope= 0.0878 '/'
Reach r18-2:	
Overland Flow Reach	
Inflow         =         0.00 cfs @         0.00 hrs, Volume=         0.000 a           Outflow         =         0.00 cfs @         0.00 hrs, Volume=         0.000 a	f f, Atten= 0%, Lag= 0.0 min
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.0 Max. Velocity= 0.0 fps, Min. Travel Time= 0.0 min Avg. Velocity = 0.0 fps, Avg. Travel Time= 0.0 min	11 hrs
Peak Depth= 0.00' @ 0.00 hrs Capacity at bank full= 434.91 cfs Inlet Invert= 973.60', Outlet Invert= 630.00' 50.00' x 1.00' deep Parabolic Channel, n= 0.060 Length= 720.0'	Slope= 0.4772 '/'
Reach r21-1a:	
Man Made Ditch	
Inflow Area =       207.817 ac, Inflow Depth =       1.82" for 25-yr ever         Inflow =       44.11 cfs @       15.25 hrs, Volume=       31.459 a         Outflow =       44.11 cfs @       15.27 hrs, Volume=       31.445 a	
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.0 Max. Velocity= 6.3 fps, Min. Travel Time= 1.7 min Avg. Velocity = 3.1 fps, Avg. Travel Time= 3.4 min	11 hrs
Peak Depth= 1.48' @ 15.27 hrs Capacity at bank full= 191.76 cfs Inlet Invert= 504.00', Outlet Invert= 494.00' 10.00' x 3.00' deep Parabolic Channel, n= 0.027 Length= 648.0'	Slope= 0.0154 '/'
Reach r21-1b:	
Overland Flow Reach	
Inflow Area =         29.123 ac, Inflow Depth =         2.62" for 25-yr ever           Inflow =         49.74 cfs @         12.55 hrs, Volume=         6.351 a           Outflow =         49.72 cfs @         12.55 hrs, Volume=         6.351 a	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 4.3 fps, Min. Travel Time= 0.6 min Avg. Velocity = 1.6 fps, Avg. Travel Time= 1.6 min Proposed Conditions 10454-01 Type III 24-hr 25-yr Rainfall=5.90" Prepared by The Chazen Companies HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems 4/10/2006 3:15:44 PM

Peak Depth= 0.50' @ 12.55 hrs Capacity at bank full= 227.81 cfs Inlet Invert= 506.70', Outlet Invert= 485.75' 50.00' x 1.00' deep Parabolic Channel, n= 0.060 Length= 160.0' Slope= 0.1309 '/'

#### Reach r22-2:

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**Overland Flow Reach** 

Inflow Area = Inflow = Outflow =	15.520 ac, Inflow Depth =1.86"for 25-yr event9.63 cfs @13.01 hrs, Volume=2.400 af9.61 cfs @13.04 hrs, Volume=2.400 af, Atten= 0%, Lag= 1.8 min			
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 4.3 fps, Min. Travel Time= 2.5 min Avg. Velocity = 1.4 fps, Avg. Travel Time= 7.7 min				
Peak Depth= 0.17' @ 13.04 hrs Capacity at bank full= 469.25 cfs Inlet Invert= 970.00', Outlet Invert= 620.00' 50.00' x 1.00' deep Parabolic Channel, n= 0.060 Length= 630.0' Slope= 0.5556 '/'				

#### Reach r25-0a:

Ditch Pipe inverts need to be surveyed

Inflow Area =	67.391 ac, Inflow Depth = 2.59"	for 25-yr event
Inflow =	45.93 cfs @ 12.52 hrs, Volume=	14.567 af
Outflow =	45.89 cfs @ 12.54 hrs, Volume=	14.563 af, Atten= 0%, Lag= 1.2 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 9.9 fps, Min. Travel Time= 1.8 min Avg. Velocity = 3.5 fps, Avg. Travel Time= 5.2 min

Peak Depth= 0.99' @ 12.54 hrs Capacity at bank full= 205.50 cfs Inlet Invert= 570.00', Outlet Invert= 504.00' 10.00' x 2.00' deep Parabolic Channel, n= 0.027 Length= 1,090.0' Slope= 0.0606 '/'

# Reach r25-0b: Wetland Reach

Wetland Reach Has wetland vegetation within reach

Inflow Area =	9.435 ac, Inflow Depth = $2.16$ "	for 25-yr event
Inflow =	13.69 cfs @ 12.35 hrs, Volume=	1.699 af
Outflow =	12.38 cfs @ 12.45 hrs, Volume=	1.699 af, Atten= 10%, Lag= 6.4 min

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Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 1.8 fps, Min. Travel Time= 6.9 min Avg. Velocity = 0.4 fps, Avg. Travel Time= 27.9 min

Peak Depth= 0.92' @ 12.45 hrs Capacity at bank full= 156.51 cfs Inlet Invert= 504.00', Outlet Invert= 499.50' 20.00' x 3.00' deep Parabolic Channel, n= 0.045 Length= 750.0' Slope= 0.0060 '/'

#### Reach r25-0c: Amenia Creek/Cascade Brook

(Steam) parallel to Route 22 Base Flow estimated from field observation (see field note 21)

138.083 ac, Inflow Depth = 29.56" for 25-yr event Inflow Area = Inflow 170.35 cfs @ 12.42 hrs, Volume= 340.192 af, Incl. 40.00 cfs Base Flow = 166.65 cfs @ 12.51 hrs, Volume= Outflow = 339.508 af, Atten= 2%, Lag= 5.8 min Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 3.0 fps, Min. Travel Time= 5.5 min Avg. Velocity = 2.7 fps, Avg. Travel Time= 6.1 min Peak Depth= 6.94' @ 12.51 hrs Capacity at bank full= 67.14 cfs Inlet Invert= 504.00', Outlet Invert= 500.00' 10.00' x 4.00' deep Parabolic Channel, n= 0.060 Length= 975.0' Slope= 0.0041 '/' Pond 8P: No field note. Water spills over cart path; no storage. Inflow Area = 41.049 ac, Inflow Depth = 2.66" for 25-yr event Inflow = 39.11 cfs @ 12.50 hrs, Volume= 9.099 af Outflow 39.11 cfs @ 12.50 hrs, Volume= 9.099 af, Atten= 0%, Lag= 0.0 min = 39.11 cfs @ 12.50 hrs, Volume= Primary 9.099 af = Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 575.40' @ 12.50 hrs Flood Elev= 574.70' Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= (not calculated)

#	Routing	Invert	Outlet Devices
1	Primary	574.70'	177.0 deg Sharp-Crested Vee/Trap Weir C= 2.46

Primary OutFlow Max=39.11 cfs @ 12.50 hrs HW=575.40' TW=570.99' (Dynamic Tailwater) ←1=Sharp-Crested Vee/Trap Weir (Weir Controls 39.11 cfs @ 2.1 fps)

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#### Pond p02-2:

Proposed culvert under proposed road at intersection with 44.

Inflow Inflow Outfle Prima	ow =	12.02 cfs 12.02 cfs	c, Inflow Depth = 2.37" for 25-yr event @ 12.44 hrs, Volume= 1.533 af @ 12.44 hrs, Volume= 1.533 af, Atten= 0%, Lag= 0.0 min @ 12.44 hrs, Volume= 1.533 af		
Peak Flood Plug-	Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 641.64' @ 12.44 hrs Flood Elev= 645.00' Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= (not calculated)				
#	Routing	Invert	Outlet Devices		
1	Primary	640.00'	<b>24.0" x 100.0' long Culvert</b> CPP, end-section conforming to fill, Ke= 0.500		

**Primary OutFlow** Max=12.02 cfs @ 12.44 hrs HW=641.64' TW=625.60' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 12.02 cfs @ 4.4 fps)

### Pond p02-3:

Outlet Invert= 638.00' S= 0.0200 '/' n= 0.012 Cc= 0.900

Simulates last DMH at bottom of small road, at intersection with 44. This culvert is only used to size the drain pipe under 44.

Inflow	= wc	23.69 cfs 23.69 cfs	c, Inflow Depth = 4.31" for 25-yr event @ 12.02 hrs, Volume= 1.470 af @ 12.02 hrs, Volume= 1.470 af, Atten= 0%, Lag= 0.0 min @ 12.02 hrs, Volume= 1.470 af			
Peak Flooc Plug-	Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 639.93' @ 12.02 hrs Flood Elev= 645.00' Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= (not calculated)					
#	Routing	Invert	Outlet Devices			
1	Primary	635.00'	24.0" x 100.0' long Culvert CPP, projecting, no headwall, Ke= 0.900			

Outlet Invert= 634.00'	S= 0.0100 '/'	n= 0.012	Cc= 0.900

Primary OutFlow Max=23.56 cfs @ 12.02 hrs HW=639.89' TW=554.92' (Dynamic Tailwater) -1=Culvert (Inlet Controls 23.56 cfs @ 7.5 fps)

#### Pond p03-2:

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Inflow Are	a =	4.738 ac, 1	nflow Depth = 4.03"	for 25-yr event	
Inflow	=	24.54 cfs @	12.02 hrs, Volume=	1.592 af	
Outflow	=	3.36 cfs @	12.50 hrs, Volume=	1.588 af,	Atten= 86%, Lag= 28.4 min
Primary	=	3.36 cfs @	12.50 hrs, Volume=	1.588 af	-

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 774.00' Surf.Area= 2,315 sf Storage= 4,095 cf Peak Elev= 778.55' @ 12.50 hrs Surf.Area= 9,503 sf Storage= 36,861 cf (32,766 cf above start) Flood Elev= 779.00' Surf.Area= 9,991 sf Storage= 41,391 cf (37,296 cf above start) Plug-Flow detention time= 345.6 min calculated for 1.494 af (94% of inflow) Center-of-Mass det. time= 278.0 min (1,060.2 - 782.1)

#	Invert	Avail.St	orage S	Storage Des	scription		
1	768.00'	51,3	363 cf	Custom Sta	age Data (Conic	) Listed below	
Elevati		Surf.Area		nc.Store	Cum.Store	Wet.Are	
(te	et)	(sq-ft)	(CU	ibic-feet)	(cubic-feet)	(sq-f	<u>t)</u>
768.	.00	67		0	0	6	57
770.	.00	345		376	376	36	51
772.	.00	729		1,050	1,426	77	7
772.	.50	842		392	1,819	90	)1
774.	.00	2,315		2,277	4,095	2,38	8
774.	.50	5,704		1,942	6,037	5,77	'9
776.	.00	6,996		9,509	15,546	7,13	8
778.	.00	8,917		15,874	31,420	9,16	60
780.	.00	11,064		19,942	51,363	11,42	21
# F	Routing	Invert	Outlet D	evices			
1 F	Primary	774.00'	3.0" Ver	t. Orifice/G	rate C= 0.600		
	Primary	776.20'	6.0" Ver	t. Orifice/G	rate X 2.00 C=	0.600	

3 Primary 778.50' 4.0' long x 6.0' high Sharp-Crested Rectangular Weir 2 End Contraction(s)

Primary OutFlow Max=3.36 cfs @ 12.50 hrs HW=778.55' TW=722.46' (Dynamic Tailwater)

-1=Orifice/Grate (Orifice Controls 0.50 cfs @ 10.1 fps)

-2=Orifice/Grate (Orifice Controls 2.74 cfs @ 7.0 fps)

-3=Sharp-Crested Rectangular Weir (Weir Controls 0.13 cfs @ 0.7 fps)

#### Pond p04-1:

Storage, inverts and culvert length based on assumed grading, check when final grading becomes available

Inflow Area	a =	34.207 ac, Inflow Depth = 2.80"	for 25-yr event
Inflow	=	42.34 cfs @ 12.45 hrs, Volume=	7.994 af
Outflow	=	34.38 cfs @ 12.73 hrs, Volume=	7.994 af, Atten= 19%, Lag= 16.6 min
Primary	=	34.38 cfs @ 12.73 hrs, Volume=	7.994 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

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Peak Elev= 644.17' @ 12.73 hrs Surf.Area= 7,590 sf Storage= 22,701 cf Flood Elev= 648.00' Surf.Area= 15,680 sf Storage= 66,062 cf Plug-Flow detention time= 4.6 min calculated for 7.994 af (100% of inflow) Center-of-Mass det. time= 4.5 min (908.8 - 904.3)

#	Invert	Avail.Sto	orage Storage De	escription		
1	638.00'	66,0	62 cf Custom St	age Data (Conic)	Listed below	
Elev	ation	Surf.Area	Inc.Store	Cum.Store	Wet.Area	
(	(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)	(sq-ft)	
63	38.00	0	0	0	0	
64	10.00	1,300	867	867	1,306	
64	12.00	6,180	6,876	7,743	6,203	
64	14.00	7,270	13,435	21,178	7,438	
64	46.00	11,100	18,235	39,414	11,327	
64	18.00	15,680	26,648	66,062	15,980	
#	Routing	Invert (	Outlet Devices			
1	Primary	638.00' 2	24.0" x 685.0' long	g Culvert CPP, er	nd-section confor	ming to fill, Ke= 0.500

Outlet Invert= 598.00' S= 0.0584 '/' n= 0.012 Cc= 0.900

Primary OutFlow Max=34.38 cfs @ 12.73 hrs HW=644.17' TW=575.39' (Dynamic Tailwater) ←1=Culvert (Inlet Controls 34.38 cfs @ 10.9 fps)

### Pond p06-0:

Field Note #22 Geometry to be confirmed by survey.

Inflow Area =	9.435 ac, Inflow Depth = 2.20"	for 25-yr event
Inflow =	15.39 cfs @ 12.25 hrs, Volume=	1.728 af
Outflow =	13.69 cfs @ 12.35 hrs, Volume=	1.699 af, Atten= 11%, Lag= 5.9 min
Primary =	13.69 cfs @ 12.35 hrs, Volume=	1.699 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 506.80' Surf.Area= 18,600 sf Storage= 42,160 cf Peak Elev= 507.48' @ 12.35 hrs Surf.Area= 21,671 sf Storage= 56,585 cf (14,425 cf above start) Flood Elev= 507.10' Surf.Area= 19,958 sf Storage= 48,537 cf (6,377 cf above start) Plug-Flow detention time= 507.9 min calculated for 0.732 af (42% of inflow) Center-of-Mass det. time= 111.5 min (962.4 - 851.0)

#	Invert	Avail.Stora	age Storage Des	cription		
1	500.00'	67,669	9 cf Custom Sta	ge Data (Conic) Li	sted below	
Eleva (fe	tion eet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
506	).00 5.80 3.00	0 18,600 24,030	0 42,160 25,509	0 42,160 67,669	0 18,672 24,138	

Prop	oosed Cor	nditions_	Type III 24-hr 25-yr Rainfall=5.90"	
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#	Routing	Invert	Outlet Devices	
1	Primary	506.80'	12.0" x 20.0' long Culvert CMP, project	ting, no headwall, Ke= 0.900
	-		Outlet Invert= 506.00' S= 0.0400 '/' n= 0	0.024 Cc= 0.900
2	Primary	507.10'	178.0 deg Sharp-Crested Vee/Trap Weir	C= 2.46

Primary OutFlow Max=13.68 cfs @ 12.35 hrs HW=507.48' TW=504.85' (Dynamic Tailwater) -1=Culvert (Inlet Controls 1.26 cfs @ 2.2 fps) -2=Sharp-Crested Vee/Trap Weir (Weir Controls 12.43 cfs @ 1.5 fps)

#### Pond p07-1:

Field Note # 29 Outlet geometry to be confirmed by survey.

Inflow Area =	26.342 ac, Inflow Depth = 2.56"	for 25-yr event
Inflow =	12.65 cfs @ 12.15 hrs, Volume=	5.613 af
Outflow =	6.83 cfs @ 12.53 hrs, Volume=	5.468 af, Atten= 46%, Lag= 22.9 min
Primary =	6.83 cfs @ 12.53 hrs, Volume=	5.468 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 572.80' Surf.Area= 21,640 sf Storage= 56,264 cf Peak Elev= 573.71' @ 12.53 hrs Surf.Area= 25,939 sf Storage= 78,551 cf (22,287 cf above start) Flood Elev= 573.50' Surf.Area= 24,936 sf Storage= 73,351 cf (17,087 cf above start) Plug-Flow detention time= 431.5 min calculated for 4.175 af (74% of inflow) Center-of-Mass det. time= 81.2 min (1,266.0 - 1,184.8)

#	Invert	Avail.St	orage Storage	rage Storage Description			
1	565.00'	85,5	557 cf Custom	n Stage Data (Conic	c) Listed below		
Elevat (fe	tion eet)	Surf.Area (sq-ft)	Inc.Stor (cubic-fee				
565	,	0		0 0			
572	.80	21,640	56,26	56,264	21,735		
574	.00	27,290	29,29	3 85,557	27,424		
# F	Routing	Invert	Outlet Devices				
1 F	Primary	572.80'	18.0" x 20.0' lo	ong Culvert CMP,	projecting, no head	dwall, Ke= 0.900	
2 F	Primary		Outlet Invert= 572.00' S= 0.0400 '/' n= 0.024 Cc= 0.900 177.0 deg Sharp-Crested Vee/Trap Weir X 2.00 C= 2.46				

Primary OutFlow Max=6.83 cfs @ 12.53 hrs HW=573.71' TW=570.99' (Dynamic Tailwater) -1=Culvert (Inlet Controls 2.89 cfs @ 2.6 fps)

-2=Sharp-Crested Vee/Trap Weir (Weir Controls 3.93 cfs @ 1.1 fps)

#### Pond p08-2:

Inflow Area =	18.762 ac, Inflow Depth = 3.67"	for 25-yr event
Inflow =	68.62 cfs @ 12.04 hrs, Volume=	5.743 af
Outflow =	25.71 cfs @ 12.37 hrs, Volume=	5.063 af, Atten= 63%, Lag= 20.0 min
Primary =	25.71 cfs @ 12.37 hrs, Volume=	5.063 af

Proposed Conditions_10454-01	Type III 24-hr 25-yr Rainfall=5.90"
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Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 550.00' Surf.Area= 8,558 sf Storage= 24,834 cf Peak Elev= 556.55' @ 12.37 hrs Surf.Area= 22,536 sf Storage= 137,013 cf (112,179 cf above start) Flood Elev= 557.00' Surf.Area= 23,344 sf Storage= 147,597 cf (122,763 cf above start) Plug-Flow detention time= 435.9 min calculated for 4.493 af (78% of inflow) Center-of-Mass det. time= 300.4 min (1,088.2 - 787.9)

# Invert	Avail.Storage	Storage Des	scription		
1 544.00'	170,918 cf	Custom Sta	age Data (Conic) Lis	sted below	
Elevation	Surf.Area	Inc.Store	Cum.Store	Wet.Area	
(feet)	(sq-ft) (	( <u>cubic-feet)</u>	(cubic-feet)	(sq-ft)	
544.00	1,962	0	0	1,962	
546.00	3,155	5,070	5,070	3,207	
548.00	4,454	7,572	12,642	4,577	
548.50	4,796	2,312	14,954	4,940	
550.00	8,558	9,880	24,834	8,726	
550.50	12,948	5,339	30,173	13,120	
552.00	15,129	21,037	51,209	15,390	
554.00	18,234	33,315	84,524	18,627	
556.00	21,565	39,752	124,277	22,105	
558.00	25,122	46,642	170,918	25,823	

#	Routing	Invert	Outlet Devices
1	Primary	550.00'	3.0" Vert. Orifice/Grate C= 0.600
2	Primary	554.09'	12.0" Vert. Orifice/Grate X 2.00 C= 0.600
3	Primary	556.00'	11.0' long x 6.0' high Sharp-Crested Rectangular Weir

2 End Contraction(s)

Primary OutFlow Max=25.71 cfs @ 12.37 hrs HW=556.55' TW=515.91' (Dynamic Tailwater) -1=Orifice/Grate (Orifice Controls 0.60 cfs @ 12.2 fps) -2=Orifice/Grate (Orifice Controls 10.58 cfs @ 6.7 fps)

-3=Sharp-Crested Rectangular Weir (Weir Controls 14.53 cfs @ 2.4 fps)

#### Pond p08-3:

Inflow Area =	2.828 ac, Inflow Depth = $3.43$ "	for 25-yr event
Inflow =	8.82 cfs @ 12.02 hrs, Volume=	0.808 af
Outflow =	3.86 cfs @ 12.36 hrs, Volume=	0.807 af, Atten= 56%, Lag= 20.4 min
Primary =	3.86 cfs @ 12.36 hrs, Volume=	0.807 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 528.00' Surf.Area= 1,849 sf Storage= 2,615 cf Peak Elev= 531.33' @ 12.36 hrs Surf.Area= 4,601 sf Storage= 13,378 cf (10,763 cf above start) Flood Elev= 533.00' Surf.Area= 6,389 sf Storage= 22,602 cf (19,987 cf above start) Plug-Flow detention time= 214.4 min calculated for 0.747 af (92% of inflow) Center-of-Mass det. time= 137.8 min (921.4 - 783.6)

#### Proposed Conditions 10454-01

Type III 2	24-hr 25-yr Rainfall=5.90"
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#	Invert	Avail.St	torage	Storage Des	scription		
1	524.00'	28,	956 cf	<b>Custom Sta</b>	ge Data (Conic) L	isted below	
Eleva	ation feet)	Surf.Area (sq-ft)	(c	Inc.Store cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
52	4.00	178		0	0	178	
52	6.00	500		651	651	524	
52	6.50	548		262	913	587	
52	8.00	1,849		1,702	2,615	1,900	
53	0.00	3,344		5,120	7,734	3,437	
53	2.00	5,240		8,513	16,248	5,388	
53	4.00	7,538		12,709	28,956	7,755	
#	Routing	Invert	Outlet I	Devices			
1 2	Primary Primary	528.00' 530.00'			rate C= 0.600 Grate C= 0.600		

Primary OutFlow Max=3.86 cfs @ 12.36 hrs HW=531.33' TW=515.89' (Dynamic Tailwater) -1=Orifice/Grate (Orifice Controls 0.42 cfs @ 8.6 fps) -2=Orifice/Grate (Orifice Controls 3.44 cfs @ 4.4 fps)

#### Pond p09-2:

Inflow Area	a =	21.180 ac, Inflow Depth = 2.85"	for 25-yr event
Inflow	=	40.12 cfs @ 12.29 hrs, Volume=	5.025 af
Outflow	=	3.94 cfs @ 14.80 hrs, Volume=	4.514 af, Atten= 90%, Lag= 150.8 min
Primary	=	3.94 cfs @ 14.80 hrs, Volume=	4.514 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 586.00' Surf.Area= 10,285 sf Storage= 36,340 cf Peak Elev= 592.10' @ 14.80 hrs Surf.Area= 25,957 sf Storage= 162,259 cf (125,919 cf above start) Flood Elev= 593.00' Surf.Area= 27,610 sf Storage= 187,200 cf (150,860 cf above start) Plug-Flow detention time= 646.3 min calculated for 3.679 af (73% of inflow) Center-of-Mass det. time= 441.5 min (1,272.3 - 830.7)

#	Invert	Avail.Storage	e Storage Des	scription		
1	580.00'	214,790 c	f Custom Sta	i <b>ge Data (Conic)</b> Li	sted below	
Elevat (fe	ion et)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
580	.00	3,968	0	0	3,968	
582	.00	5,102	9,046	9,046	5,198	
584	.00	6,343	11,423	20,469	6,550	
584	.50	6,670	3,253	23,722	6,907	
586	.00	10,285	12,619	36,340	10,554	
586	.50	16,887	6,725	43,066	17,159	
588	.00	19,143	27,005	70,070	19,525	
590	.00	22,349	41,451	111,521	22,890	
592	.00	25,781	48,089	159,610	26,494	
594	.00	29,439	55,180	214,790	30,336	

Proposed Conditions\_10454-01Type III 24-hr 25-yrRainfall=5.90"Prepared by The Chazen CompaniesPage 189HydroCAD® 7.00s/n 000927© 1986-2003 Applied Microcomputer Systems4/10/20063:15:45 PM

#	Routing	Invert	Outlet Devices	
1	Primary	586.00'	3.0" Vert. Orifice/Grate C= 0.600	

 1
 Primary
 586.00'
 3.0"
 Vert. Orifice/Grate
 C= 0.600

 2
 Primary
 588.21'
 8.0"
 Vert. Orifice/Grate
 C= 0.600

3 Primary 592.00' 2.0' long x 6.0' high Sharp-Crested Rectangular Weir 2 End Contraction(s)

Primary OutFlow Max=3.94 cfs @ 14.80 hrs HW=592.10' TW=573.67' (Dynamic Tailwater)

-1=Orifice/Grate (Orifice Controls 0.58 cfs @ 11.8 fps)

-2=Orifice/Grate (Orifice Controls 3.17 cfs @ 9.1 fps)

-3=Sharp-Crested Rectangular Weir (Weir Controls 0.19 cfs @ 1.0 fps)

### Pond p10:

Field Note #25 Need to get full story on how this pond works

Inflow Area = Inflow = Outflow =	59.531 ac, Inflow Depth =1.56"for 25-yr event22.95 cfs @12.56 hrs, Volume=7.754 af0.00 cfs @0.00 hrs, Volume=0.000 af, Atten= 100%, Lag= 0.0 min
Starting Elev= 49 Peak Elev= 504.4 Plug-Flow detent	Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs 98.40' Surf.Area= 36,110 sf Storage= 101,108 cf 48' @ 48.00 hrs Surf.Area= 86,113 sf Storage= 438,865 cf (337,757 cf above start) ion time= (not calculated) det. time= (not calculated)

#	Invert	Avail.Stora	ge Storage Des	cription		
1 4	90.00'	581,029	cf Custom Stag	<b>ge Data (Conic)</b> Li	sted below	
Elevation (feet)		Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft <u>)</u>	
490.00		0	0	0	0	
498.40		36,110	101,108	101,108	36,221	
500.00		42,400	62,741	163,849	42,610	
502.00		54,880	97,012	260,861	55,187	
504.00		78,730	132,895	393,755	79,107	
506.00		109,382	187,274	581,029	109,836	

#### Pond p13-1:

No Field Note Natural depression.

Inflow Are	a =	12.222 ac, Inflow Depth = 4.16	6" for 25-yr event	
Inflow	=	53.28 cfs @ 12.04 hrs, Volume	e= 4.237 af	
Outflow	=	48.20 cfs @ 12.07 hrs, Volume:	e= 4.221 af, Atten= 10%, Lag= 1.9 min	
Primary	=	48.20 cfs @ 12.07 hrs, Volume:	e= 4.221 af	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 524.00' Surf.Area= 5,894 sf Storage= 16,480 cf Peak Elev= 526.84' @ 12.07 hrs Surf.Area= 9,818 sf Storage= 39,228 cf (22,748 cf above start) Flood Elev= 527.00' Surf.Area= 10,067 sf Storage= 40,862 cf (24,383 cf above start) Plug-Flow detention time= 192.2 min calculated for 3.843 af (91% of inflow) Center-of-Mass det. time= 113.0 min (884.1 - 771.1)

#	Invert	Avail.S	torage	Storage Des	scription		
1	518.00'	50,	891 cf	Custom Sta	i <b>ge Data (Conic)</b> Li	isted below	
	vation (feet)	Surf.Area (sq-ft)		Inc.Store cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
5	18.00	1,331		0	0	1,331	
52	20.00	2,048		3,353	3,353	2,104	
52	22.00	2,912		4,935	8,288	3,037	
52	22.50	3,150		1,515	9,803	3,294	
52	24.00	5,894		6,676	16,480	6,061	
52	26.00	8,542		14,354	30,834	8,776	
52	28.00	11,592		20,057	50,891	11,908	
#	Routing	Invert	Outlet	Devices			
1 2	Primary Primary	524.00' 525.90'	15.0' lo		rate C= 0.600 h Sharp-Crested F )	Rectangular Weir	

Primary OutFlow Max=48.11 cfs @ 12.07 hrs HW=526.84' TW=501.36' (Dynamic Tailwater) 1=Orifice/Grate (Orifice Controls 0.39 cfs @ 7.9 fps) 2=Sharp-Crested Rectangular Weir (Weir Controls 47.72 cfs @ 3.4 fps)

# Pond p14-1:

Field Note #26 Need to figure out how this pond works

Inflow Area = Inflow = Outflow = Primary =	50.663 ac, Inflow Depth = 3.60"for 25-yr event141.05 cfs @ 12.08 hrs, Volume=15.193 af15.27 cfs @ 13.61 hrs, Volume=5.641 af, Atten= 89%, Lag= 91.3 min15.27 cfs @ 13.61 hrs, Volume=5.641 af
Starting Elev= 4 Peak Elev= 504 Plug-Flow deter	Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs 97.40' Surf.Area= 22,200 sf Storage= 54,760 cf .48' @ 48.00 hrs Surf.Area= 86,011 sf Storage= 470,828 cf (416,068 cf above start) ntion time= 364.2 min calculated for 4.384 af (29% of inflow) det. time= 113.3 min ( 973.6 - 860.3 )

#	Invert	Avail.Storage	Storage Description
1	490.00'	805,062 cf	Custom Stage Data (Conic) Listed below

#### Proposed Conditions\_10454-01

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Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
490.00	0	0	0	0
497.40	22,200	54,760	54,760	22,286
498.00	25,330	14,249	69,009	25,433
500.00	52,810	76,476	145,485	52,948
502.00	73,360	125,608	271,093	73,574
504.00	84,070	157,308	428,402	84,467
506.00	92,130	176,139	604,540	92,797
508.00	108,618	200,522	805,062	109,437

# Routing Invert Outlet Devices

1 Primary 500.00' **24.0" x 80.0' long Culvert** CPP, end-section conforming to fill, Ke= 0.500 Outlet Invert= 502.00' S= -0.0250 '/' n= 0.012 Cc= 0.900

Primary OutFlow Max=15.27 cfs @ 13.61 hrs HW=504.02' TW=501.17' (Dynamic Tailwater)

#### Pond p14-2:

Inflow Area	a =	15.934 ac, Inflow Depth = 4.26"	for 25-yr event	
Inflow	=	69.58 cfs @ 12.05 hrs, Volume=	5.651 af	
Outflow	=	62.82 cfs @ 12.09 hrs, Volume=	5.623 af,	Atten= 10%, Lag= 2.5 min
Primary	=	62.82 cfs @ 12.09 hrs, Volume=	5.623 af	-

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 532.00' Surf.Area= 7,681 sf Storage= 23,903 cf Peak Elev= 534.77' @ 12.09 hrs Surf.Area= 11,998 sf Storage= 51,702 cf (27,799 cf above start) Flood Elev= 535.00' Surf.Area= 12,390 sf Storage= 54,538 cf (30,635 cf above start) Plug-Flow detention time= 179.7 min calculated for 5.074 af (90% of inflow) Center-of-Mass det. time= 96.1 min (866.7 - 770.6)

#	Invert	Avail.St	torage Storage E	Description		
1	526.00'	66,	889 cf Custom S	Stage Data (Conic)	Listed below	
-	ration (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft <u>)</u>	
52	26.00	2,239	0	0	2,239	
52	28.00	3,156	5,369	5,369	3,227	
53	30.00	4,207	7,338	12,707	4,362	
53	30.50	4,491	2,174	14,881	4,669	
53	32.00	7,681	9,023	23,903	7,885	
53	34.00	10,686	18,285	42,188	10,966	
53	36.00	14,093	24,701	66,889	14,463	
#	Routing	Invert	Outlet Devices			
1 2	Primary Primary	532.00' 533.60'		/ert. Orifice/Grate C= 0.600 long x 1.5' high Sharp-Crested Rectangular Weir Contraction(s)		

Primary OutFlow Max=62.81 cfs @ 12.09 hrs HW=534.77' TW=501.49' (Dynamic Tailwater) -1=Orifice/Grate (Orifice Controls 0.38 cfs @ 7.8 fps) -2=Sharp-Crested Rectangular Weir (Weir Controls 62.43 cfs @ 3.9 fps)

### Pond p16-1:

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Field Note # 49 Large pond with man-made island. Geometry to be verified by survey. In particular, we are making big guesses about the outlets. Also need to find out about valves...

Inflow Area =	176.893 ac, Inflow Depth = 2.51"	for 25-yr event
Inflow =	165.02 cfs @ 12.35 hrs, Volume=	36.963 af
Outflow =	38.33 cfs @ 15.19 hrs, Volume=	25.205 af, Atten= 77%, Lag= 170.5 min
Primary =	38.33 cfs @ 15.19 hrs, Volume=	25.205 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 507.00' Surf.Area= 199,799 sf Storage= 878,320 cf Peak Elev= 511.09' @ 15.19 hrs Surf.Area= 297,832 sf Storage= 1,796,794 cf (918,474 cf above start) Flood Elev= 510.50' Surf.Area= 271,550 sf Storage= 1,623,217 cf (744,897 cf above start) Plug-Flow detention time= 1,516.4 min calculated for 5.042 af (14% of inflow) Center-of-Mass det. time= 448.8 min (1,355.3 - 906.6)

# Invert	Avail.Sto	orage Storage Des	scription		
1 500.00'	2,062,0	087 cf Custom Sta	<b>ige Data (Conic)</b> Li	sted below	
	0		0		
Elevation	Surf.Area	Inc.Store	Cum.Store	Wet.Area	
(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)	(sq-ft)	
500.00	0	0	0	0	
503.00	140,344	140,344	140,344	140,358	
509.20	232,500	1,143,862	1,284,206	232,994	
510.00	249,400	192,720	1,476,927	249,951	
512.00	338,000	585,160	2,062,087	338,634	
# Routing	Invert	Outlet Devices			
1 Primary	509.00'	18.0" x 110.0' long	Culvert CMP. pro	piecting, no headw	all. Ke= 0.900
,		Outlet Invert= 505.7		, .	
2 Primary	500.00'	8.0" x 100.0' long a	assumed equalizat	ion pipe w/ valve	X 0.00
		CMP, projecting, no	•		
		Outlet Invert= 500.00' S= 0.0000 '/' n= 0.013 Cc= 0.900			
3 Primary	510.50'	175.0 deg Sharp-Cr	rested Vee/Trap We	eir X 2.00 C= 2.4	6
,		<b>U</b>	•		

Primary OutFlow Max=38.33 cfs @ 15.19 hrs HW=511.09' TW=506.23' (Dynamic Tailwater) -1=Culvert (Inlet Controls 7.79 cfs @ 4.4 fps)

-2=assumed equalization pipe w/ valve (Controls 0.00 cfs)

-3=Sharp-Crested Vee/Trap Weir (Weir Controls 30.55 cfs @ 1.9 fps)

#### Proposed Conditions\_10454-01

# Pond p17-1:

Field Note #45 Golf pond Geometry to be confirmed by surveyed

Inflow Area Inflow Outflow Primary	=	112.240 ac, Inflow Depth = 2.36"for 25-yr event73.46 cfs @ 13.20 hrs, Volume=22.093 af73.45 cfs @ 13.21 hrs, Volume=22.093 af, Atten= 0%, Lag=73.45 cfs @ 13.21 hrs, Volume=22.093 af	= 0.6 min
2			

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 523.80' Surf.Area= 7,290 sf Storage= 9,234 cf Peak Elev= 525.53' @ 13.21 hrs Surf.Area= 11,254 sf Storage= 25,658 cf (16,424 cf above start) Flood Elev= 524.30' Surf.Area= 8,074 sf Storage= 13,623 cf (4,389 cf above start) Plug-Flow detention time= 18.0 min calculated for 21.881 af (99% of inflow) Center-of-Mass det. time= 9.6 min (924.8 - 915.2)

#Inv	vert Avail.S	storage Storag	ge Description		
1 520	00' 30	,224 cf Custo	om Stage Data (Con	ic) Listed below	
Elevation (feet) 520.00 523.80 524.00 526.00	Surf.Area (sq-ft) 7,290 7,300 12,460	) (cubic-fe ) ) 9,2 ) 1,4	(cubic-feet           0         0           234         9,234           159         10,693	t) (sq-ft) 0 0 4 7,313 3 7,374	
# Routin	g Invert	Outlet Device:	S		
1 Primar 2 Primar 3 Primar	y 524.30'	2.2' long x 0.5' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 Coef. (English) 2.80 2.92 3.08 3.30 3.32 143.0 deg Sharp-Crested Vee/Trap Weir C= 2.47 178.0 deg x 60.0' long Sharp-Crested Vee/Trap Weir C= 2.46			

Primary OutFlow Max=73.45 cfs @ 13.21 hrs HW=525.53' TW=516.06' (Dynamic Tailwater) =Broad-Crested Rectangular Weir (Weir Controls 16.66 cfs @ 4.4 fps)

-2=Sharp-Crested Vee/Trap Weir (Weir Controls 12.45 cfs @ 2.7 fps)

-3=Sharp-Crested Vee/Trap Weir (Weir Controls 44.34 cfs @ 1.7 fps)

### Pond p18-1:

Field Note #46 Golf pond Geometry to be confirmed by surveyed

Inflow Area =	131.862 ac, Inflow Depth = 2.39"	for 25-yr event
Inflow =	95.05 cfs @ 12.45 hrs, Volume=	26.219 af
Outflow =	94.10 cfs @ 12.50 hrs, Volume=	26.215 af, Atten= 1%, Lag= 3.3 min
Primary =	94.10 cfs @ 12.50 hrs, Volume=	26.215 af

Proposed Conditions_10454-01	Type III 24-hr 25-yr Rainfall=5.90"
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Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 513.90' Surf.Area= 20,680 sf Storage= 26,884 cf Peak Elev= 516.14' @ 12.50 hrs Surf.Area= 29,289 sf Storage= 82,606 cf (55,722 cf above start) Flood Elev= 514.81' Surf.Area= 23,768 sf Storage= 48,709 cf (21,825 cf above start) Plug-Flow detention time= 47.2 min calculated for 25.592 af (98% of inflow) Center-of-Mass det. time= 27.4 min (940.2 - 912.9)

HI	nvert Avail.	Storage Storage	Description		
1 51	0.00' 14	8,288 cf <b>Custom</b>	Stage Data (Conic	) Listed below	
Elevation (feet)	Surf.Are (sq-f		•••••••	Wet.Area (sq-ft <u>)</u>	
510.00		0 0	0	0	
513.90	20,68	30 26,884	26,884	20,704	
514.00	20,69	90 2,068	28,952	20,756	
516.00	28,29	90 48,782	77,735	28,436	
518.00	42,76	60 70,554	148,288	42,967	
# Routi	ing Invert	Outlet Devices			

r Weir
ir C= 2.46

Primary OutFlow Max=94.09 cfs @ 12.50 hrs HW=516.14' TW=508.74' (Dynamic Tailwater) -1=Broad-Crested Rectangular Weir (Weir Controls 22.23 cfs @ 5.0 fps) -2=Sharp-Crested Vee/Trap Weir (Weir Controls 15.00 cfs @ 2.8 fps)

-3=Sharp-Crested Vee/Trap Weir (Weir Controls 56.85 cfs @ 2.4 fps)

# Pond p19-0:

Wetland Geometry to be confirmed by survey Based off aerial topo, and assumed topo contour

Pond Unchanged from existing to proposed conditions

Inflow Area = Inflow = Outflow = Primary = Secondary =	15.520 ac, Inflow 15.56 cfs @ 12.6 9.63 cfs @ 13.0 9.63 cfs @ 13.0 0.00 cfs @ 0.0	1 hrs, Volume= 1 hrs, Volume= 1 hrs, Volume=	2.401 af	n= 38%, Lag= 24.1 min			
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 972.00' Surf.Area= 86,000 sf Storage= 57,333 cf Peak Elev= 972.21' @ 13.01 hrs Surf.Area= 91,873 sf Storage= 81,241 cf (23,907 cf above start) Plug-Flow detention time= 386.6 min calculated for 1.083 af (45% of inflow) Center-of-Mass det. time= 65.1 min (960.9 - 895.8)							

### Proposed Conditions 10454-01

HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems 4/10/2006 3:15:45 PM							<u>3:15:45 PM</u>
			Storage D				
1 97	0.00' 2	82,329 cf	Custom S	tage Data (Conic	) Listed below		
Elevation (feet)	Surf.Ar (sq		Inc.Store ubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)		
970.00		0	0	0	0		
972.00	86,0	00	57,333	57,333	86,006		
974.00	141,2	70	224,996	282,329	141,327		
# Rout 1 Seco 2 Prima	ndary 973.60	0' <b>178.0 de</b> 0' <b>35.0' lor</b> Head (fe	eg x 51.0' ng x 0.5' b eet) 0.20				

.. . . . .

Primary OutFlow Max=9.63 cfs @ 13.01 hrs HW=972.21' TW=970.17' (Dynamic Tailwater) **2=Broad-Crested Rectangular Weir** (Weir Controls 9.63 cfs @ 1.3 fps)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=972.00' TW=973.60' (Dynamic Tailwater) -1=Sharp-Crested Vee/Trap Weir (Controls 0.00 cfs)

#### Pond p20-1:

Field Note #50 Spring Fed Pond Geometry to be confirmed by surveyed

Inflow Area	a =	207.817 ac, Inflow Depth = 1.87" for 25-yr event	
Inflow	=	44.32 cfs @ 15.13 hrs, Volume= 32.395 af	
Outflow	=	44.11 cfs @ 15.25 hrs, Volume= 31.459 af, Atten= 0%, Lag= 7.4 mir	n
Primary	=	44.11 cfs @ 15.25 hrs, Volume= 31.459 af	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 505.10' Surf.Area= 89,370 sf Storage= 138,524 cf Peak Elev= 506.23' @ 15.25 hrs Surf.Area= 90,518 sf Storage= 240,656 cf (102,132 cf above start) Plug-Flow detention time= 287.0 min calculated for 28.273 af (87% of inflow) Center-of-Mass det. time= 69.7 min (1,357.9 - 1,288.2)

#	Invert	Avail.Storag	ge Storage Des	cription	
1	502.00'	615,682	cf Custom Sta	ge Data (Prismati	c) Listed below
Elevat		Surf.Area	Inc.Store	Cum.Store	
502	eet)	(sq-ft) 0	(cubic-feet) 0	(cubic-feet) 0	
502		89,370	138,524	138,524	
506	.00	89,380	80,437	218,961	
508		99,280	188,660	407,621	
510	.00	108,781	208,061	615,682	

Type III 24-hr 25-yr Rainfall=5.90" Page 195

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#### Proposed Conditions 10454-01

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Type III 24-hr 25-yr Rainfall=5.90" Page 196 HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems 4/10/2006 3:15:45 PM

#	Routing	Invert	Outlet Devices
1	Primary	505.10'	3.0' long x 1.5' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50
			3.00
			Coef. (English) 2.62 2.64 2.64 2.68 2.75 2.86 2.92 3.07 3.07 3.03 3.28
			3.32
2	Primary	506.20'	6.5' long x 1.5' breadth Broad-Crested Rectangular Weir
	-		Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50
			3.00
			Coef. (English) 2.62 2.64 2.64 2.68 2.75 2.86 2.92 3.07 3.07 3.03 3.28
			3.32
~			
3	Primary	506.00'	176.0 deg x 97.0' long Sharp-Crested Vee/Trap Weir C= 2.46

Primary OutFlow Max=44.11 cfs @ 15.25 hrs HW=506.23' TW=505.48' (Dynamic Tailwater)

-1=Broad-Crested Rectangular Weir (Weir Controls 9.34 cfs @ 2.8 fps)

-2=Broad-Crested Rectangular Weir (Weir Controls 0.09 cfs @ 0.5 fps)

-3=Sharp-Crested Vee/Trap Weir (Weir Controls 34.69 cfs @ 1.5 fps)

#### Pond p20-2:

Inflow Are	a =	13.511 ac, Inflow Depth = 4.12"	for 25-yr event
Inflow	=	56.07 cfs @ 12.09 hrs, Volume=	4.639 af
Outflow	=	4.62 cfs @ 13.36 hrs, Volume=	2.971 af, Atten= 92%, Lag= 76.6 min
Primary	=	4.62 cfs @ 13.36 hrs, Volume=	2.971 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 552.00' Surf.Area= 10,535 sf Storage= 35,913 cf Peak Elev= 558.54' @ 13.36 hrs Surf.Area= 24,827 sf Storage= 162,348 cf (126,435 cf above start) Flood Elev= 559.00' Surf.Area= 25,653 sf Storage= 174,016 cf (138,102 cf above start) Plug-Flow detention time= 952.4 min calculated for 2.146 af (46% of inflow) Center-of-Mass det. time= 590.6 min (1,374.0 - 783.4)

#	Invert	Avail.St	orage Storage D	Description		
1	546.00'	199,	647 cf Custom S	Stage Data (Conic)	Listed below	
Eleva (f	ation eet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft <u>)</u>	
546	6.00	3,714	0	0	3,714	
548	8.00	4,960	8,644	8,644	5,044	
550	0.00	6,308	11,241	19,885	6,493	
550	0.50	6,661	3,242	23,127	6,874	
552	2.00	10,535	12,786	35,913	10,779	
552	2.50	15,037	6,360	42,273	15,285	
554	4.00	17,268	24,209	66,483	17,616	
556	6.00	20,441	37,664	104,147	20,935	
558	8.00	23,840	44,237	148,384	24,494	
560	0.00	27,465	51,262	199,647	28,292	
#	Routing	Invert	Outlet Devices			
1	Primary	552.00'	3.0" Vert. Orifice/	<b>Grate</b> C= 0.600		
	Primary	558.20'	6.1' long x 6.2' hi	gh Sharp-Crested	Rectangular Weir	2 End Contraction(s)

Primary OutFlow Max=4.62 cfs @ 13.36 hrs HW=558.54' TW=506.07' (Dynamic Tailwater) 1=Orifice/Grate (Orifice Controls 0.60 cfs @ 12.2 fps) 2=Sharp-Crested Rectangular Weir (Weir Controls 4.02 cfs @ 1.9 fps)

#### Pond p21-1:

Inflow Area	=	459.188 ac, I	nflow Depth = 2.4	.41"	for 25-yr event		
Inflow =	=	421.31 cfs @	12.28 hrs, Volun	me=	92.100 af		
Outflow =	=	32.31 cfs @	20.39 hrs, Volun	me=	79.726 af,	Atten= 92%,	Lag= 486.7 min
Primary =	=	32.31 cfs @	20.39 hrs, Volun	ne=	79.726 af		

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 485.66' @ 20.39 hrs Surf.Area= 1,142,622 sf Storage= 2,187,708 cf Plug-Flow detention time= 797.4 min calculated for 79.710 af (87% of inflow) Center-of-Mass det. time= 649.1 min (1,703.8 - 1,054.7)

#	Invert	Avail.Sto	orage Storage De	escription				
1	480.40'	5,244,8	5,244,885 cf Custom Stage Data (Conic) Listed below					
Eleva		Surf.Area	Inc.Store	Cum.Store	Wet.Area			
(16	eet)	(sq-ft)	(cubic-feet)	(cubic-feet)	(sq-ft)			
480	).40	0	0	0	0			
482	2.00	202,230	107,856	107,856	202,234			
484	1.00	485,198	667,114	774,970	485,231			
486	6.00	1,275,481	1,698,237	2,473,208	1,275,541			
488	3.00	1,499,208	2,771,678	5,244,885	1,499,423			
#	Routing	Invert	Outlet Devices					
1	Primary	480.40'	30.0" x 70.0' long	Culvert CMP, proj	ecting, no headwal	l, Ke= 0.900		

Outlet Invert= 480.40' S= 0.0000 '/' n= 0.024 Cc= 0.900

Primary OutFlow Max=32.31 cfs @ 20.39 hrs HW=485.66' TW=0.00' (Dynamic Tailwater) **1=Culvert** (Barrel Controls 32.31 cfs @ 6.6 fps)

#### Pond p21-4:

Inflow Area	a =	5.152 ac, Inflow Depth = 3.56"	for 25-yr event
Inflow	=	15.10 cfs @ 12.03 hrs, Volume=	= 1.527 af
Outflow	=	10.57 cfs @ 12.26 hrs, Volume=	= 1.516 af, Atten= 30%, Lag= 13.9 min
Primary	=	10.57 cfs @ 12.26 hrs, Volume=	= 1.516 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 496.00' Surf.Area= 5,112 sf Storage= 14,306 cf Peak Elev= 498.82' @ 12.26 hrs Surf.Area= 8,602 sf Storage= 34,058 cf (19,752 cf above start) Flood Elev= 499.00' Surf.Area= 8,847 sf Storage= 35,622 cf (21,317 cf above start) Plug-Flow detention time= 438.3 min calculated for 1.188 af (78% of inflow) Center-of-Mass det. time= 252.8 min (1,044.3 - 791.5)

### Proposed Conditions 10454-01

Type III	24-hr 25-yr Rainfall=5.90"
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toms	1/10/2006 3.15.15 DM

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#	Invert	Avail.St	torage S	Storage Description				
1	490.00'	44,	433 cf <b>C</b>	ustom St	age Data (Conic)	Listed below		
Elev	ation feet)	Surf.Area (sq-ft)		nc.Store bic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)		
	0.00	1,146		0	0	1,146		
-	2.00	1,784		2,907	2,907	1,839		
49	4.00	2,530		4,292	7,199	2,654		
49	4.50	2,733		1,315	8,514	2,876		
49	6.00	5,112		5,791	14,306	5,278		
49	8.00	7,468		12,506	26,812	7,699		
50	0.00	10,226		17,622	44,433	10,536		
#	Routing	Invert	Outlet De					
1	Primary	496.00'			Grate C= 0.600			
2	Primary	498.10'	5.0' long	x 2.0' hig	h Sharp-Crested	Rectangular Weir	2 End Contraction(s)	

Primary OutFlow Max=10.57 cfs @ 12.26 hrs HW=498.82' TW=483.21' (Dynamic Tailwater) -1=Orifice/Grate (Orifice Controls 0.39 cfs @ 7.9 fps) -2=Sharp-Crested Rectangular Weir (Weir Controls 10.18 cfs @ 2.9 fps)

### Pond p21-5:

Inflow Area =	=	2.398 ac, 1	nflow Depth	= 3.20"	for 25-yr event	
Inflow =		7.00 cfs @	12.19 hrs,	Volume=	0.639 af	
Primary =		7.00 cfs @	12.19 hrs,	Volume=	0.639 af,	Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

#### Pond p21-6:

Inflow Are	a =	6.182 ac, Inflow Depth = 3.57"	for 25-yr event	
Inflow	=	16.62 cfs @ 12.23 hrs, Volume=	1.840 af	
Outflow	=	11.40 cfs @ 12.43 hrs, Volume=	1.830 af, Atte	en= 31%, Lag= 12.1 min
Primary	=	11.40 cfs @ 12.43 hrs, Volume=	1.830 af	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 492.00' Surf.Area= 3,323 sf Storage= 4,847 cf Peak Elev= 494.76' @ 12.43 hrs Surf.Area= 13,423 sf Storage= 31,159 cf (26,313 cf above start) Flood Elev= 495.00' Surf.Area= 13,824 sf Storage= 34,456 cf (29,609 cf above start) Plug-Flow detention time= 330.2 min calculated for 1.719 af (93% of inflow) Center-of-Mass det. time= 267.3 min (1,084.9 - 817.6)

#	Invert	Avail.Storage	Storage Description
1	488.00'	48,245 cf	Custom Stage Data (Conic) Listed below

#### Proposed Conditions 10454-01

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Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
(1661)	(54-11)	(Cubic-leet)	(CUDIC-IEEL)	(54-11)
488.00	296	0	0	296
490.00	924	1,162	1,162	946
490.50	1,110	508	1,670	1,141
492.00	3,323	3,177	4,847	3,367
492.50	6,166	2,336	7,182	6,212
494.00	12,147	13,484	20,666	12,214
496.00	15,500	27,579	48,245	15,669

Invert Outlet Devices # Routing 1

Primary 492.00' **3.0" Vert. Orifice/Grate** C= 0.600

5.0' long x 2.0' high Sharp-Crested Rectangular Weir 2 End Contraction(s) 2 Primary 494.00'

Primary OutFlow Max=11.39 cfs @ 12.43 hrs HW=494.76' TW=483.95' (Dynamic Tailwater) -1=Orifice/Grate (Orifice Controls 0.38 cfs @ 7.8 fps)

-2=Sharp-Crested Rectangular Weir (Weir Controls 11.01 cfs @ 3.0 fps)

#### Pond p21-7:

Inflow Area	a =	8.355 ac, Inflow Depth = 3.85"	' for 25-yr event
Inflow	=	30.08 cfs @ 12.04 hrs, Volume=	= 2.679 af
Outflow	=	10.03 cfs @ 12.43 hrs, Volume=	= 2.657 af, Atten= 67%, Lag= 23.7 min
Primary	=	10.03 cfs @ 12.43 hrs, Volume=	= 2.657 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 492.00' Surf.Area= 3,941 sf Storage= 8,984 cf Peak Elev= 498.55' @ 12.43 hrs Surf.Area= 12,617 sf Storage= 61,366 cf (52,382 cf above start) Flood Elev= 499.00' Surf.Area= 13,379 sf Storage= 67,369 cf (58,385 cf above start) Plug-Flow detention time= 440.8 min calculated for 2.450 af (91% of inflow) Center-of-Mass det. time= 348.8 min (1,125.8 - 777.0)

#	Invert	Avail.St	orage	Storage De	scription		
1	486.00'	80,	712 cf	Custom Sta	age Data (Conic)	Listed below	
Eleva (fe	tion eet)	Surf.Area (sq-ft)	(c	Inc.Store	Cum.Store (cubic-feet)	Wet.Area (sq-ft <u>)</u>	
486	6.00	478		0	0	478	
488	3.00	964		1,414	1,414	999	
490	0.00	1,601		2,538	3,952	1,684	
490	).50	1,782		845	4,797	1,879	
492	2.00	3,941		4,187	8,984	4,056	
494	4.00	6,120		9,981	18,965	6,292	
496	6.00	8,702		14,746	33,712	8,944	
498	3.00	11,686		20,315	54,027	12,012	
500	0.00	15,071		26,685	80,712	15,495	
#	Routing	Invert		Devices			
	Primary				rate C= 0.600		
	Primary				rate X 2.00 C= (		
3	Primary	498.00'	5.0' lon	ig x 6.0' high	n Sharp-Crested	Rectangular Weir	2 End Contraction(s)

Primary OutFlow Max=10.03 cfs @ 12.43 hrs HW=498.55' TW=483.95' (Dynamic Tailwater) -1=Orifice/Grate (Orifice Controls 0.60 cfs @ 12.2 fps) -2=Orifice/Grate (Orifice Controls 2.84 cfs @ 7.2 fps) -3=Sharp-Crested Rectangular Weir (Weir Controls 6.59 cfs @ 2.5 fps)

# Pond p22-1:

Field Note #54 Golf Pond Geometry to be confirmed by survey

Pond unchanged from existing to proposed conditions

Inflow Area	a =	78.382 ac, Inflow Depth = 2.50" for 25-yr event	
Inflow	=	120.81 cfs @ 12.29 hrs, Volume= 16.313 af	
Outflow	=	119.41 cfs @ 12.33 hrs, Volume= 16.014 af, Atten= 1	%, Lag= 2.1 min
Primary	=	119.41 cfs @ 12.33 hrs, Volume= 16.014 af	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 498.10' Surf.Area= 6,520 sf Storage= 10,106 cf Peak Elev= 502.10' @ 12.33 hrs Surf.Area= 11,685 sf Storage= 45,533 cf (35,427 cf above start) Plug-Flow detention time= 39.9 min calculated for 15.782 af (97% of inflow) Center-of-Mass det. time= 17.4 min (888.9 - 871.5)

#	Invert	Avail.St	torage Storage D	escription	
1	495.00'	143,	770 cf Custom S	tage Data (Prism	atic) Listed below
	ation	Surf.Area	Inc.Store	Cum.Store	
(	(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)	
49	95.00	0	0	0	
49	98.10	6,520	10,106	10,106	
50	00.00	8,390	14,164	24,270	
50	)2.00	11,530	19,920	44,190	
50	04.00	14,530	26,060	70,250	
50	06.00	18,340	32,870	103,120	
50	00.80	22,310	40,650	143,770	
#	Routing	Invert	Outlet Devices		
1	Primary	499.75'	18.0" x 21.0' long	g Culvert CMP, p	projecting, no headwall, Ke= 0.900
	-		Outlet Invert= 499	.75' S= 0.0000 '/	n= 0.024 Cc= 0.900
2	Primary	500.50'	1.0' long x 15.0' l	preadth Broad-Cr	ested Rectangular Weir
	-				1.00 1.20 1.40 1.60
			Coef. (English) 2.	68 2.70 2.70 2.6	64 2.63 2.64 2.64 2.63
3	Primary	500.50'	20.0' long x 13.5'	breadth Broad-C	Frested Rectangular Weir
	-		Head (feet) 0.20	0.40 0.60 0.80	1.00 1.20 1.40 1.60
			Coef. (English) 2.	62 2.66 2.70 2.6	66 2.65 2.66 2.65 2.63

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Primary OutFlow Max=119.39 cfs @ 12.33 hrs HW=502.10' TW=483.50' (Dynamic Tailwater) -1=Culvert (Barrel Controls 7.31 cfs @ 4.1 fps) -2=Broad-Crested Rectangular Weir (Weir Controls 5.34 cfs @ 3.3 fps) -3=Broad-Crested Rectangular Weir (Weir Controls 106.74 cfs @ 3.3 fps)

### Pond p23-1:

Inflow Area	l =	29.123 ac, Inflow	Depth = 2.91"	for 25-yr event	
Inflow	=	49.78 cfs @ 12.54	1 hrs, Volume=	7.066 af	
Outflow	=	49.74 cfs @ 12.55	5 hrs, Volume=	6.351 af,	Atten= 0%, Lag= 0.5 min
Primary	=	49.74 cfs @ 12.55	5 hrs, Volume=	6.351 af	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 507.90' @ 12.55 hrs Surf.Area= 14,369 sf Storage= 33,460 cf Plug-Flow detention time= 69.7 min calculated for 6.351 af (90% of inflow) Center-of-Mass det. time= 20.5 min (885.0 - 864.5)

#	Invert	Avail.Stora	age Storage Des	scription	
1	503.50'	68,915	5 cf Custom Sta	ge Data (Conic) Li	sted below
Elevat (fe	tion eet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
503	.50	0	0	0	0
504	.00	2,390	398	398	2,390
506.00		9,090	10,761	11,159	9,110
508.00		14,660	23,529	34,688	14,732
510	.00	19,690	34,227	68,915	19,847
# F	Routing	Invert Ou	utlet Devices		

507.70' 178.0 deg x 178.0' long Sharp-Crested Vee/Trap Weir C= 2.46 1 Primary

Primary OutFlow Max=49.73 cfs @ 12.55 hrs HW=507.90' TW=507.20' (Dynamic Tailwater) **1=Sharp-Crested Vee/Trap Weir** (Weir Controls 49.73 cfs @ 1.3 fps)

#### Pond p23-2:

Inflow Area	a =	16.094 ac, Inflow Depth = 4.43"	for 25-yr event
Inflow	=	82.70 cfs @ 12.06 hrs, Volume=	5.943 af
Outflow	=	35.84 cfs @ 12.21 hrs, Volume=	5.103 af, Atten= 57%, Lag= 9.4 min
Primary	=	35.84 cfs @ 12.21 hrs, Volume=	5.103 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 508.00' Surf.Area= 7,318 sf Storage= 15,927 cf Peak Elev= 514.54' @ 12.21 hrs Surf.Area= 23,842 sf Storage= 130,567 cf (114,641 cf above start) Flood Elev= 515.00' Surf.Area= 24,788 sf Storage= 141,986 cf (126,059 cf above start) Plug-Flow detention time= 430.8 min calculated for 4.737 af (80% of inflow) Center-of-Mass det. time= 316.9 min (1,092.1 - 775.2)

Proposed Conditions\_10454-01Type III 24-hPrepared by The Chazen CompaniesHydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems

Type III 24-hr 25-yr Rainfall=5.90" Page 202 4/10/2006 3:15:46 PM

# Invert	Avail.Storage	e Storage Deso	cription		
1 502.00'	166,746 c	of Custom Stag	ge Data (Conic) Lis	sted below	
Elevation	Surf.Area	Inc.Store	Cum.Store	Wet.Area	
(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)	(sq-ft)	
502.00	826	0	0	826	
504.00	1,667	2,444	2,444	1,702	
506.00	2,788	4,407	6,852	2,872	
506.50	3,112	1,474	8,326	3,210	
508.00	7,318	7,601	15,927	7,432	
508.50	12,618	4,924	20,851	12,735	
510.00	15,208	20,839	41,690	15,400	
512.00	18,859	34,002	75,692	19,166	
514.00	22,736	41,535	117,227	23,175	
516.00	26,840	49,519	166,746	27,428	
	- ,	- )	, -	, -	
# Routing	Invert Outle	et Devices			
1 Primary	508.00' <b>3.0"</b>	Vert. Orifice/Gra	ate C= 0.600		
2 Primary		" Vert. Orifice/G	rate X 2.00 C= 0.	600	
3 Primary			Sharp-Crested R		
		nd Contraction(s)		5	
-1=Orifice/Gra -2=Orifice/Gra	ate (Orifice Contr ate (Orifice Contr	rols 0.60 cfs @ 1 rols 9.23 cfs @ 5	2.2 fps)	33.02' (Dynamic Tailwater) 2.4 fps)	
		Pond zDP1	: Design Point	1	
Field note #10. Culvert dimensio	ons to be confirme	ed by survey.			
Inflow Area =       26.658 ac, Inflow Depth =       2.88" for 25-yr event         Inflow =       36.10 cfs @       12.51 hrs, Volume=       6.393 af         Outflow =       36.10 cfs @       12.51 hrs, Volume=       6.393 af, Atten= 0%, Lag= 0.1 min         Primary =       36.10 cfs @       12.51 hrs, Volume=       6.393 af					
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 722.46' @ 12.51 hrs Surf.Area= 148 sf Storage= 150 cf Flood Elev= 727.00' Surf.Area= 1,105 sf Storage= 2,619 cf Plug-Flow detention time= 0.1 min calculated for 6.393 af (100% of inflow) Center-of-Mass det. time= 0.1 min (918.2 - 918.1)					

_	#	Invert	Avail.Storage	Storage Description
	1	720.10'	3,706 cf	Custom Stage Data (Conic) Listed below

### Proposed Conditions 10454-01

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Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
720.10	0	0	0	0
722.00	90	57	57	96
724.00	340	403	460	364
726.00	760	1,072	1,533	815
728.00	1,450	2,173	3,706	1,543

#	Routing	Invert	Outlet Devices
1	Primary	720.10'	<b>42.0" x 120.0' long Culvert</b> CMP, square edge headwall, Ke= 0.500
	-		Outlet Invert= 700.00' S= 0.1675 '/' n= 0.024 Cc= 0.900
2	Primary	727.00'	155.0 deg Sharp-Crested Vee/Trap Weir C= 2.47

Primary OutFlow Max=36.10 cfs @ 12.51 hrs HW=722.46' TW=686.61' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 36.10 cfs @ 5.2 fps)

-2=Sharp-Crested Vee/Trap Weir (Controls 0.00 cfs)

### Pond zDP2: Design Point 2

Field Note #15 Culvert dimensions to be confirmed by survey Overflow to ditch is currently discarded... We may have to model that area...

Inflow Area =	93.367 ac, Inflow Depth = 2.29"	for 25-yr event
Inflow =	92.09 cfs @ 12.87 hrs, Volume=	17.788 af
Outflow =	92.06 cfs @ 12.87 hrs, Volume=	17.788 af, Atten= 0%, Lag= 0.1 min
Discarded =	56.85 cfs @ 12.87 hrs, Volume=	4.219 af
Primary =	35.21 cfs @ 12.87 hrs, Volume=	13.568 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 626.02' @ 12.87 hrs Surf.Area= 1,430 sf Storage= 3,372 cf Flood Elev= 624.50' Surf.Area= 925 sf Storage= 1,728 cf Plug-Flow detention time= 0.5 min calculated for 17.784 af (100% of inflow) Center-of-Mass det. time= 0.5 min ( 900.2 - 899.6 )

#	Invert	Avail.S	torage	Storage Des	scription		
1	619.60'	7,	280 cf	Custom Sta	ige Data (Conic)	_isted below	
	ation (feet)	Surf.Area (sq-ft)		Inc.Store Jbic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
61	19.60	0		0	0	0	
62	20.00	10		1	1	10	
62	22.00	260		214	215	269	
62	24.00	760		976	1,192	793	
62	26.00	1,420		2,146	3,338	1,492	
62	28.00	2,580		3,943	7,280	2,694	
#	Routing	Invert	Outlet D	evices			
1	Primary	619.60'			<b>Culvert</b> RCP, er 0' S= 0.0773 '/'		ming to fill, Ke= 0.500 .900
2	Discarded	624.50'	166.0 de	eg Sharp-Cr	ested Vee/Trap V	<b>Veir</b> C= 2.46	

**Discarded OutFlow** Max=56.84 cfs @ 12.87 hrs HW=626.02' (Free Discharge) **2=Sharp-Crested Vee/Trap Weir** (Weir Controls 56.84 cfs @ 3.0 fps)

Primary OutFlow Max=35.21 cfs @ 12.87 hrs HW=626.02' TW=607.68' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 35.21 cfs @ 11.2 fps)

#### Pond zDP3: Design Point 3

Inflow Are	a =	228.471 ac, Inflow Depth = 18.83"	for 25-yr event
Inflow	=	242.90 cfs @ 12.48 hrs, Volume=	358.444 af
Primary	=	242.90 cfs @ 12.48 hrs, Volume=	358.444 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

#### Pond zDP4: Design Point 4

Inflow Area	a =	459.188 ac, Inflow Depth = 2.08"	for 25-yr event	
Inflow	=	32.31 cfs @ 20.39 hrs, Volume=	= 79.726 af	
Primary	=	32.31 cfs @ 20.39 hrs, Volume=	79.726 af, Atten= 0%, Lag= 0.0 min	n

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

#### Pond zDP5: Design Point 5

Inflow Area =		28.325 ac, Inflow Depth = 2.73"		for 25-yr event		
Inflow	=	50.24 cfs @	12.45 hrs, V	'olume=	6.435 af	
Primary	=	50.24 cfs @	12.45 hrs, V	'olume=	6.435 af,	Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

# Post-Development Conditions 50 year 24 hour Storm Event Model Computations

Proposed Conditions_10454-01         Type III 24-hr 50-yr         Rainfall=6.70"           Prepared by The Chazen Companies         Page 205           HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems         4/10/2006 3:16:00 PM							
Subcatchment s01-0:							
Runoff = 20.20 cfs @ 12.60 hrs, Volume= 3.033 af, Depth= 3.17"							
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 50-yr Rainfall=6.70"							
Area (ac) CN Description 11.485 68							
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)							
42.8 Direct Entry,							
Subcatchment s02-1:							
Runoff = 110.14 cfs @ 12.87 hrs, Volume= 20.488 af, Depth= 2.87"							
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 50-yr Rainfall=6.70"							
Area (ac) CN Description							
85.591 65							
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)							
61.3 Direct Entry,							
Subcatchment s02-2:							
Runoff = 15.25 cfs @ 12.42 hrs, Volume= 1.925 af, Depth= 2.97"							
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 50-yr Rainfall=6.70"							
Area (ac) CN Description							
7.776 66							
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)							
29.4Direct Entry,							
Subcatchment s02-3:							
Runoff = 27.66 cfs @ 12.02 hrs, Volume= 1.730 af, Depth= 5.08"							
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs							

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 50-yr Rainfall=6.70"

Proposed Conditions_10454-01 Prepared by The Chazen Companies HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer System	Type III 24-hr 50-yr Rainfall=6.70" Page 206 ns 4/10/2006 3:16:00 PM							
Area (ac) CN Description								
4.088 86								
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)								
1.6 Direct Entry,								
Subcatchment s03-1:								
Runoff = 23.66 cfs @ 12.41 hrs, Volume= 2.931 af,	Depth= 3.37"							
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, Type III 24-hr 50-yr Rainfall=6.70"	dt= 0.01 hrs							
Area (ac) CN Description								
10.435 70								
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)								
28.8 Direct Entry,								
Subcatchment s03-2:								
Runoff = 15.78 cfs @ 12.02 hrs, Volume= 0.952 af,	Depth= 3.78"							
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, Type III 24-hr 50-yr Rainfall=6.70"	dt= 0.01 hrs							
Area (ac) CN Description								
3.021 74								
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)								
1.5 Direct Entry,								
Subcatchment s03-2(IC): s03-2 Impervious Cover								
Runoff = 12.83 cfs @ 12.02 hrs, Volume= 0.895 af,	Depth= 6.46"							
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, Type III 24-hr 50-yr Rainfall=6.70"	dt= 0.01 hrs							
Area (ac) CN Description								
1.663 98								
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)								
1.5 Direct Entry,								

Proposed Conditions_10454-01Type III 24-hr 50-yrRainfall=6.70"Prepared by The Chazen CompaniesPage 207HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems4/10/2006 3:16:00 PM							
Subcatchment s03-2(OW): s03-2 Open Water							
Runoff = 0.43 cfs @ 12.00 hrs, Volume= 0.030 af, Depth= 6.70"							
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 50-yr Rainfall=6.70"							
Area (ac) CN Description							
0.054 100							
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)							
0.0 Direct Entry,							
Subcatchment s04-1:							
Runoff = 27.83 cfs @ 12.09 hrs, Volume= 1.994 af, Depth= 3.17"							
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 50-yr Rainfall=6.70"							
Area (ac) CN Description							
7.549 68							
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)							
6.1     Direct Entry,							
Subcatchment s05-1:							
Runoff = 14.80 cfs @ 12.21 hrs, Volume= 1.418 af, Depth= 2.49"							
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 50-yr Rainfall=6.70"							
Area (ac) CN Description							
6.842 61							
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)							
14.4   Direct Entry,							
Subcatchment s06-0:							
Runoff = 18.93 cfs @ 12.25 hrs, Volume= 1.938 af, Depth= 2.58"							
Runoff by SCS TR-20 method. UH=SCS. Time Span= 0.00-48.00 hrs. dt= 0.01 hrs							

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 50-yr Rainfall=6.70"

Proposed Conditions_10454-01 Prepared by The Chazen Companies HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Syste	Type III 24-hr 50-yr         Rainfall=6.70"           Page 208           ms         4/10/2006         3:16:00 PM						
Area (ac) CN Description 9.007 62							
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)							
17.3 Direct Entry,							
Subcatchment s06-0(OW): s06 Open Water							
Runoff = 3.44 cfs @ 12.00 hrs, Volume= 0.239 at	f, Depth= 6.70"						
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs Type III 24-hr 50-yr Rainfall=6.70"	, dt= 0.01 hrs						
Area (ac) CN Description 0.428 100							
Subcatchment s07-1:							
Runoff = 13.29 cfs @ 12.14 hrs, Volume= 1.077 at	f, Depth= 2.78"						
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs Type III 24-hr 50-yr Rainfall=6.70"	, dt= 0.01 hrs						
Area (ac) CN Description							
4.656 64							
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)							
9.3 Direct Entry,							
Subcatchment s07-1(OW): s07 Ope	en Water						
Runoff = 4.07 cfs @ 12.00 hrs, Volume= 0.283 at	f, Depth= 6.70"						
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs Type III 24-hr 50-yr Rainfall=6.70"	, dt= 0.01 hrs						
Area (ac) CN Description							
0.506 100							
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)							
0.0 Direct Entry,							

Proposed Conditions_10454-01Type III 24-hr 50-yrRainfall=6.70"Prepared by The Chazen CompaniesPage 209HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems4/10/2006 3:16:00 PM								
Subcatchment s08-1:								
Runoff = 38.51 cfs @ 12.36 hrs, Volume= 4.613 af, Depth= 2.39"								
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 50-yr Rainfall=6.70"								
Area (ac) CN Description 23.126 60								
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)								
24.5Direct Entry,								
Subcatchment s08-2:								
Runoff = 22.97 cfs @ 12.17 hrs, Volume= 1.999 af, Depth= 2.68"								
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 50-yr Rainfall=6.70"								
Area (ac) CN Description								
8.958 63								
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)								
11.4 Direct Entry,								
Subcatchment s08-2(IC):								
Runoff = 40.56 cfs @ 12.04 hrs, Volume= 2.974 af, Depth= 6.46"								
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 50-yr Rainfall=6.70"								
Area (ac) CN Description								
5.524 98								
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)								
2.9 Direct Entry,								
Subcatchment s08-2(OW):								
Runoff = 1.54 cfs @ 12.00 hrs, Volume= 0.107 af, Depth= 6.70"								
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 50-yr Rainfall=6.70"								

Proposed Conditions_10454-01 7 Prepared by The Chazen Companies HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems	ype III 24-hr 50-yr Rainfall=6.70" Page 210 4/10/2006 3:16:00 PM
Area (ac) CN Description	
0.192 100	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
0.0 Direct Entry,	
Subcatchment s08-3:	
Runoff = 3.76 cfs @ 12.20 hrs, Volume= 0.352 af,	Depth= 2.49"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, d Type III 24-hr 50-yr Rainfall=6.70"	t= 0.01 hrs
Area (ac) CN Description	
1.700 61	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
13.6 Direct Entry,	
Subcatchment s08-3(IC): s08-3 Impervic	ous Cover
Runoff = 8.50 cfs @ 12.01 hrs, Volume= 0.585 af,	Depth= 6.46"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, d Type III 24-hr 50-yr Rainfall=6.70"	t= 0.01 hrs
Area (ac) CN Description	
1.086 98	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
1.0Direct Entry,	
Subcatchment s08-3(OW): s08-3 Oper	n Water
Runoff = 0.34 cfs @ 12.00 hrs, Volume= 0.023 af,	Depth= 6.70"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, d Type III 24-hr 50-yr Rainfall=6.70"	t= 0.01 hrs
Area (ac) CN Description	
0.042 100	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
0.0 Direct Entry,	

Proposed Conditions_10454-01Type III 24-hr 50-yrRainfall=6.70"Prepared by The Chazen CompaniesPage 211HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems4/10/2006 3:16:00 PM
Subcatchment s09-1:
Runoff = 6.34 cfs @ 12.13 hrs, Volume= 0.519 af, Depth= 2.39"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 50-yr Rainfall=6.70"
Area (ac) CN Description 2.604 60
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
9.0 Direct Entry,
Subcatchment s09-2:
Runoff = 44.22 cfs @ 12.29 hrs, Volume= 4.760 af, Depth= 3.07"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 50-yr Rainfall=6.70"
Area (ac) CN Description
18.608 67
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
20.6 Direct Entry,
Subcatchment s09-2(IC): s09-2 Impervious Cover
Runoff = 17.39 cfs @ 12.04 hrs, Volume= 1.258 af, Depth= 6.46"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 50-yr Rainfall=6.70"
Area (ac) CN Description
2.336 98
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
2.5 Direct Entry,
Subcatchment s09-2(OW): s09-2 Open Water
Runoff = 1.90 cfs @ 12.00 hrs, Volume= 0.132 af, Depth= 6.70"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 50-yr Rainfall=6.70"

Proposed Conditions_10454-01Type III 24-hr 50-yrRainfall=6.70Prepared by The Chazen CompaniesPage 212HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems4/10/2006 3:16:00 PM
Area (ac) CN Description
0.236 100
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
0.0 Direct Entry,
Subcatchment s09-3:
Runoff = 11.98 cfs @ 12.15 hrs, Volume= 1.008 af, Depth= 3.17"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 50-yr Rainfall=6.70"
Area (ac) CN Description
3.818 68
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
10.9   Direct Entry,
Subcatchment s10-1:
Runoff = 17.32 cfs @ 12.40 hrs, Volume= 2.123 af, Depth= 3.17"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 50-yr Rainfall=6.70"
Area (ac) CN Description
8.038 68
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
27.9 Direct Entry,
Subcatchment s10-1(OW): s10 Open Water
Runoff = 6.68 cfs @ 12.00 hrs, Volume= 0.463 af, Depth= 6.70"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 50-yr Rainfall=6.70"
Area (ac) CN Description 0.830 100

Proposed Conditions_10454-01 Prepared by The Chazen Companies HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer	<i>Type III 24-hr 50-yr Rainfall=6.70"</i> Page 213 <u>Systems 4/10/2006 3:16:01 PM</u>
Subcatchment s13-	1:
Runoff = 11.30 cfs @ 12.05 hrs, Volume= 0.7	'37 af, Depth= 2.49"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.0 Type III 24-hr 50-yr Rainfall=6.70"	0 hrs, dt= 0.01 hrs
Area (ac) CN Description	
3.555 61	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
2.8 Direct Entry,	
Subcatchment s13-1(IC): s13-1 Im	pervious Cover
Runoff = 46.87 cfs @ 12.04 hrs, Volume= 3.4	24 af, Depth= 6.46"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.0 Type III 24-hr 50-yr Rainfall=6.70"	0 hrs, dt= 0.01 hrs
Area (ac) CN Description	
6.360 98	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
2.8 Direct Entry,	
Subcatchment s13-1(OW): s13-	1 Open Water
Runoff = 1.05 cfs @ 12.00 hrs, Volume= 0.0	073 af, Depth= 6.70"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.0 Type III 24-hr 50-yr Rainfall=6.70"	0 hrs, dt= 0.01 hrs
Area (ac) CN Description	
0.131 100	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
0.0 Direct Entry,	
Subcatchment s14-	1:
Runoff = 30.54 cfs @ 12.40 hrs, Volume= 3.7	740 af, Depth= 3.27"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.0 Type III 24-hr 50-yr Rainfall=6.70"	0 hrs, dt= 0.01 hrs

Proposed Conditions_10454-01Type III 24-hr 50-yrRainfall=6.70"Prepared by The Chazen CompaniesPage 214HydroCAD® 7.00s/n 000927© 1986-2003 Applied Microcomputer Systems4/10/20063:16:01 PM
Area (ac) CN Description 13.727 69
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
28.1 Direct Entry,
Subcatchment s14-1(IC): s14-1 Impervious Cover
Runoff = 13.81 cfs @ 12.03 hrs, Volume= 0.991 af, Depth= 6.46"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 50-yr Rainfall=6.70"
Area (ac) CN Description
1.840 98
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
2.3 Direct Entry,
Subcatchment s14-1(OW): s14 Open Water
Runoff = 4.17 cfs @ 12.00 hrs, Volume= 0.289 af, Depth= 6.70"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 50-yr Rainfall=6.70"
Area (ac) CN Description
0.518 100
Subcatchment s14-2:
Runoff = 1.41 cfs @ 12.10 hrs, Volume= 0.104 af, Depth= 2.49"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 50-yr Rainfall=6.70"
Area (ac) CN Description
0.504 61
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
6.4 Direct Entry,

Proposed Conditions_10454-01 Prepared by The Chazen Companies HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer System	Type III 24-hr 50-yr Rainfall=6.70" Page 215 ns 4/10/2006 3:16:01 PM
Subcatchment s14-2(OW): s14-2 Ope	en Water
Runoff = 1.42 cfs @ 12.00 hrs, Volume= 0.098 af,	Depth= 6.70"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, Type III 24-hr 50-yr Rainfall=6.70"	dt= 0.01 hrs
Area (ac) CN Description 0.176 100	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
0.0 Direct Entry,	
Subcatchment s14-3:	
Runoff = 23.82 cfs @ 12.12 hrs, Volume= 1.851 af,	Depth= 3.27"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, Type III 24-hr 50-yr Rainfall=6.70"	dt= 0.01 hrs
Area (ac) CN Description 6.794 69	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
8.5 Direct Entry,	
Subcatchment s14-3(IC): s14-3 Imperv	ious Cover
Runoff = $62.12 \text{ cfs} @ 12.04 \text{ hrs}$ , Volume= $4.555 \text{ af}$ ,	Depth= 6.46"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, Type III 24-hr 50-yr Rainfall=6.70"	dt= 0.01 hrs
Area (ac) CN Description 8.460 98	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
2.9 Direct Entry,	
Subcatchment s16-1:	
	Depth= 3.07"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, Type III 24-hr 50-yr Rainfall=6.70"	dt= 0.01 hrs

Proposed Conditions_10454-01 Prepared by The Chazen Companies HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Syste	Type III 24-hr 50-yr         Rainfall=6.70"           Page 216           ms         4/10/2006         3:16:01 PM
	4, 10, 2000 0.10.011 M
Area (ac) CN Description	
39.680 67	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
19.0 Direct Entry,	
Subcatchment s16-1(OW): s16-1 Op	ben Water
Runoff = 43.04 cfs @ 12.00 hrs, Volume= 2.988 af	f, Depth= 6.70"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs Type III 24-hr 50-yr Rainfall=6.70"	, dt= 0.01 hrs
Area (ac) CN Description	
5.351 100	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
0.0 Direct Entry,	
Subcatchment s16-2:	
Runoff = 7.54 cfs @ 12.21 hrs, Volume= 0.724 af	f, Depth= 3.99"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs Type III 24-hr 50-yr Rainfall=6.70"	, dt= 0.01 hrs
Area (ac) CN Description	
2.176 76	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
15.8 Direct Entry,	
Subcatchment s17-1:	
Runoff = 10.43 cfs @ 12.50 hrs, Volume= 1.413 af	f, Depth= 2.78"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs Type III 24-hr 50-yr Rainfall=6.70"	, dt= 0.01 hrs
Area (ac) CN Description	
6.110 64	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
33.8 Direct Entry,	

Proposed Conditions_10454-01 7 Prepared by The Chazen Companies HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems	ype III 24-hr 50-yr Rainfall=6.70" Page 217 s 4/10/2006 3:16:01 PM
Subcatchment s17-1(OW): s17-1 Ope	n Water
Runoff = 1.32 cfs @ 12.00 hrs, Volume= 0.092 af,	Depth= 6.70"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, d Type III 24-hr 50-yr Rainfall=6.70"	lt= 0.01 hrs
Area (ac) CN Description	
0.164 100	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
0.0 Direct Entry,	
Subcatchment s17-2:	
Runoff = 76.21 cfs @ 13.29 hrs, Volume= 18.835 af,	Depth= 2.97"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, d Type III 24-hr 50-yr Rainfall=6.70"	It= 0.01 hrs
Area (ac) CN Description	
76.086 66	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
95.7 Direct Entry,	
Subcatchment s17-3:	
Runoff = 57.57 cfs @ 12.45 hrs, Volume= 7.397 af,	Depth= 2.97"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, d Type III 24-hr 50-yr Rainfall=6.70"	lt= 0.01 hrs
Area (ac) CN Description	
29.880 66	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
30.7 Direct Entry,	
Subcatchment s18-1:	
Runoff = 19.65 cfs @ 12.23 hrs, Volume= 1.949 af,	Depth= 2.78"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs. d	lt= 0.01 hrs

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 50-yr Rainfall=6.70"

Proposed Conditions_10454-01Type III 24Prepared by The Chazen CompaniesHydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems	<i>hr 50-yr Rainfall=6.70"</i> Page 218 4/10/2006 3:16:01 PM
	<u></u>
Area (ac) CN Description 8.429 64	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
16.4 Direct Entry,	
Subcatchment s18-1(OW): s18-1 Open Water	
Runoff = 3.80 cfs @ 12.00 hrs, Volume= 0.264 af, Depth= 6.	70"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 50-yr Rainfall=6.70"	S
Area (ac) CN Description 0.472 100	
Subcatchment s18-2:	
Runoff = 28.80 cfs @ 12.26 hrs, Volume= 2.921 af, Depth= 3.	27"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 50-yr Rainfall=6.70"	8
Area (ac) CN Description 10.721 69	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
18.0Direct Entry,	
Subcatchment s19-0:	
Runoff = 20.51 cfs @ 12.61 hrs, Volume= 3.096 af, Depth= 2.	.39"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 50-yr Rainfall=6.70"	8
Area (ac) CN Description	
15.520 60	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
40.4 Direct Entry,	

Proposed Conditions_10454-01 Prepared by The Chazen Companies HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer System	Type III 24-hr 50-yr         Rainfall=6.70"           Page 219           tems         4/10/2006 3:16:01 PM
Subcatchment s20-1:	
Runoff = 19.27 cfs @ 12.30 hrs, Volume= 2.119	af, Depth= 2.97"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hr Type III 24-hr 50-yr Rainfall=6.70"	s, dt= 0.01 hrs
Area (ac) CN Description	
8.559 66	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
21.5 Direct Entry,	
Subcatchment s20-1(OW): s20-1 C	pen Water
Runoff = 15.83 cfs @ 12.00 hrs, Volume= 1.099	af, Depth= 6.70"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hr Type III 24-hr 50-yr Rainfall=6.70"	s, dt= 0.01 hrs
Area (ac) CN Description	
1.968 100	
Subcatchment s20-2:	
Runoff = 33.31 cfs @ 12.12 hrs, Volume= 2.571	af, Depth= 3.78"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hr Type III 24-hr 50-yr Rainfall=6.70"	s, dt= 0.01 hrs
Area (ac) CN Description	
8.157 74	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
8.4 Direct Entry,	
Subcatchment s20-2(IC): s20-2 Impe	rvious Cover
Runoff = 35.17 cfs @ 12.07 hrs, Volume= 2.752	af, Depth= 6.46"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hr Type III 24-hr 50-yr Rainfall=6.70"	s, dt= 0.01 hrs
Area (ac) CN Description	
5.112 98	

Proposed Conditions_10454-01Type III 24-hr 50-yrRainfall=6.70"Prepared by The Chazen CompaniesPage 220HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems4/10/2006 3:16:01 PM
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
4.7 Direct Entry,
Subcatchment s20-2(OW): s20-2 Open Water
Runoff = 1.95 cfs @ 12.00 hrs, Volume= 0.135 af, Depth= 6.70"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 50-yr Rainfall=6.70"
Area (ac) CN Description
0.242 100
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
0.0 Direct Entry,
Subcatchment s20-3:
Runoff = 17.57 cfs @ 12.30 hrs, Volume= 1.934 af, Depth= 3.37"
Runoff = $17.57 \text{ cfs} @ 12.30 \text{ hrs}$ , Volume= $1.934 \text{ at}$ , Depth= $3.37^{\circ}$
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 50-yr Rainfall=6.70"
Area (ac) CN Description
6.886 70
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
22.0 Direct Entry,
Subcatchment s21-1:
Runoff = 171.73 cfs @ 12.23 hrs, Volume= 16.887 af, Depth= 3.17"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 50-yr Rainfall=6.70"
Area (ac) CN Description
63.942 68
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
16.6 <b>Direct Entry</b> ,

Proposed Conditions_10454-01Type III 24-hr 50-yrRainfall=6.70"Prepared by The Chazen CompaniesPage 221HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems4/10/2006 3:16:01 PM
Subcatchment s21-1(OW):
Runoff = 98.41 cfs @ 12.00 hrs, Volume= 6.831 af, Depth= 6.70"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 50-yr Rainfall=6.70"
Area (ac) CN Description 12.235 100
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
0.0 Direct Entry,
Subcatchment s21-2:
Runoff = 45.70 cfs @ 12.45 hrs, Volume= 5.882 af, Depth= 3.37"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 50-yr Rainfall=6.70"
Area (ac) CN Description
20.941 70
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
31.4 Direct Entry,
Subcatchment s21-3:
Runoff = 31.99 cfs @ 12.16 hrs, Volume= 2.700 af, Depth= 3.78"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 50-yr Rainfall=6.70"
Area (ac) CN Description
8.567 74
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
11.2 Direct Entry,
Subcatchment s21-4:
Runoff = 9.45 cfs @ 12.19 hrs, Volume= 0.868 af, Depth= 3.07"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 50-yr Rainfall=6.70"

Proposed Conditions_10454-01Type III 24-hr 50-yrRainfall=6.70"Prepared by The Chazen CompaniesPage 222HydroCAD® 7.00s/n 000927© 1986-2003 Applied Microcomputer Systems4/10/20063:16:02 PM
Area (ac) CN Description 3.392 67
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
13.8 Direct Entry,
Subcatchment s21-4(IC): s21-4 Impervious Cover
Runoff = 12.63 cfs @ 12.02 hrs, Volume= 0.885 af, Depth= 6.46"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 50-yr Rainfall=6.70"
Area (ac) CN Description 1.643 98
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
1.6Direct Entry,
Subcatchment s21-4(OW): s21-4 Open Water
Runoff = 0.94 cfs @ 12.00 hrs, Volume= 0.065 af, Depth= 6.70"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 50-yr Rainfall=6.70"
Area (ac) CN Description 0.117 100
Subcatchment s21-5:
Runoff = 8.52 cfs @ 12.19 hrs, Volume= 0.777 af, Depth= 3.89"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 50-yr Rainfall=6.70"
Area (ac) CN Description
2.398 75
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
13.9 Direct Entry,

Proposed Conditions_10454-01Type III 24-hr 50-yrRainfall=6.70"Prepared by The Chazen CompaniesPage 223HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems4/10/2006 3:16:02 PM				
Subcatchment s21-6:				
Runoff = 18.22 cfs @ 12.23 hrs, Volume= 1.817 af, Depth= 3.99"				
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 50-yr Rainfall=6.70"				
Area (ac) CN Description				
5.463 76				
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)				
17.5   Direct Entry,				
Subcatchment s21-6(IC): s21-6 Impervious Cover				
Runoff = 4.96 cfs @ 12.02 hrs, Volume= 0.346 af, Depth= 6.46"				
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 50-yr Rainfall=6.70"				
Area (ac) CN Description				
0.643 98				
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)				
1.5 Direct Entry,				
Subcatchment s21-6(OW): s21-6 Open Water				
Runoff = 0.61 cfs @ 12.00 hrs, Volume= 0.042 af, Depth= 6.70"				
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 50-yr Rainfall=6.70"				
Area (ac) CN Description				
0.076 100				
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)				
0.0 Direct Entry,				
Subcatchment s21-7:				
Runoff = 11.11 cfs @ 12.19 hrs, Volume= 1.012 af, Depth= 2.78"				
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 50-yr Rainfall=6.70"				

Proposed Conditions_10454-01	Type III 24-hr 50-yr Rainfall=6.70"
Prepared by The Chazen Companies HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Syste	Page 224 ems 4/10/2006 3:16:02 PM
Area (ac) CN Description 4.375 64	
4.373 04	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
13.1 Direct Entry,	
Subcatchment s21-7(IC): s21-7 Imperv	vious Cover
Runoff = 28.96 cfs @ 12.04 hrs, Volume= 2.094 at	f, Depth= 6.46"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs Type III 24-hr 50-yr Rainfall=6.70"	, dt= 0.01 hrs
Area (ac) CN Description	
3.890 98	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
2.5 <b>Direct Entry</b> ,	
Subcatchment s21-7(OW): s21-7 Op	oen Water
Runoff = $0.72 \text{ cfs} @ 12.00 \text{ hrs}, \text{ Volume} = 0.050 \text{ at}$	f, Depth= 6.70"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs Type III 24-hr 50-yr Rainfall=6.70"	, dt= 0.01 hrs
Area (ac) CN Description	
0.090 100	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
0.0 Direct Entry,	
Subsetshment 22.4	
Subcatchment s22-1:	
Runoff = 57.17 cfs @ 12.20 hrs, Volume= 5.326 at	f, Depth= 3.57"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs Type III 24-hr 50-yr Rainfall=6.70"	, dt= 0.01 hrs
Area (ac) CN Description	
17.878 72	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
14.7 Direct Entry,	

Proposed Conditions_10454-01 Prepared by The Chazen Companies HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Syste	Type III 24-hr 50-yr         Rainfall=6.70"           Page 225           ems         4/10/2006 3:16:02 PM
Subcatchment s22-1(OW): s22-1 O	ben Water
Runoff = 1.09 cfs @ 12.00 hrs, Volume= 0.076 a	f, Depth= 6.70"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs Type III 24-hr 50-yr Rainfall=6.70"	s, dt= 0.01 hrs
Area (ac) CN Description 0.136 100	
Subcatchment s22-2:	
Runoff = 103.33 cfs @ 12.35 hrs, Volume= 11.844 a	f, Depth= 3.17"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs Type III 24-hr 50-yr Rainfall=6.70"	s, dt= 0.01 hrs
Area (ac) CN Description	
44.848 68	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
24.0Direct Entry,	
Subcatchment s23-1:	
Runoff = 61.35 cfs @ 12.53 hrs, Volume= 8.676 a	f, Depth= 3.57"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs Type III 24-hr 50-yr Rainfall=6.70"	s, dt= 0.01 hrs
Area (ac) CN Description	
29.123 72	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
38.5 Direct Entry,	
Subcatchment s23-2:	
Runoff = 45.04 cfs @ 12.06 hrs, Volume= 2.984 a	f, Depth= 4.10"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs Type III 24-hr 50-yr Rainfall=6.70"	s, dt= 0.01 hrs
Area (ac) CN Description 8.741 77	

Proposed Conditions_10454-01Type III 24-hr 50-yrRainfall=6.70Prepared by The Chazen CompaniesPage 226HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems4/10/2006 3:16:02 PM
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
4.0 Direct Entry,
Subcatchment s23-2(IC): s23-2 Impervious Cover
Runoff = 50.68 cfs @ 12.06 hrs, Volume= 3.869 af, Depth= 6.46"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 50-yr Rainfall=6.70"
Area (ac) CN Description
7.185 98
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
4.0 Direct Entry,
Subcatchment s23-2(OW): s23-2 Open Water
Runoff = 1.35 cfs @ 12.00 hrs, Volume= 0.094 af, Depth= 6.70"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 50-yr Rainfall=6.70"
Area (ac) CN Description
0.168 100
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
0.0 Direct Entry,
Subcatchment s24-0:
Runoff = 62.42 cfs @ 12.45 hrs, Volume= 7.956 af, Depth= 3.37"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 50-yr Rainfall=6.70"
Area (ac) CN Description
28.325 70
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
30.7 Direct Entry,

Proposed Conditions_10454-01Type III 24-hr 50-yrRainfall=6.70"Prepared by The Chazen CompaniesPage 227HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems4/10/2006 3:16:02 PM
Subcatchment s25-0:
Runoff = 31.32 cfs @ 12.28 hrs, Volume= 3.357 af, Depth= 2.97"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 50-yr Rainfall=6.70"
Area (ac) CN Description 13.562 66
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
20.2 Direct Entry,
Reach 25R:
Overland Flow Reach
Inflow Area =       15.520 ac, Inflow Depth =       2.39" for 50-yr event         Inflow =       13.58 cfs @       12.98 hrs, Volume=       3.094 af         Outflow =       13.50 cfs @       13.04 hrs, Volume=       3.094 af, Atten= 1%, Lag= 3.3 min
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 2.4 fps, Min. Travel Time= 4.3 min Avg. Velocity = 0.7 fps, Avg. Travel Time= 15.0 min
Peak Depth= 0.31' @ 13.04 hrs Capacity at bank full= 175.17 cfs Inlet Invert= 560.00', Outlet Invert= 512.00' 50.00' x 1.00' deep Parabolic Channel, n= 0.060 Length= 620.0' Slope= 0.0774 '/'
Reach r03-1:
Overland Flow Reach Requires more survey
Inflow Area =       11.485 ac, Inflow Depth = 3.17"       for 50-yr event         Inflow =       20.20 cfs @       12.60 hrs, Volume=       3.033 af         Outflow =       20.10 cfs @       12.63 hrs, Volume=       3.033 af, Atten= 0%, Lag= 1.5 min
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 5.8 fps, Min. Travel Time= 2.2 min Avg. Velocity = 2.2 fps, Avg. Travel Time= 5.9 min
Peak Depth= 0.74' @ 12.63 hrs Capacity at bank full= 92.14 cfs Inlet Invert= 845.00', Outlet Invert= 728.00' 10.00' x 1.50' deep Parabolic Channel, n= 0.060 Length= 785.0' Slope= 0.1490 '/'

# Reach r04-1:

Channel

Inflow Area = Inflow = Outflow =				
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 8.4 fps, Min. Travel Time= 0.7 min Avg. Velocity = 2.2 fps, Avg. Travel Time= 2.5 min				
Peak Depth= 1.26' @ 12.48 hrs Capacity at bank full= 530.15 cfs Inlet Invert= 685.50', Outlet Invert= 632.00' 12.00' x 4.00' deep Parabolic Channel, n= 0.060 Length= 330.0' Slope= 0.1621 '/'				
Reach r08-1a:				
Man Made Ditch Inverts of pipe to	be surveyed			

Inflow Area =	=	93.367 ac, Inflow Depth	n = 2.02"	for 50-yr event	
Inflow =		35.96 cfs @ 12.82 hrs,	Volume=	15.703 af	
Outflow =	•	35.96 cfs @ 12.83 hrs,	Volume=	15.703 af,	Atten= 0%, Lag= 0.4 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 9.6 fps, Min. Travel Time= 0.4 min Avg. Velocity = 5.5 fps, Avg. Travel Time= 0.7 min

Peak Depth= 0.68' @ 12.83 hrs Capacity at bank full= 81.88 cfs Inlet Invert= 607.00', Outlet Invert= 587.00' 10.00' x 1.00' deep Parabolic Channel, n= 0.027 Length= 226.0' Slope= 0.0885 '/'

## Reach r08-1b:

24" HDPE Inverts to be surveyed

Inflow Area = 93.367 ac, Inflow Depth = 2.02" for 50-yr event Inflow = 35.96 cfs @ 12.83 hrs, Volume= 15.703 af Outflow = 35.96 cfs @ 12.83 hrs, Volume= 15.703 af, Atten= 0%, Lag= 0.2 min Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 24.1 fps, Min. Travel Time= 0.2 min Avg. Velocity = 14.5 fps, Avg. Travel Time= 0.3 min

## Proposed Conditions\_10454-01 Type III Prepared by The Chazen Companies HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems

Peak Depth= 0.96' @ 12.83 hrs Capacity at bank full= 77.17 cfs Inlet Invert= 587.00', Outlet Invert= 557.75' 24.0" Diameter Pipe n= 0.012 Length= 295.0' Slope= 0.0992 '/'

# Reach r08-1c:

Ditch Pipe inverts to be surveyed

Inflow Area	a =	93.367 ac, Inflow Dept	h = 2.02"	for 50-yr event	
Inflow	=	35.96 cfs @ 12.83 hrs,	Volume=	15.703 af	
Outflow	=	35.96 cfs @ 12.86 hrs,	Volume=	15.703 af,	Atten= 0%, Lag= 1.6 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 9.1 fps, Min. Travel Time= 1.1 min Avg. Velocity = 5.1 fps, Avg. Travel Time= 1.9 min

Peak Depth= 0.70' @ 12.86 hrs Capacity at bank full= 76.65 cfs Inlet Invert= 557.75', Outlet Invert= 512.00' 10.00' x 1.00' deep Parabolic Channel, n= 0.027 Length= 590.0' Slope= 0.0775 '/'

# Reach r08-1d: Amenia Creek/Cascade Brook

(Steam) parallel to Route 22 Base Flow estimated from field observation (see field note 21)

Inflow Area = 114.957 ac, Inflow Depth = 18.95" for 50-yr event Inflow = 117.74 cfs @ 12.29 hrs, Volume= 181.498 af, Incl. 40.00 cfs Base Flow Outflow = 116.65 cfs @ 12.35 hrs, Volume= 181.253 af, Atten= 1%, Lag= 3.4 min Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 4.0 fps, Min. Travel Time= 3.3 min Avg. Velocity = 3.1 fps, Avg. Travel Time= 4.3 min

Peak Depth= 4.23' @ 12.35 hrs Capacity at bank full= 104.49 cfs Inlet Invert= 512.00', Outlet Invert= 504.00' 10.00' x 4.00' deep Parabolic Channel, n= 0.060 Length= 805.0' Slope= 0.0099 '/'

# Reach r13-1:

 Inflow Area =
 2.176 ac, Inflow Depth = 3.99" for 50-yr event

 Inflow =
 7.54 cfs @ 12.21 hrs, Volume=
 0.724 af

 Outflow =
 7.48 cfs @ 12.24 hrs, Volume=
 0.724 af, Atten= 1%, Lag= 1.3 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 9.6 fps, Min. Travel Time= 1.6 min Avg. Velocity = 3.5 fps, Avg. Travel Time= 4.2 min

## Proposed Conditions\_10454-01 Type In Prepared by The Chazen Companies HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems

Peak Depth= 0.68' @ 12.24 hrs Capacity at bank full= 17.79 cfs Inlet Invert= 546.00', Outlet Invert= 524.00' 18.0" Diameter Pipe n= 0.012 Length= 900.0' Slope= 0.0244 '/'

## Reach r14-3a:

30" HDPE Under Main Entrance Road

	a = = =	18.15 cfs @	nflow Depth = 12.16 hrs, Vo 12.16 hrs, Vo	olume=	for 50-yr event 1.528 af 1.528 af,	Atten= 0%,	Lag= 0.4 min
Routing by Dyn-Stor-Ind method. Time Span- 0.00-48.00 brs. dt- 0.01 brs							

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 14.9 fps, Min. Travel Time= 0.5 min Avg. Velocity = 5.5 fps, Avg. Travel Time= 1.3 min

Peak Depth= 0.74' @ 12.16 hrs Capacity at bank full= 94.91 cfs Inlet Invert= 526.00', Outlet Invert= 505.70' 30.0" Diameter Pipe n= 0.012 Length= 445.0' Slope= 0.0456 '/'

## Reach r14-3b:

Grass lined channel

Inflow Area = 6.422 ac, Inflow Depth = 2.85" for 50-yr event 18.24 cfs @ 12.15 hrs, Volume= Inflow 1.528 af = Outflow = 18.15 cfs @ 12.16 hrs, Volume= 1.528 af, Atten= 0%, Lag= 0.6 min Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 7.1 fps, Min. Travel Time= 0.8 min Avg. Velocity = 2.6 fps, Avg. Travel Time= 2.3 min Peak Depth= 0.76' @ 12.16 hrs Capacity at bank full= 325.42 cfs Inlet Invert= 542.00', Outlet Invert= 526.00'

10.00' x 3.00' deep Parabolic Channel, n= 0.027 Length= 360.0' Slope= 0.0444 '/'

## Reach r17-1:

 Inflow Area =
 76.086 ac, Inflow Depth =
 2.97"
 for 50-yr event

 Inflow =
 76.21 cfs @
 13.29 hrs, Volume=
 18.835 af

 Outflow =
 75.95 cfs @
 13.33 hrs, Volume=
 18.835 af, Atten= 0%, Lag= 2.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 8.8 fps, Min. Travel Time= 2.6 min Avg. Velocity = 3.4 fps, Avg. Travel Time= 6.8 min

Proposed Conditions_10454-01 Prepared by The Chazen Companies HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Sy	Type III 24-hr 50-yr         Rainfall=6.70"           Page 231           stems         4/10/2006         3:16:03 PM
Peak Depth= 1.33' @ 13.33 hrs Capacity at bank full= 181.28 cfs Inlet Invert= 646.00', Outlet Invert= 524.00' 12.00' x 2.00' deep Parabolic Channel, n= 0.045 Length= 1,390	0.0' Slope= 0.0878 '/'
Reach r18-2:	
Overland Flow Reach	
Inflow         =         0.00 cfs @         0.00 hrs, Volume=         0.000           Outflow         =         0.00 cfs @         0.00 hrs, Volume=         0.000	) af ) af, Atten= 0%, Lag= 0.0 min
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0 Max. Velocity= 0.0 fps, Min. Travel Time= 0.0 min Avg. Velocity = 0.0 fps, Avg. Travel Time= 0.0 min	0.01 hrs
Peak Depth= 0.00' @ 0.00 hrs Capacity at bank full= 434.91 cfs Inlet Invert= 973.60', Outlet Invert= 630.00' 50.00' x 1.00' deep Parabolic Channel, n= 0.060 Length= 720.0	0' Slope= 0.4772 '/'
Reach r21-1a:	
Man Made Ditch	
Inflow Area =       207.817 ac, Inflow Depth =       2.44"       for 50-yr ev         Inflow =       82.48 cfs @       14.29 hrs, Volume=       42.185         Outflow =       82.46 cfs @       14.31 hrs, Volume=       42.171	
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0 Max. Velocity= 7.6 fps, Min. Travel Time= 1.4 min Avg. Velocity = 3.3 fps, Avg. Travel Time= 3.3 min	0.01 hrs
Peak Depth= 2.00' @ 14.31 hrs Capacity at bank full= 191.76 cfs Inlet Invert= 504.00', Outlet Invert= 494.00' 10.00' x 3.00' deep Parabolic Channel, n= 0.027 Length= 648.0	0' Slope= 0.0154 '/'
Reach r21-1b:	
Overland Flow Reach	
Inflow Area = $29.123 \text{ ac}$ , Inflow Depth = $3.28"$ for 50-yr evInflow = $61.30 \text{ cfs}$ @ $12.54 \text{ hrs}$ , Volume= $7.960$ Outflow = $61.27 \text{ cfs}$ @ $12.55 \text{ hrs}$ , Volume= $7.960$	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 4.6 fps, Min. Travel Time= 0.6 min Avg. Velocity = 1.7 fps, Avg. Travel Time= 1.5 min Proposed Conditions 10454-01 Prepared by The Chazen Companies HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems

Peak Depth= 0.55' @ 12.55 hrs Capacity at bank full= 227.81 cfs Inlet Invert= 506.70', Outlet Invert= 485.75' 50.00' x 1.00' deep Parabolic Channel, n= 0.060 Length= 160.0' Slope= 0.1309 '/'

## Reach r22-2:

**Overland Flow Reach** 

Inflow Area = Inflow = Outflow =	15.520 ac, Inflow Depth =2.39"for 50-yr event13.60 cfs @12.96 hrs, Volume=3.094 af13.58 cfs @12.98 hrs, Volume=3.094 af, Atten= 0%, Lag= 1.6 min	
Max. Velocity= 4	Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs I.7 fps, Min. Travel Time= 2.2 min I.4 fps, Avg. Travel Time= 7.4 min	
Peak Depth= 0.1 Capacity at bank Inlet Invert= 970.		

50.00' x 1.00' deep Parabolic Channel, n= 0.060 Length= 630.0' Slope= 0.5556 '/'

Ditch Pipe inverts need to be surveyed

Inflow Area	a =	67.391 ac, Inflow Depth = 3	.22" for 50-yr event
Inflow	=	56.48 cfs @ 12.40 hrs, Volu	me= 18.081 af
Outflow	=	56.45 cfs @ 12.41 hrs, Volu	me= 18.078 af, Atten= 0%, Lag= 1.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 10.5 fps, Min. Travel Time= 1.7 min Avg. Velocity = 3.7 fps, Avg. Travel Time= 5.0 min

Peak Depth= 1.09' @ 12.41 hrs Capacity at bank full= 205.50 cfs Inlet Invert= 570.00', Outlet Invert= 504.00' 10.00' x 2.00' deep Parabolic Channel, n= 0.027 Length= 1,090.0' Slope= 0.0606 '/'

## Reach r25-0b: Wetland Reach

Wetland Reach Has wetland vegetation within reach

Inflow Are	a =	9.435 ac, I	nflow Depth = 2.73	for 50-yr event	
Inflow	=	18.58 cfs @	12.32 hrs, Volume	= 2.148 af	
Outflow	=	17.11 cfs @	12.40 hrs, Volume	= 2.147 af,	Atten= 8%, Lag= 5.3 min

Reach r25-0a:

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Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 2.0 fps, Min. Travel Time= 6.2 min Avg. Velocity = 0.5 fps, Avg. Travel Time= 26.8 min

Peak Depth= 1.07' @ 12.40 hrs Capacity at bank full= 156.51 cfs Inlet Invert= 504.00', Outlet Invert= 499.50' 20.00' x 3.00' deep Parabolic Channel, n= 0.045 Length= 750.0' Slope= 0.0060 '/'

## Reach r25-0c: Amenia Creek/Cascade Brook

(Steam) parallel to Route 22 Base Flow estimated from field observation (see field note 21)

138.083 ac, Inflow Depth = 29.95" for 50-yr event Inflow Area = Inflow 195.13 cfs @ 12.36 hrs, Volume= 344.577 af, Incl. 40.00 cfs Base Flow = 190.12 cfs @ 12.45 hrs, Volume= Outflow 343.893 af, Atten= 3%, Lag= 5.6 min = Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 3.0 fps, Min. Travel Time= 5.4 min Avg. Velocity = 2.7 fps, Avg. Travel Time= 6.1 min Peak Depth= 7.63' @ 12.45 hrs Capacity at bank full= 67.14 cfs Inlet Invert= 504.00', Outlet Invert= 500.00' 10.00' x 4.00' deep Parabolic Channel, n= 0.060 Length= 975.0' Slope= 0.0041 '/' Pond 8P:

No field note. Water spills over cart path; no storage.

Inflow Area =	41.049 ac, Inflow Depth = 3.29"	for 50-yr event
Inflow =	46.03 cfs @ 12.47 hrs, Volume=	11.249 af
Outflow =	46.03 cfs @ 12.47 hrs, Volume=	11.249 af, Atten= 0%, Lag= 0.0 min
Primary =	46.03 cfs @ 12.47 hrs, Volume=	11.249 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 575.45' @ 12.47 hrs Flood Elev= 574.70' Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= (not calculated)

#	Routing	Invert	Outlet Devices
1	Primary	574.70'	177.0 deg Sharp-Crested Vee/Trap Weir C= 2.46

Primary OutFlow Max=46.03 cfs @ 12.47 hrs HW=575.45' TW=571.09' (Dynamic Tailwater) ←1=Sharp-Crested Vee/Trap Weir (Weir Controls 46.03 cfs @ 2.1 fps)

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# Pond p02-2:

Proposed culvert under proposed road at intersection with 44.

Inflov Inflov Outfl Prima	ow =	15.25 cfs 15.25 cfs	c, Inflow Depth = 2.97" for 50-yr event @ 12.42 hrs, Volume= 1.925 af @ 12.42 hrs, Volume= 1.925 af, Atten= 0%, Lag= 0.0 min @ 12.42 hrs, Volume= 1.925 af				
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 642.02' @ 12.42 hrs Flood Elev= 645.00' Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= (not calculated)							
#	Routing	Invert	Outlet Devices				
1	Primary	640.00'	<b>24.0"</b> x <b>100.0' long Culvert</b> CPP, end-section conforming to fill, Ke= 0.500 Outlet Invert= 638.00' S= 0.0200 '/' n= 0.012 Cc= 0.900				

Primary OutFlow Max=15.24 cfs @ 12.42 hrs HW=642.02' TW=625.83' (Dynamic Tailwater)

# Pond p02-3:

Simulates last DMH at bottom of small road, at intersection with 44. This culvert is only used to size the drain pipe under 44.

Inflow Inflow Outflo Prima	w =	27.66 cfs 27.66 cfs	c, Inflow Depth = 5.08"       for 50-yr event         @ 12.02 hrs, Volume=       1.730 af         @ 12.02 hrs, Volume=       1.730 af, Atten= 0%, Lag= 0.0 min         @ 12.02 hrs, Volume=       1.730 af			
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 641.36' @ 12.02 hrs Flood Elev= 645.00' Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= (not calculated)						
#	Routing	Invert	Outlet Devices			
1	Primary	635.00'	24.0" x 100.0' long Culvert CPP, projecting, no headwall, Ke= 0.900			

5	· ·	<b>j</b> 0,	,	
Outlet Invert= 634.00'	S= 0.0100 '/'	n= 0.012	Cc = 0.900	

Primary OutFlow Max=27.51 cfs @ 12.02 hrs HW=641.31' TW=555.62' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 27.51 cfs @ 8.8 fps)

## Pond p03-2:

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Inflow Area	a =	4.738 ac, 1	nflow Depth = 4.76"	for 50-yr event	
Inflow	=	28.98 cfs @	12.02 hrs, Volume=	1.878 af	
Outflow	=	6.85 cfs @	12.37 hrs, Volume=	1.873 af,	Atten= 76%, Lag= 21.0 min
Primary	=	6.85 cfs @	12.37 hrs, Volume=	1.873 af	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 774.00' Surf.Area= 2,315 sf Storage= 4,095 cf Peak Elev= 778.91' @ 12.37 hrs Surf.Area= 9,892 sf Storage= 40,474 cf (36,379 cf above start) Flood Elev= 779.00' Surf.Area= 9,991 sf Storage= 41,391 cf (37,296 cf above start) Plug-Flow detention time= 310.0 min calculated for 1.779 af (95% of inflow) Center-of-Mass det. time= 252.1 min (1,031.9 - 779.8)

# Inv	vert Avail.	Storage	Storage De	scription	
1 768.	.00' 5'	1,363 cf	Custom St	age Data (Conic)	Listed below
Elevation (feet)	Surf.Are (sq-f		Inc.Store ubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
768.00	6	7	0	0	67
770.00	34	5	376	376	361
772.00	72	9	1,050	1,426	777
772.50	84	2	392	1,819	901
774.00	2,31	5	2,277	4,095	2,388
774.50	5,70	4	1,942	6,037	5,779
776.00	6,99	6	9,509	15,546	7,138
778.00	8,91	7	15,874	31,420	9,160
780.00	11,06	4	19,942	51,363	11,421
# Routing	g Invert	Outlet [	Devices		
1 Primar	y 774.00'	3.0" Ve	rt. Orifice/C	irate C= 0.600	
2 Drimor	776 20'	6 0" Vo	rt OrificalC	roto V 2 00 C- (	0.00

2 Primary 776.20' 6.0" Vert. Orifice/Grate X 2.00 C= 0.600

3 Primary 778.50' 4.0' long x 6.0' high Sharp-Crested Rectangular Weir 2 End Contraction(s)

Primary OutFlow Max=6.85 cfs @ 12.37 hrs HW=778.91' TW=722.80' (Dynamic Tailwater)

-1=Orifice/Grate (Orifice Controls 0.52 cfs @ 10.5 fps)

-2=Orifice/Grate (Orifice Controls 2.96 cfs @ 7.5 fps)

-3=Sharp-Crested Rectangular Weir (Weir Controls 3.37 cfs @ 2.1 fps)

## Pond p04-1:

Storage, inverts and culvert length based on assumed grading, check when final grading becomes available

Inflow Are	ea =	34.207 ac, Inflow Depth = 3.45"	for 50-yr event
Inflow	=	55.55 cfs @ 12.43 hrs, Volume=	9.831 af
Outflow	=	40.31 cfs @ 12.77 hrs, Volume=	9.831 af, Atten= 27%, Lag= 20.6 min
Primary	=	40.31 cfs @ 12.77 hrs, Volume=	9.831 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

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Peak Elev= 646.10' @ 12.77 hrs Surf.Area= 11,332 sf Storage= 40,766 cf Flood Elev= 648.00' Surf.Area= 15,680 sf Storage= 66,062 cf Plug-Flow detention time= 6.7 min calculated for 9.829 af (100% of inflow) Center-of-Mass det. time= 6.7 min (898.8 - 892.1)

#	Invert	Avail.Sto	orage Stora	ge De	scription		
1	638.00'	66,0	62 cf Custo	om Sta	age Data (Conic	) Listed below	
Eleva		Surf.Area	Inc.St		Cum.Store	Wet.Area	
	feet)	<u>(sq-ft)</u>	(cubic-fe		(cubic-feet)	(sq-ft)	
	8.00	0		0	0	0	
	0.00	1,300		867	867	1,306	
64	2.00	6,180	6,8	876	7,743	6,203	
64	4.00	7,270	13,4	435	21,178	7,438	
64	6.00	11,100	18,	235	39,414	11,327	
64	8.00	15,680	26,	648	66,062	15,980	
#	Routing	Invert	Outlet Device	S			
1	Primary	638.00'	24.0" x 685.0	)' long	Culvert CPP,	end-section confo	rming to fill, Ke= 0.500

Outlet Invert= 598.00' S= 0.0584 '/' n= 0.012 Cc= 0.900

Primary OutFlow Max=40.31 cfs @ 12.77 hrs HW=646.10' TW=575.44' (Dynamic Tailwater) ←1=Culvert (Inlet Controls 40.31 cfs @ 12.8 fps)

## Pond p06-0:

Field Note #22 Geometry to be confirmed by survey.

Inflow Area	a =	9.435 ac, Inflow Depth	= 2.77"	for 50-yr event	
Inflow	=	19.92 cfs @ 12.25 hrs,	Volume=	2.177 af	
Outflow	=	18.58 cfs @ 12.32 hrs,	Volume=	2.148 af,	Atten= 7%, Lag= 4.3 min
Primary	=	18.58 cfs @ 12.32 hrs,	Volume=	2.148 af	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 506.80' Surf.Area= 18,600 sf Storage= 42,160 cf Peak Elev= 507.53' @ 12.32 hrs Surf.Area= 21,907 sf Storage= 57,694 cf (15,534 cf above start) Flood Elev= 507.10' Surf.Area= 19,958 sf Storage= 48,537 cf (6,377 cf above start) Plug-Flow detention time= 376.2 min calculated for 1.180 af (54% of inflow) Center-of-Mass det. time= 92.9 min ( 939.1 - 846.1 )

#	lnvert	Avail.Stora	ige Storage Des	scription		
1	500.00'	67,669	ocf Custom Sta	ge Data (Conic) Li	sted below	
Ele	evation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
5	500.00 506.80 508.00	0 18,600 24,030	0 42,160 25,509	42,160 67,669	0 18,672 24,138	

Pro	posed Co	nditions_	10454-01	Type III 24	Type III 24-hr 50-yr Rainfall=6.70"		
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Hydro	DCAD® 7.00	s/n 000927	© 1986-2003 Applied Microcom	nputer Systems	4/10/2006 3:16:03 PM		
#	Routing	Invert	Outlet Devices				
1	Primary	506.80'	12.0" x 20.0' long Culvert	CMP, projecting, no hea	dwall, Ke= 0.900		

	i iiiiai y	500.00	
			Outlet Invert= 506.00' S= 0.0400 '/' n= 0.024 Cc= 0.900
2	Primary	507.10'	178.0 deg Sharp-Crested Vee/Trap Weir C= 2.46

Primary OutFlow Max=18.57 cfs @ 12.32 hrs HW=507.53' TW=505.01' (Dynamic Tailwater) -1=Culvert (Inlet Controls 1.41 cfs @ 2.3 fps) -2=Sharp-Crested Vee/Trap Weir (Weir Controls 17.16 cfs @ 1.6 fps)

## Pond p07-1:

Field Note # 29 Outlet geometry to be confirmed by survey.

Inflow Area =	26.342 ac, Inflow Depth = 3.18"	for 50-yr event
Inflow =	16.63 cfs @ 12.14 hrs, Volume=	6.979 af
Outflow =	10.58 cfs @ 12.38 hrs, Volume=	6.832 af, Atten= 36%, Lag= 14.4 min
Primary =	10.58 cfs @ 12.38 hrs, Volume=	6.832 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 572.80' Surf.Area= 21,640 sf Storage= 56,264 cf Peak Elev= 573.77' @ 12.38 hrs Surf.Area= 26,224 sf Storage= 80,032 cf (23,768 cf above start) Flood Elev= 573.50' Surf.Area= 24,936 sf Storage= 73,351 cf (17,087 cf above start) Plug-Flow detention time= 347.3 min calculated for 5.539 af (79% of inflow) Center-of-Mass det. time= 68.3 min (1,210.6 - 1,142.3)

#	Invert	Avail.St	orage	ge Storage Description					
1	565.00'	85,	557 cf	cf Custom Stage Data (Conic) Listed below					
Eleva (fe	tion eet)	Surf.Area (sq-ft)		Inc.Store Jbic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)			
565	5.00	0		0	0	0			
572	2.80	21,640		56,264	56,264	21,735			
574	4.00	27,290		29,293	85,557	27,424			
#	Routing	Invert	Outlet D	evices					
1	Primary	572.80'				jecting, no headwall			
2	Primary	573.50'		Outlet Invert= 572.00' S= 0.0400 '/' n= 0.024 Cc= 0.900 177.0 deg Sharp-Crested Vee/Trap Weir X 2.00 C= 2.46					

Primary OutFlow Max=10.58 cfs @ 12.38 hrs HW=573.77' TW=571.09' (Dynamic Tailwater) -1=Culvert (Inlet Controls 3.22 cfs @ 2.7 fps)

-2=Sharp-Crested Vee/Trap Weir (Weir Controls 7.36 cfs @ 1.3 fps)

## Pond p08-2:

Inflow Area	=	18.762 ac, Inflow Depth = 4.36"	for 50-yr event	
Inflow	=	80.52 cfs @ 12.04 hrs, Volume=	6.811 af	
Outflow	=	40.24 cfs @ 12.27 hrs, Volume=	6.125 af, Atten= 50%, La	ag= 14.0 min
Primary	=	40.24 cfs @ 12.27 hrs, Volume=	6.125 af	-

Proposed Conditions_10454-01	Type III 24-hr 50-yr Rainfall=6.70"
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Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 550.00' Surf.Area= 8,558 sf Storage= 24,834 cf Peak Elev= 556.85' @ 12.27 hrs Surf.Area= 23,078 sf Storage= 144,110 cf (119,276 cf above start) Flood Elev= 557.00' Surf.Area= 23,344 sf Storage= 147,597 cf (122,763 cf above start) Plug-Flow detention time= 372.1 min calculated for 5.555 af (82% of inflow) Center-of-Mass det. time= 257.9 min (1,043.9 - 786.0)

# Invert	Avail.Storage	Storage Des	scription		
1 544.00'	170,918 cf	Custom Sta	age Data (Conic) Lis	sted below	
Elevation (feet)		Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
544.00	1,962	0	0	1,962	
546.00	3,155	5,070	5,070	3,207	
548.00	4,454	7,572	12,642	4,577	
548.50	4,796	2,312	14,954	4,940	
550.00	8,558	9,880	24,834	8,726	
550.50	12,948	5,339	30,173	13,120	
552.00	15,129	21,037	51,209	15,390	
554.00	18,234	33,315	84,524	18,627	
556.00	21,565	39,752	124,277	22,105	
558.00	25,122	46,642	170,918	25,823	

#	Routing	Invert	Outlet Devices
1	Primary	550.00'	3.0" Vert. Orifice/Grate C= 0.600
2	Primary	554.09'	12.0" Vert. Orifice/Grate X 2.00 C= 0.600
3	Primary	556.00'	11.0' long x 6.0' high Sharp-Crested Rectangular Weir

2 End Contraction(s)

Primary OutFlow Max=40.23 cfs @ 12.27 hrs HW=556.85' TW=516.17' (Dynamic Tailwater) 1=Orifice/Grate (Orifice Controls 0.61 cfs @ 12.5 fps) 2=Orifice/Grate (Orifice Controls 11.37 cfs @ 7.2 fps)

-2=Orifice/Grate (Orifice Controls 11.37 cfs @ 7.2 fps)

-3=Sharp-Crested Rectangular Weir (Weir Controls 28.25 cfs @ 3.1 fps)

## Pond p08-3:

Inflow Are	a =	2.828 ac, Inflow Depth = 4.08"	" for 50-yr event
Inflow	=	10.30 cfs @ 12.02 hrs, Volume=	= 0.961 af
Outflow	=	4.63 cfs @ 12.36 hrs, Volume=	= 0.960 af, Atten= 55%, Lag= 20.7 min
Primary	=	4.63 cfs @ 12.36 hrs, Volume=	= 0.960 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 528.00' Surf.Area= 1,849 sf Storage= 2,615 cf Peak Elev= 531.72' @ 12.36 hrs Surf.Area= 4,977 sf Storage= 15,065 cf (12,451 cf above start) Flood Elev= 533.00' Surf.Area= 6,389 sf Storage= 22,602 cf (19,987 cf above start) Plug-Flow detention time= 194.3 min calculated for 0.900 af (94% of inflow) Center-of-Mass det. time= 128.0 min (911.2 - 783.1)

Type III 2	4-hr 50-yr Rainfall=6.70"
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#	Invert	Avail.S	torage Storage D	escription		
1	524.00'		<u> </u>	tage Data (Conic)	Listed below	
Elevat (fe	ion eet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
524	.00	178	0	0	178	
526	.00	500	651	651	524	
526	.50	548	262	913	587	
528	.00	1,849	1,702	2,615	1,900	
530	.00	3,344	5,120	7,734	3,437	
532	.00	5,240	8,513	16,248	5,388	
534	.00	7,538	12,709	28,956	7,755	
# F	Routing	Invert	Outlet Devices			
1 F	Primary	528.00'	3.0" Vert. Orifice/	Grate C= 0.600		
	Primary	530.00'	12.0" Vert. Orifice	<b>/Grate</b> C= 0.600		

Primary OutFlow Max=4.63 cfs @ 12.36 hrs HW=531.72' TW=516.23' (Dynamic Tailwater) -1=Orifice/Grate (Orifice Controls 0.45 cfs @ 9.1 fps) -2=Orifice/Grate (Orifice Controls 4.18 cfs @ 5.3 fps)

## Pond p09-2:

Inflow Area =	21.180 ac, Inflow Depth = 3.48"	for 50-yr event
Inflow =	50.05 cfs @ 12.29 hrs, Volume=	6.149 af
Outflow =	7.72 cfs @ 13.37 hrs, Volume=	5.620 af, Atten= 85%, Lag= 64.8 min
Primary =	7.72 cfs @ 13.37 hrs, Volume=	5.620 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 586.00' Surf.Area= 10,285 sf Storage= 36,340 cf Peak Elev= 592.71' @ 13.37 hrs Surf.Area= 27,079 sf Storage= 179,197 cf (142,857 cf above start) Flood Elev= 593.00' Surf.Area= 27,610 sf Storage= 187,200 cf (150,860 cf above start) Plug-Flow detention time= 557.8 min calculated for 4.786 af (78% of inflow) Center-of-Mass det. time= 392.3 min (1,219.6 - 827.3)

#	Invert	Avail.Storag	e Storage Des	cription				
1	580.00'	214,790	cf Custom Sta	Custom Stage Data (Conic) Listed below				
Elevat (fe	ion et)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft <u>)</u>			
580	.00	3,968	0	0	3,968			
582	.00	5,102	9,046	9,046	5,198			
584	.00	6,343	11,423	20,469	6,550			
584	.50	6,670	3,253	23,722	6,907			
586	.00	10,285	12,619	36,340	10,554			
586	.50	16,887	6,725	43,066	17,159			
588	.00	19,143	27,005	70,070	19,525			
590	.00	22,349	41,451	111,521	22,890			
592	.00	25,781	48,089	159,610	26,494			
594	.00	29,439	55,180	214,790	30,336			

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#	Routing	Invert	Outlet Devices
1	Primary	586.00'	3.0" Vert. Orifice/Grate C= 0.600
2	Primary	588.21'	8.0" Vert. Orifice/Grate C= 0.600
3	Primary	592.00'	2.0' long x 6.0' high Sharp-Crested Rectangular Weir 2 End Contraction(s)

Primary OutFlow Max=7.72 cfs @ 13.37 hrs HW=592.71' TW=573.76' (Dynamic Tailwater)

-1=Orifice/Grate (Orifice Controls 0.61 cfs @ 12.4 fps)

-2=Orifice/Grate (Orifice Controls 3.43 cfs @ 9.8 fps)

-3=Sharp-Crested Rectangular Weir (Weir Controls 3.69 cfs @ 2.8 fps)

## Pond p10:

Field Note #25 Need to get full story on how this pond works

Inflow Area = Inflow = Outflow =	59.531 ac, Inflow Depth =1.91"for50-yr event33.13 cfs @12.47 hrs, Volume=9.481 af0.00 cfs @0.00 hrs, Volume=0.000 af, Atten= 100%, Lag= 0.0 min
Starting Elev= 49 Peak Elev= 505.2 Plug-Flow detent	Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs 98.40' Surf.Area= 36,110 sf Storage= 101,108 cf 29' @ 48.00 hrs Surf.Area= 98,426 sf Storage= 514,091 cf (412,983 cf above start) tion time= (not calculated) det. time= (not calculated)

#	Invert	Avail.Storage	Storage Des	scription		
1	490.00'	581,029 c	f Custom Sta	i <b>ge Data (Conic)</b> Li	sted below	
Elevat (fe	ion et)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
490	.00	0	0	0	0	
498	.40	36,110	101,108	101,108	36,221	
500	.00	42,400	62,741	163,849	42,610	
502	.00	54,880	97,012	260,861	55,187	
504	.00	78,730	132,895	393,755	79,107	
506	.00	109,382	187,274	581,029	109,836	

## Pond p13-1:

No Field Note Natural depression.

Inflow Area	a =	12.222 ac, Inflow Depth = 4.87" for	50-yr event
Inflow	=	62.38 cfs @ 12.04 hrs, Volume=	4.958 af
Outflow	=	56.92 cfs @ 12.07 hrs, Volume=	4.942 af, Atten= 9%, Lag= 1.8 min
Primary	=	56.92 cfs @ 12.07 hrs, Volume=	4.942 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 524.00' Surf.Area= 5,894 sf Storage= 16,480 cf Peak Elev= 526.94' @ 12.07 hrs Surf.Area= 9,979 sf Storage= 40,285 cf (23,805 cf above start) Flood Elev= 527.00' Surf.Area= 10,067 sf Storage= 40,862 cf (24,383 cf above start) Plug-Flow detention time= 168.7 min calculated for 4.563 af (92% of inflow) Center-of-Mass det. time= 99.2 min (869.1 - 769.9)

#	Invert	Avail.S	torage	Storage Description					
1	518.00'	50,	891 cf	Custom Sta	Custom Stage Data (Conic) Listed below				
	vation (feet)	Surf.Area (sq-ft)		Inc.Store cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)			
5	18.00	1,331		0	0	1,331			
52	20.00	2,048		3,353	3,353	2,104			
52	22.00	2,912		4,935	8,288	3,037			
52	22.50	3,150		1,515	9,803	3,294			
52	24.00	5,894		6,676	16,480	6,061			
52	26.00	8,542		14,354	30,834	8,776			
52	28.00	11,592		20,057	50,891	11,908			
#	Routing	Invert	Outlet	Devices					
1 2	Primary Primary	524.00' 525.90'	15.0' lo		rate C= 0.600 I <b>h Sharp-Crested I</b> )	Rectangular Weir			

Primary OutFlow Max=56.84 cfs @ 12.07 hrs HW=526.94' TW=502.00' (Dynamic Tailwater) -1=Orifice/Grate (Orifice Controls 0.40 cfs @ 8.1 fps)

-2=Sharp-Crested Rectangular Weir (Weir Controls 56.44 cfs @ 3.7 fps)

# Pond p14-1:

Field Note #26 Need to figure out how this pond works

Inflow Area =	50.663 ac, Inflow Depth = 4.28" for	50-yr event
Inflow =	168.24 cfs @ 12.08 hrs, Volume=	18.070 af
Outflow =	19.94 cfs @ 13.39 hrs, Volume=	6.895 af, Atten= 88%, Lag= 78.2 min
Primary =	19.94 cfs @ 13.39 hrs, Volume=	6.895 af
Starting Elev= 4 Peak Elev= 505 Plug-Flow deter	n-Stor-Ind method, Time Span= 0.00-48.00 497.40' Surf.Area= 22,200 sf Storage= 5 5.29' @ 48.00 hrs Surf.Area= 89,249 sf S ntion time= 330.8 min calculated for 5.637 a s det. time= 104.1 min (953.8 - 849.6)	4,760 cf Storage= 541,581 cf (486,821 cf above start)

#	Invert	Avail.Storage	Storage Description
1	490.00'	805,062 cf	Custom Stage Data (Conic) Listed below

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Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
490.00	0	0	0	0
497.40	22,200	54,760	54,760	22,286
498.00	25,330	14,249	69,009	25,433
500.00	52,810	76,476	145,485	52,948
502.00	73,360	125,608	271,093	73,574
504.00	84,070	157,308	428,402	84,467
506.00	92,130	176,139	604,540	92,797
508.00	108,618	200,522	805,062	109,437

# Routing Invert Outlet Devices

1 Primary 500.00' **24.0" x 80.0' long Culvert** CPP, end-section conforming to fill, Ke= 0.500 Outlet Invert= 502.00' S= -0.0250 '/' n= 0.012 Cc= 0.900

Primary OutFlow Max=19.95 cfs @ 13.39 hrs HW=504.74' TW=501.68' (Dynamic Tailwater)

## Pond p14-2:

Inflow Area	a =	15.934 ac, I	nflow Depth = 4.98	" for	50-yr event	
Inflow	=	81.03 cfs @	12.05 hrs, Volume	=	6.609 af	
Outflow	=	73.94 cfs @	12.09 hrs, Volume	=	6.580 af,	Atten= 9%, Lag= 2.4 min
Primary	=	73.94 cfs @	12.09 hrs, Volume	=	6.580 af	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 532.00' Surf.Area= 7,681 sf Storage= 23,903 cf Peak Elev= 534.90' @ 12.09 hrs Surf.Area= 12,217 sf Storage= 53,287 cf (29,384 cf above start) Flood Elev= 535.00' Surf.Area= 12,390 sf Storage= 54,538 cf (30,635 cf above start) Plug-Flow detention time= 158.6 min calculated for 6.032 af (91% of inflow) Center-of-Mass det. time= 84.4 min (853.7 - 769.3)

#	Invert	Avail.St	torage Storage D	escription			
1	526.00'	66,	889 cf Custom S	itage Data (Conic)	Listed below		
	ation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)		
52	26.00	2,239	0	0	2,239		
52	28.00	3,156	5,369	5,369	3,227		
53	80.00	4,207	7,338	12,707	4,362		
53	80.50	4,491	2,174	14,881	4,669		
53	32.00	7,681	9,023	23,903	7,885		
53	84.00	10,686	18,285	42,188	10,966		
53	36.00	14,093	24,701	66,889	14,463		
#	Routing		Outlet Devices	<b>2</b>			
1 2	Primary Primary	532.00' 533.60'		Vert. Orifice/Grate C= 0.600 Viong x 1.5' high Sharp-Crested Rectangular Weir and Contraction(s)			

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Primary OutFlow Max=73.91 cfs @ 12.09 hrs HW=534.90' TW=502.14' (Dynamic Tailwater) -1=Orifice/Grate (Orifice Controls 0.39 cfs @ 8.0 fps) -2=Sharp-Crested Rectangular Weir (Weir Controls 73.52 cfs @ 4.1 fps)

# Pond p16-1:

Field Note # 49 Large pond with man-made island. Geometry to be verified by survey. In particular, we are making big guesses about the outlets. Also need to find out about valves...

Inflow Area =	176.893 ac, Inflow Depth = 3.12"	for 50-yr event
Inflow =	213.25 cfs @ 12.33 hrs, Volume=	46.003 af
Outflow =	73.86 cfs @ 14.22 hrs, Volume=	34.198 af, Atten= 65%, Lag= 113.4 min
Primary =	73.86 cfs @ 14.22 hrs, Volume=	34.198 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 507.00' Surf.Area= 199,799 sf Storage= 878,320 cf Peak Elev= 511.30' @ 14.22 hrs Surf.Area= 307,205 sf Storage= 1,858,698 cf (980,378 cf above start) Flood Elev= 510.50' Surf.Area= 271,550 sf Storage= 1,623,217 cf (744,897 cf above start) Plug-Flow detention time= 971.7 min calculated for 14.032 af (31% of inflow) Center-of-Mass det. time= 351.2 min (1,249.7 - 898.4)

# Invert	Avail.Sto	orage Storage De	scription		
1 500.00'	2,062,0	087 cf Custom Sta	age Data (Conic) Li	sted below	
Elevation	Surf.Area	Inc.Store	Cum.Store	Wet.Area	
(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)	(sq-ft)	
500.00	0	0	0	0	
503.00	140,344	140,344	140,344	140,358	
509.20	232,500	1,143,862	1,284,206	232,994	
510.00	249,400	192,720	1,476,927	249,951	
512.00	338,000	585,160	2,062,087	338,634	
# Routing	Invert	Outlet Devices			
1 Primary	509.00'	18.0" x 110.0' long	Culvert CMP, pro	pjecting, no headw	all, Ke= 0.900
,		Outlet Invert= 505.7			
2 Primary		8.0" x 100.0' long a			
,		CMP, projecting, no	•		
		Outlet Invert= 500.0	-		00
3 Primary	510.50'	175.0 deg Sharp-C	rested Vee/Trap We	eir X 2.00 C= 2.4	6
,		5 1	•		

Primary OutFlow Max=73.86 cfs @ 14.22 hrs HW=511.30' TW=506.37' (Dynamic Tailwater) -1=Culvert (Inlet Controls 8.38 cfs @ 4.7 fps)

-2=assumed equalization pipe w/ valve (Controls 0.00 cfs)

-3=Sharp-Crested Vee/Trap Weir (Weir Controls 65.49 cfs @ 2.2 fps)

# Pond p17-1:

Field Note #45 Golf pond Geometry to be confirmed by surveyed

Inflow Area	ι =	112.240 ac, Ir	nflow Depth = 2.97"	for 50-yr event	
Inflow	=	93.28 cfs @	13.19 hrs, Volume=	27.736 af	
Outflow	=	93.27 cfs @	13.20 hrs, Volume=	27.736 af,	Atten= 0%, Lag= 0.4 min
Primary	=	93.27 cfs @	13.20 hrs, Volume=	27.736 af	-
-					

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 523.80' Surf.Area= 7,290 sf Storage= 9,234 cf Peak Elev= 525.60' @ 13.20 hrs Surf.Area= 11,432 sf Storage= 26,333 cf (17,099 cf above start) Flood Elev= 524.30' Surf.Area= 8,074 sf Storage= 13,623 cf (4,389 cf above start) Plug-Flow detention time= 15.3 min calculated for 27.524 af (99% of inflow) Center-of-Mass det. time= 8.4 min (916.7 - 908.3)

#	Invert	Avail.St	orage Storag	Storage Description				
1	520.00'	30,	224 cf Custo	Custom Stage Data (Conic) Listed below				
Eleva	ation feet)	Surf.Area (sq-ft)	Inc.St (cubic-fe		Cum.Store (cubic-feet)	Wet.Area (sq-ft)		
-	0.00 3.80	0 7,290	9.2	0 234	0 9,234	0 7,313		
52	4.00	7,300	1,4	159	10,693	7,374		
52	6.00	12,460	19,5	531	30,224	12,581		
#	Routing	Invert	Outlet Device	s				
1	Primary	523.80'	<b>2.2' long x 0.5' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 Coef. (English) 2.80 2.92 3.08 3.30 3.32					
2 3	Primary Primary	524.30' 525.20'	143.0 deg Sharp-Crested Vee/Trap Weir C= 2.47 178.0 deg x 60.0' long Sharp-Crested Vee/Trap Weir C= 2.46					

Primary OutFlow Max=93.27 cfs @ 13.20 hrs HW=525.60' TW=516.17' (Dynamic Tailwater) =Broad-Crested Rectangular Weir (Weir Controls 17.66 cfs @ 4.5 fps)

-2=Sharp-Crested Vee/Trap Weir (Weir Controls 14.27 cfs @ 2.8 fps)

-3=Sharp-Crested Vee/Trap Weir (Weir Controls 61.35 cfs @ 1.8 fps)

# Pond p18-1:

Field Note #46 Golf pond Geometry to be confirmed by surveyed

Inflow Area	a =	131.862 ac, In	nflow Depth = 2.99"	for 50-yr event	
Inflow	=	122.12 cfs @	12.43 hrs, Volume=	32.870 af	
Outflow	=	121.37 cfs @	12.48 hrs, Volume=	32.866 af,	Atten= 1%, Lag= 2.9 min
Primary	=	121.37 cfs @	12.48 hrs, Volume=	32.866 af	-

Proposed Conditions_10454-01	Type III 24-hr 50-yr Rai
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ainfall=6.70" Page 245 4/10/2006 3:16:04 PM

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 513.90' Surf.Area= 20,680 sf Storage= 26,884 cf Peak Elev= 516.27' @ 12.48 hrs Surf.Area= 30,246 sf Storage= 87,274 cf (60,390 cf above start) Flood Elev= 514.81' Surf.Area= 23,768 sf Storage= 48,709 cf (21,825 cf above start) Plug-Flow detention time= 39.9 min calculated for 32.242 af (98% of inflow) Center-of-Mass det. time= 23.8 min (929.1 - 905.3)

#	Invert	Avail.Stora	ge Storage Des	scription		
1	510.00'	148,288	cf Custom Sta	Custom Stage Data (Conic) Listed below		
Elevat	ion eet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
510	.00	0	0	0	0	
513	.90	20,680	26,884	26,884	20,704	
514.	.00	20,690	2,068	28,952	20,756	
516	.00	28,290	48,782	77,735	28,436	
518	.00	42,760	70,554	148,288	42,967	

#	Routing	Invert	Outlet Devices
1	Primary	513.90'	2.0' long x 0.5' breadth Broad-Crested Rectangular Weir
	-		Head (feet) 0.20 0.40 0.60 0.80 1.00
			Coef. (English) 2.80 2.92 3.08 3.30 3.32
2	Primary	514.81'	143.0 deg Sharp-Crested Vee/Trap Weir C= 2.47
3	Primary	515.32'	175.0 deg x 10.0' long Sharp-Crested Vee/Trap Weir C= 2.46
	-		

Primary OutFlow Max=121.36 cfs @ 12.48 hrs HW=516.27' TW=509.26' (Dynamic Tailwater) -1=Broad-Crested Rectangular Weir (Weir Controls 24.23 cfs @ 5.1 fps) -2=Sharp-Crested Vee/Trap Weir (Weir Controls 19.03 cfs @ 3.0 fps)

-3=Sharp-Crested Vee/Trap Weir (Weir Controls 78.11 cfs @ 2.6 fps)

# Pond p19-0:

Wetland Geometry to be confirmed by survey Based off aerial topo, and assumed topo contour

Pond Unchanged from existing to proposed conditions

Inflow Area = Inflow = Outflow = Primary = Secondary =	20.51 cfs @ 13.60 cfs @ 13.60 cfs @	nflow Depth = 2 12.61 hrs, Volu 12.96 hrs, Volu 12.96 hrs, Volu 0.00 hrs, Volu	ıme= 3.094 af, ıme= 3.094 af		Lag= 20.9 min	
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 972.00' Surf.Area= 86,000 sf Storage= 57,333 cf Peak Elev= 972.27' @ 12.96 hrs Surf.Area= 93,340 sf Storage= 87,215 cf (29,881 cf above start) Plug-Flow detention time= 300.9 min calculated for 1.778 af (57% of inflow) Center-of-Mass det. time= 59.3 min (947.2 - 888.0)						

#### Proposed Conditions\_10454-01 Prepared by The Chazen Companies

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HydroCAD® 7.00	s/n 000927	© 1986-2003 Applie	ed Microcomputer S	ystems	4/10/2006	3:16:04 PM
# Invert	Avail.Sto	orage Storage De	escription			
1 970.00'	282,3	329 cf Custom S	tage Data (Conic)	Listed below		
Elevation	Surf.Area	Inc.Store	Cum.Store	Wet.Area		
(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)	(sq-ft)		
970.00	0	0	0	0		
972.00	86,000	57,333	57,333	86,006		
974.00	141,270	224,996	282,329	141,327		
# Routing	Invert	Outlet Devices				
1 Secondary	973.60'	178.0 deg x 51.0'	long Sharp-Crest	ed Vee/Trap Weir	C= 2.46	
2 Primary	Primary 972.00' <b>35.0' long x 0.5' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00					
		Coef. (English) 2.8	80 2.92 3.08 3.3	0 3.32		

Primary OutFlow Max=13.60 cfs @ 12.96 hrs HW=972.27' TW=970.19' (Dynamic Tailwater) ←2=Broad-Crested Rectangular Weir (Weir Controls 13.60 cfs @ 1.5 fps)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=972.00' TW=973.60' (Dynamic Tailwater) 1=Sharp-Crested Vee/Trap Weir (Controls 0.00 cfs)

## Pond p20-1:

Field Note #50 Spring Fed Pond Geometry to be confirmed by surveyed

Inflow Area =	207.817 ac, Inflow Depth = 2.49"	for 50-yr event
Inflow =	83.08 cfs @ 14.19 hrs, Volume=	43.134 af
Outflow =	82.48 cfs @ 14.29 hrs, Volume=	42.185 af, Atten= 1%, Lag= 6.1 min
Primary =	82.48 cfs @ 14.29 hrs, Volume=	42.185 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 505.10' Surf.Area= 89,370 sf Storage= 138,524 cf Peak Elev= 506.37' @ 14.30 hrs Surf.Area= 91,205 sf Storage= 253,741 cf (115,217 cf above start) Plug-Flow detention time= 215.3 min calculated for 39.005 af (90% of inflow) Center-of-Mass det. time= 52.4 min (1,252.0 - 1,199.6)

#	Invert	Avail.Storag	ge Storage Des	cription	
1	502.00'	615,682	cf Custom Sta	ge Data (Prismat	tic) Listed below
Elevatio (fee		Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
502.0		0	0	0	
505.1	10	89,370	138,524	138,524	
506.0	00	89,380	80,437	218,961	
508.0	00	99,280	188,660	407,621	
510.0	00	108,781	208,061	615,682	

*Type III 24-hr 50-yr Rainfall=6.70"* Page 246 ns 4/10/2006 3:16:04 PM

## Proposed Conditions 10454-01

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Type III 24-hr 50-yr Rainfall=6.70" Page 247 HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems 4/10/2006 3:16:04 PM

#	Routing	Invert	Outlet Devices
1	Primary	505.10'	3.0' long x 1.5' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00
			Coef. (English) 2.62 2.64 2.64 2.68 2.75 2.86 2.92 3.07 3.07 3.03 3.28
			3.32
2	Primary	506.20'	6.5' long x 1.5' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50
			3.00
			Coef. (English) 2.62 2.64 2.64 2.68 2.75 2.86 2.92 3.07 3.07 3.03 3.28
			3.32
3	Primary	506.00'	176.0 deg x 97.0' long Sharp-Crested Vee/Trap Weir C= 2.46

Primary OutFlow Max=82.48 cfs @ 14.29 hrs HW=506.37' TW=506.00' (Dynamic Tailwater)

-1=Broad-Crested Rectangular Weir (Weir Controls 8.71 cfs @ 2.3 fps)

-2=Broad-Crested Rectangular Weir (Weir Controls 1.18 cfs @ 1.1 fps)

-3=Sharp-Crested Vee/Trap Weir (Weir Controls 72.59 cfs @ 1.8 fps)

## Pond p20-2:

Inflow Are	a =	13.511 ac, Inflow Depth = 4.85"	for 50-yr event	
Inflow	=	65.99 cfs @ 12.09 hrs, Volume=	= 5.458 af	
Outflow	=	11.19 cfs @ 12.59 hrs, Volume=	= 3.784 af, Atten= 83%, Lag= 30.3	min
Primary	=	11.19 cfs @ 12.59 hrs, Volume=	= 3.784 af	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 552.00' Surf.Area= 10,535 sf Storage= 35,913 cf Peak Elev= 558.86' @ 12.59 hrs Surf.Area= 25,397 sf Storage= 170,404 cf (134,490 cf above start) Flood Elev= 559.00' Surf.Area= 25,653 sf Storage= 174,016 cf (138,102 cf above start) Plug-Flow detention time= 744.4 min calculated for 2.959 af (54% of inflow) Center-of-Mass det. time= 477.2 min (1,258.5 - 781.2)

#	Invert	Avail.St	orage Storage D	Description		
1	546.00'	199,	647 cf Custom S	Stage Data (Conic)	Listed below	
	feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
54	6.00 8.00	3,714 4,960	0 8,644	0 8,644	3,714 5,044	
	0.00 0.50	6,308 6,661	11,241 3,242	19,885 23,127	6,493 6,874	
	2.00 2.50	10,535 15,037	12,786 6,360	35,913 42,273	10,779 15,285	
55	4.00 6.00	17,268 20,441	24,209 37,664	66,483 104,147	17,616 20,935	
55	8.00	23,840 27,465	44,237 51,262	148,384 199,647	24,494 28,292	
#	Routing	Invert	Outlet Devices	199,047	20,232	
1 2	Primary Primary	552.00' 558.20'	3.0" Vert. Orifice/ 6.1' long x 6.2' hi		Rectangular Weir	2 End Contraction(s)

Primary OutFlow Max=11.19 cfs @ 12.59 hrs HW=558.86' TW=506.17' (Dynamic Tailwater) 1=Orifice/Grate (Orifice Controls 0.61 cfs @ 12.5 fps) 2=Sharp-Crested Rectangular Weir (Weir Controls 10.58 cfs @ 2.7 fps)

# Pond p21-1:

Inflow Area	=	459.188 ac, I	nflow Depth = 3.04	for 50-yr event
Inflow :	=	535.64 cfs @	12.27 hrs, Volume	= 116.484 af
Outflow :	=	36.39 cfs @	20.66 hrs, Volume	= 94.467 af, Atten= 93%, Lag= 503.5 min
Primary :	=	36.39 cfs @	20.66 hrs, Volume	= 94.467 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 486.41' @ 20.66 hrs Surf.Area= 1,320,933 sf Storage= 3,036,292 cf Plug-Flow detention time= 873.8 min calculated for 94.448 af (81% of inflow) Center-of-Mass det. time= 708.7 min (1,726.3 - 1,017.6)

#	Invert	Avail.St	orage Storage D	escription		
<u></u> 1	480.40'	5,244,8	U	tage Data (Conic)	Listed below	
•	100.10	0,211,		lugo Dulu (como)		
Eleva	ation	Surf.Area	Inc.Store	Cum.Store	Wet.Area	
(1	feet)	(sq-ft)	(cubic-feet)	(cubic-feet)	(sq-ft)	
48	0.40	0	0	0	0	
48	2.00	202,230	107,856	107,856	202,234	
. •	4.00	485,198	667,114	774,970	485,231	
. •	6.00	1,275,481	1,698,237	2,473,208	1,275,541	
48	8.00	1,499,208	2,771,678	5,244,885	1,499,423	
#	Routing	Invert	Outlet Devices			
1	Primary	480.40'	30.0" x 70.0' long	<b>Culvert</b> CMP, p	rojecting, no head	wall, Ke= 0.900

1 Primary 480.40' **30.0" x 70.0' long Culvert** CMP, projecting, no headwall, Ke= 0.900 Outlet Invert= 480.40' S= 0.0000 '/' n= 0.024 Cc= 0.900

Primary OutFlow Max=36.39 cfs @ 20.66 hrs HW=486.41' TW=0.00' (Dynamic Tailwater) -1=Culvert (Barrel Controls 36.39 cfs @ 7.4 fps)

## Pond p21-4:

Inflow Are	ea =	5.152 ac, Inflow Depth = $4.23$ "	for 50-yr event
Inflow	=	17.73 cfs @ 12.03 hrs, Volume=	1.818 af
Outflow	=	13.53 cfs @ 12.23 hrs, Volume=	1.806 af, Atten= 24%, Lag= 12.4 min
Primary	=	13.53 cfs @ 12.23 hrs, Volume=	1.806 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 496.00' Surf.Area= 5,112 sf Storage= 14,306 cf Peak Elev= 498.95' @ 12.23 hrs Surf.Area= 8,785 sf Storage= 35,224 cf (20,918 cf above start) Flood Elev= 499.00' Surf.Area= 8,847 sf Storage= 35,622 cf (21,317 cf above start) Plug-Flow detention time= 373.9 min calculated for 1.478 af (81% of inflow) Center-of-Mass det. time= 221.5 min (1,011.6 - 790.1)

# Proposed Conditions 10454-01

Type III 2	24-hr 50-yr	Rainfall=6.70"
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#	Invert	Avail.St	torage Sto	orage De	escription			
1	490.00'	44,	433 cf Cu	Custom Stage Data (Conic) Listed below				
Eleva (fe	ition eet)	Surf.Area (sq-ft)	-	:.Store c-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)		
490	0.00	1,146		0	0	1,146		
	2.00	1,784		2,907	2,907	1,839		
494	4.00	2,530		4,292	7,199	2,654		
494	4.50	2,733		1,315	8,514	2,876		
496	5.00	5,112		5,791	14,306	5,278		
498	3.00	7,468	1	12,506	26,812	7,699		
500	0.00	10,226	1	17,622	44,433	10,536		
	Routing Primary	Invert 496.00'	Outlet Dev 3.0" Vert.		Grate C= 0.600			
2	Primary	498.10'	5.0' long x	2.0' hig	h Sharp-Crested	<b>Rectangular Weir</b>	2 End Contraction(s)	

Primary OutFlow Max=13.53 cfs @ 12.23 hrs HW=498.95' TW=483.60' (Dynamic Tailwater) -1=Orifice/Grate (Orifice Controls 0.40 cfs @ 8.1 fps)

-2=Sharp-Crested Rectangular Weir (Weir Controls 13.13 cfs @ 3.2 fps)

## Pond p21-5:

Inflow Area	a =	2.398 ac, 1	nflow Depth = 3.8	9" for	50-yr event		
Inflow	=	8.52 cfs @	12.19 hrs, Volum	<del>)</del> =	0.777 af		
Primary	=	8.52 cfs @	12.19 hrs, Volum	)=	0.777 af,	Atten= 0%,	Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

## Pond p21-6:

Inflow Area	a =	6.182 ac, Inflow Depth = $4$	28" for 50-yr event	
Inflow	=	20.03 cfs @ 12.23 hrs, Volu	ne= 2.206 af	
Outflow	=	15.21 cfs @ 12.39 hrs, Volu	ne= 2.196 af, At	ten= 24%, Lag= 9.5 min
Primary	=	15.21 cfs @ 12.39 hrs, Volu	ne= 2.196 af	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 492.00' Surf.Area= 3,323 sf Storage= 4,847 cf Peak Elev= 494.93' @ 12.39 hrs Surf.Area= 13,699 sf Storage= 33,430 cf (28,584 cf above start) Flood Elev= 495.00' Surf.Area= 13,824 sf Storage= 34,456 cf (29,609 cf above start) Plug-Flow detention time= 282.4 min calculated for 2.084 af (94% of inflow) Center-of-Mass det. time= 230.2 min (1,043.8 - 813.6)

#	Invert	Avail.Storage	Storage Description
1	488.00'	48,245 cf	Custom Stage Data (Conic) Listed below

## Proposed Conditions\_10454-01

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Wet.Area	Cum.Store	Inc.Store	Surf.Area	Elevation
(sq-ft)	(cubic-feet)	(cubic-feet)	(sq-ft)	(feet)
296	0	0	296	488.00
946	1,162	1,162	924	490.00
1,141	1,670	508	1,110	490.50
3,367	4,847	3,177	3,323	492.00
6,212	7,182	2,336	6,166	492.50
12,214	20,666	13,484	12,147	494.00
15,669	48,245	27,579	15,500	496.00

 # Routing
 Invert
 Outlet Devices

 1
 Primary
 492.00'
 3.0" Vert. Orifice/Grate

Primary 492.00' **3.0" Vert. Orifice/Grate** C= 0.600

2 Primary 494.00' 5.0' long x 2.0' high Sharp-Crested Rectangular Weir 2 End Contraction(s)

**Primary OutFlow** Max=15.21 cfs @ 12.39 hrs HW=494.93' TW=484.18' (Dynamic Tailwater) **1=Orifice/Grate** (Orifice Controls 0.40 cfs @ 8.1 fps)

2=Sharp-Crested Rectangular Weir (Weir Controls 14.81 cfs @ 3.3 fps)

## Pond p21-7:

Inflow Are	a =	8.355 ac, Inflow Depth = $4.53$ "	for 50-yr event	
Inflow	=	35.01 cfs @ 12.04 hrs, Volume=	3.156 af	
Outflow	=	15.67 cfs @ 12.34 hrs, Volume=	3.133 af, Atten= 55%, Lag= 18.3 mir	۱
Primary	=	15.67 cfs @ 12.34 hrs, Volume=	3.133 af	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 492.00' Surf.Area= 3,941 sf Storage= 8,984 cf Peak Elev= 498.83' @ 12.34 hrs Surf.Area= 13,082 sf Storage= 65,036 cf (56,052 cf above start) Flood Elev= 499.00' Surf.Area= 13,379 sf Storage= 67,369 cf (58,385 cf above start) Plug-Flow detention time= 386.6 min calculated for 2.926 af (93% of inflow) Center-of-Mass det. time= 308.8 min (1,085.0 - 776.1)

#	Invert	Avail.St	torage	Storage De	scription		
1	486.00'	80,	712 cf	Custom Sta	age Data (Conic	<b>c)</b> Listed below	
Eleva	tion eet)	Surf.Area (sq-ft)		Inc.Store ubic-feet)	Cum.Store (cubic-feet)		
486	6.00	478		0	0	478	
488	3.00	964		1,414	1,414	999	
490	0.00	1,601		2,538	3,952	1,684	
490	).50	1,782		845	4,797	1,879	
492	2.00	3,941		4,187	8,984	4,056	
494	1.00	6,120		9,981	18,965	6,292	
496	6.00	8,702		14,746	33,712	8,944	
498	3.00	11,686		20,315	54,027	12,012	
500	0.00	15,071		26,685	80,712	15,495	
1 2	Routing Primary Primary	Invert 492.00' 496.05'	6.0" Vei	rt. Orifice/G rt. Orifice/G	rate C= 0.600 rate X 2.00 C=	= 0.600	
3	Primary	498.00'	5.0' long	g x 6.0' high	n Sharp-Creste	d Rectangular Weir	2 End Contraction(s)

Primary OutFlow Max=15.66 cfs @ 12.34 hrs HW=498.82' TW=484.09' (Dynamic Tailwater)

-1=Orifice/Grate (Orifice Controls 0.61 cfs @ 12.5 fps)

-2=Orifice/Grate (Orifice Controls 3.00 cfs @ 7.7 fps)

-3=Sharp-Crested Rectangular Weir (Weir Controls 12.05 cfs @ 3.0 fps)

# Pond p22-1:

Field Note #54 Golf Pond Geometry to be confirmed by survey

Pond unchanged from existing to proposed conditions

Inflow Area	a =	78.382 ac, Inflow Depth = 3.11" for 50-yr event	
Inflow	=	151.44 cfs @ 12.29 hrs, Volume= 20.340 af	
Outflow	=	149.85 cfs @ 12.32 hrs, Volume= 20.041 af, Atten= 1%, Lag= 1.	.9 min
Primary	=	149.85 cfs @ 12.32 hrs, Volume= 20.041 af	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 498.10' Surf.Area= 6,520 sf Storage= 10,106 cf Peak Elev= 502.37' @ 12.32 hrs Surf.Area= 12,088 sf Storage= 49,038 cf (38,932 cf above start) Plug-Flow detention time= 33.3 min calculated for 19.805 af (97% of inflow) Center-of-Mass det. time= 15.0 min (879.7 - 864.7)

#	Invert	Avail.St	torage Storage [	Description	
1	495.00'	143,	770 cf Custom	Stage Data (Prism	atic) Listed below
Elev	vation	Surf.Area	Inc.Store	Cum.Store	
-	(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)	
- 49	95.00	0	0	0	
49	98.10	6,520	10,106	10,106	
50	00.00	8,390	14,164	24,270	
50	02.00	11,530	19,920	44,190	
50	04.00	14,530	26,060	70,250	
	06.00	18,340	,	,	
50	00.80	22,310	40,650	143,770	
#	Routing	Invert	Outlet Devices		
1	Primary	499.75'	18.0" x 21.0' lon	g Culvert CMP, p	projecting, no headwall, Ke= 0.900
	-		Outlet Invert= 49	9.75' S= 0.0000 '/	' n= 0.024 Cc= 0.900
2	Primary	500.50'			ested Rectangular Weir
			( )		1.00 1.20 1.40 1.60
_					64 2.63 2.64 2.64 2.63
3	Primary	500.50'			Crested Rectangular Weir
			( )		
			Coer. (English) 2	2.62 2.66 2.70 2.6	66 2.65 2.66 2.65 2.63

Primary OutFlow Max=149.84 cfs @ 12.32 hrs HW=502.37' TW=484.04' (Dynamic Tailwater) -1=Culvert (Barrel Controls 8.39 cfs @ 4.7 fps) -2=Broad-Crested Rectangular Weir (Weir Controls 6.74 cfs @ 3.6 fps) -3=Broad-Crested Rectangular Weir (Weir Controls 134.72 cfs @ 3.6 fps)

# Pond p23-1:

Inflow Area	a =	29.123 ac, Infle	low Depth = $3.57$ "	for 50-yr event	
Inflow	=	61.35 cfs @ 12	2.53 hrs, Volume=	8.676 af	
Outflow	=	61.30 cfs @ 12	2.54 hrs, Volume=	7.960 af,	Atten= 0%, Lag= 0.5 min
Primary	=	61.30 cfs @ 12	2.54 hrs, Volume=	7.960 af	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 507.92' @ 12.54 hrs Surf.Area= 14,448 sf Storage= 33,792 cf Plug-Flow detention time= 59.7 min calculated for 7.959 af (92% of inflow) Center-of-Mass det. time= 17.9 min (876.5 - 858.5)

#	Invert	Avail.Stor	age Storage De	scription			
1	503.50'	503.50' 68,915 cf		Custom Stage Data (Conic) Listed below			
Elevat (fe	tion eet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)		
503	5.50	0	0	0	0		
504	.00	2,390	398	398	2,390		
506	6.00	9,090	10,761	11,159	9,110		
508	6.00	14,660	23,529	34,688	14,732		
510.00		19,690	34,227	68,915	19,847		
#_ F	Routing	Invert C	outlet Devices				

1 Primary 507.70' **178.0 deg x 178.0' long Sharp-Crested Vee/Trap Weir** C= 2.46

Primary OutFlow Max=61.29 cfs @ 12.54 hrs HW=507.92' TW=507.25' (Dynamic Tailwater) **1=Sharp-Crested Vee/Trap Weir** (Weir Controls 61.29 cfs @ 1.4 fps)

## Pond p23-2:

Inflow Area	a =	16.094 ac, Inflow Depth = 5.18"	for 50-yr event
Inflow	=	96.57 cfs @ 12.06 hrs, Volume=	= 6.947 af
Outflow	=	60.09 cfs @ 12.14 hrs, Volume=	6.101 af, Atten= 38%, Lag= 5.0 min
Primary	=	60.09 cfs @ 12.14 hrs, Volume=	= 6.101 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 508.00' Surf.Area= 7,318 sf Storage= 15,927 cf Peak Elev= 514.82' @ 12.14 hrs Surf.Area= 24,429 sf Storage= 137,649 cf (121,722 cf above start) Flood Elev= 515.00' Surf.Area= 24,788 sf Storage= 141,986 cf (126,059 cf above start) Plug-Flow detention time= 373.5 min calculated for 5.735 af (83% of inflow) Center-of-Mass det. time= 274.3 min (1,047.2 - 772.9)

# Proposed Conditions\_10454-01

Type III 24-hr 50-yr Rainfall=6.70" Prepared by The Chazen Companies HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems

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#	Invert	Avail.St	orage	Storage De	scription		
1	502.00'		746 cf		Custom Stage Data (Conic) Listed below		
	otion	Surf.Area		Inc.Store Cum.Store Wet.Area			
	ation (feet)	(sq-ft)	((	Inc.Store cubic-feet)	Cum.Store (cubic-feet		
	)2.00	826	(	0	)	· · · · · ·	<u>10</u> 326
	)4.00	1,667		2,444	2,444		/02
	)6.00	2,788		4,407	6,852		
	06.50	3,112		1,474	8,326	,	
	00.80	7,318		7,601	15,927		
	08.50	12,618		4,924	20,851		
51	0.00	15,208		20,839	41,690		00
51	2.00	18,859		34,002	75,692	2 19,1	66
51	4.00	22,736		41,535	117,227	' 23,1	75
51	6.00	26,840		49,519	166,746	6 27,4	28
#	Pouting	Invort	Outlot	Dovices			
	Routing			Devices		<u> </u>	
1	Primary				Grate C= 0.600 Grate X 2.00		
2 3	Primary Primary					ed Rectangula	r Woir
3	Fillinary			Contraction(		eu Reclanyula	i wen
					5)		
Prim	ary OutFlow	/ Max=60.0	)5 cfs @	2 12.14 hrs	HW=514.82' T	W=483.13' (Dv	/namic Tailwater)
				s 0.61 cfs @			,
				s 10.07 cfs @			
					Controls 49.37 c	fs @ 3.0 fps)	
				Pond zDF	P1: Design P	oint 1	
<b>F</b> ield	note #10						
	note #10. ert dimensio	ne to bo cor	firmod	by survoy			
			IIIIIeu	by Survey.			
Inflov	v Area =	26.658 ac	. Inflov	v Depth = 3	8.53" for 50-y	r event	
Inflov				47 hrs, Volu		837 af	
Outfle	= wc			48 hrs, Volu		837 af, Atten=	0%, Lag= 0.1 min
Prima	ary =			48 hrs, Volu		837 af	
<b>–</b>				•	00.40.00 l		
					.00-48.00 hrs, c		
					4 sf Storage=	∠40 CT	
Flood Elev= 727.00' Surf.Area= 1,105 sf Storage= 2,619 cf							

Plug-Flow detention time= 0.1 min calculated for 7.837 af (100% of inflow) Center-of-Mass det. time= 0.1 min (904.8 - 904.8)

_	#	Invert	Avail.Storage	Storage Description
_	1	720.10'	3,706 cf	Custom Stage Data (Conic) Listed below

# Proposed Conditions 10454-01

Prepared by The Chazen Co	mpanies
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Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
720.10	0	0	0	0
722.00	90	57	57	96
724.00	340	403	460	364
726.00	760	1,072	1,533	815
728.00	1,450	2,173	3,706	1,543

#	Routing	Invert	Outlet Devices
1	Primary	720.10'	<b>42.0" x 120.0' long Culvert</b> CMP, square edge headwall, Ke= 0.500
	-		Outlet Invert= 700.00' S= 0.1675 '/' n= 0.024 Cc= 0.900
2	Primary	727.00'	155.0 deg Sharp-Crested Vee/Trap Weir C= 2.47

Primary OutFlow Max=47.25 cfs @ 12.48 hrs HW=722.91' TW=686.76' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 47.25 cfs @ 5.7 fps)

-2=Sharp-Crested Vee/Trap Weir (Controls 0.00 cfs)

# Pond zDP2: Design Point 2

Field Note #15 Culvert dimensions to be confirmed by survey Overflow to ditch is currently discarded... We may have to model that area...

Inflow Area =	93.367 ac, Inflow Depth = 2.88"	for 50-yr event
Inflow =	117.46 cfs @ 12.81 hrs, Volume=	22.413 af
Outflow =	117.45 cfs @ 12.82 hrs, Volume=	22.413 af, Atten= 0%, Lag= 0.5 min
Discarded =	81.48 cfs @ 12.82 hrs, Volume=	6.710 af
Primary =	35.96 cfs @ 12.82 hrs, Volume=	15.703 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 626.25' @ 12.82 hrs Surf.Area= 1,567 sf Storage= 3,836 cf Flood Elev= 624.50' Surf.Area= 925 sf Storage= 1,728 cf Plug-Flow detention time= 0.5 min calculated for 22.408 af (100% of inflow) Center-of-Mass det. time= 0.5 min (893.3 - 892.8)

#	Invert	Avail.S	torage	Storage Des	scription		
1	619.60'	7,	7,280 cf Custor		tom Stage Data (Conic) Listed below		
Elevation (feet)		Surf.Area (sq-ft)		Inc.Store Jbic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
61	19.60	0		0	0	0	
62	20.00	10		1	1	10	
62	22.00	260		214	215	269	
62	24.00	760		976	1,192	793	
62	26.00	1,420		2,146	3,338	1,492	
62	28.00	2,580		3,943	7,280	2,694	
# Routing		Invert	Outlet D	evices			
1	Primary	619.60'			<b>Culvert</b> RCP, er 0' S= 0.0773 '/'		ming to fill, Ke= 0.500 .900
2 Discarded 624.50' 166.0 deg Sharp-C		ested Vee/Trap V	<b>Veir</b> C= 2.46				

**Discarded OutFlow** Max=81.48 cfs @ 12.82 hrs HW=626.25' (Free Discharge) **2=Sharp-Crested Vee/Trap Weir** (Weir Controls 81.48 cfs @ 3.3 fps)

Primary OutFlow Max=35.96 cfs @ 12.82 hrs HW=626.25' TW=607.68' (Dynamic Tailwater)

## Pond zDP3: Design Point 3

Inflow Area	a =	228.471 ac, Inflow Depth = 19.30"	for 50-yr ev	vent
Inflow	=	289.71 cfs @ 12.41 hrs, Volume=	367.475	5 af
Primary	=	289.71 cfs @ 12.41 hrs, Volume=	367.47	5 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

## Pond zDP4: Design Point 4

Inflow Area =		459.188 ac, Inflow Depth = 2.47"	" for 50-yr event
Inflow	=	36.39 cfs @ 20.66 hrs, Volume=	= 94.467 af
Primary	=	36.39 cfs @ 20.66 hrs, Volume=	= 94.467 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

## Pond zDP5: Design Point 5

Inflow Area =		28.325 ac, I	nflow Depth = 3.37"	for 50-yr event	
Inflow	=	62.42 cfs @	12.45 hrs, Volume=	7.956 af	
Primary	=	62.42 cfs @	12.45 hrs, Volume=	7.956 af,	Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

# Post-Development Conditions 100 year 24 hour Storm Event Model Computations

Proposed Conditions_10454-01Type III 24-hr 100-yr Rainfall=7.00"Prepared by The Chazen CompaniesPage 256HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems4/10/2006 3:16:19 PM			
Subcatchment s01-0:			
Runoff = 21.78 cfs @ 12.60 hrs, Volume= 3.264 af, Depth= 3.41"			
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=7.00"			
Area (ac) CN Description			
11.485 68			
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)			
42.8 Direct Entry,			
Subcatchment s02-1:			
Runoff = 119.37 cfs @ 12.87 hrs, Volume= 22.129 af, Depth= 3.10"			
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=7.00"			
Area (ac) CN Description			
85.591 65			
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)			
61.3 Direct Entry,			
Subcatchment s02-2:			
Runoff = 16.49 cfs @ 12.42 hrs, Volume= 2.076 af, Depth= 3.20"			
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=7.00"			
Area (ac) CN Description			
7.776 66			
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)			
29.4Direct Entry,			
Subcatchment s02-3:			
Runoff = 29.15 cfs @ 12.02 hrs, Volume= 1.828 af, Depth= 5.37"			
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=7.00"			

Prepared by The Chazen Companies $4/10/2006 3:16:19 \text{ PM}$ Area (ac) CN Description (min) (teet) (fuft) (fusec) (cfs) 1.6 Direct Entry, Subcatchment s03-1: Runoff = 25.44 cfs @ 12.39 hrs, Volume= 3.146 af, Depth= 3.62" Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=7.00" Area (ac) CN Description (min) (teet) (fuft) (fusec) (cfs) Direct Entry, Subcatchment s03-2: Runoff = 16.85 cfs @ 12.02 hrs, Volume= 1.018 af, Depth= 4.04" Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=7.00" Area (ac) CN Description (min) (teet) (fuft) (fusec) (cfs) Direct Entry, Subcatchment s03-2: Runoff = 16.85 cfs @ 12.02 hrs, Volume= 1.018 af, Depth= 4.04" Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=7.00" Area (ac) CN Description (min) (teet) (fuft) (fusec) (cfs) 1.5 Direct Entry, Subcatchment s03-2(IC): s03-2 Impervious Cover Runoff = 13.41 cfs @ 12.02 hrs, Volume= 0.937 af, Depth= 6.76" Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=7.00" Area (ac) CN Description (min) (teet) (fuft) (fusec) (cfs) Direct Entry, Subcatchment s03-2(IC): s03-2 Impervious Cover Runoff = 13.41 cfs @ 12.02 hrs, Volume= 0.937 af, Depth= 6.76" Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=7.00" Area (ac) CN Description (min) (teet) (fuft) (fusec) (cfs) Tc Length Slope Velocity Capacity Description (min) (teet) (fuft) (fusec) (cfs) Area (ac) CN Description (min) (teet) (fuft) (fusec) (cfs) Direct Entry, Tc Length Slope Velocity Capacity Description (min) (teet) (fuft) (fusec) (cfs) Direct Entry, Area (ac) CN Description (min) (teet) (fuft) (fusec) (cfs) Direct Entry, Area (ac) CN Description (min) (teet) (fuft) (fusec) (cfs) Direct Entry, Area (ac) CN Description (min) (teet) (fuft) (fusec) (fuft) Direct Entry	Proposed Conditions_10454-01	Type III 24-hr 100-yr Rainfall=7.00"
Area (ac)       CN       Description         4.088       86         Tc       Length       Slope       Velocity       Capacity       Description         (min)       (feet)       (ft/ft)       (ft/sec)       (cfs)         1.6       Direct Entry,         Subcatchment s03-1:         Runoff       =       25.44 cfs @       12.39 hrs, Volume=       3.146 af, Depth=       3.62"         Runoff       =       25.44 cfs @       12.39 hrs, Volume=       3.146 af, Depth=       3.62"         Runoff by SCS TR-20 method, UH=SCS, Time Span=       0.00-48.00 hrs, dt=       0.01 hrs         Tc       Length       Slope       Velocity       Capacity       Description         (min)       (feet)       (ft/ft)       (ft/sec)       (cfs)       28.8       Direct Entry,         Subcatchment s03-2:         Runoff       =       16.85 cfs @       12.02 hrs, Volume=       1.018 af, Depth=       4.04"         Runoff by SCS TR-20 method, UH=SCS, Time Span=       0.00-48.00 hrs, dt=       0.01 hrs       Type III 24-hr       100-yr       Rainfall=7.00"         Area (ac)       CN       Description       (cfs)       Direct Entry,	Prepared by The Chazen Companies HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied	Page 257 Microcomputer Systems 4/10/2006 3:16:19 PM
4.088         86           Tc         Length         Slope         Velocity         Capacity         Description           (min)         (feet)         (ft/ft)         (ft/sec)         (cfs)           1.6         Direct Entry,           Subcatchment s03-1:           Runoff         =         25.44 cfs @         12.39 hrs, Volume=         3.146 af, Depth=         3.62"           Runoff         =         25.44 cfs @         12.39 hrs, Volume=         3.146 af, Depth=         3.62"           Runoff         =         25.44 cfs @         12.39 hrs, Volume=         3.146 af, Depth=         3.62"           Runoff         type         III 24-hr 100-yr         Rainfall=7.00"         Area (ac)         CN         Description           (min)         (feet)         (ft/ft)         (ft/sec)         (cfs)         Direct Entry,           28.8         Direct Entry,         Subcatchment s03-2:         Subcatchment s03-2:         Nunoff         4.04"           Runoff         =         16.85 cfs @         12.02 hrs, Volume=         1.018 af, Depth= 4.04"           Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs         Type III 24-hr 100-yr         Charact Color           Jozet         Direct Entry,         D		
$\begin{array}{rrrrrrrrrrrrrrrrrrrrrrrrrrrrrrrrrrrr$		
(min)         (feet)         (ft/ft)         (ft/sec)         (cfs)           1.6         Direct Entry,         Subcatchment s03-1:           Runoff         =         25.44 cfs @ 12.39 hrs, Volume=         3.146 af, Depth= 3.62"           Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs         Type III 24-hr 100-yr Rainfall=7.00"           Area (ac)         CN         Description           10.435         70         Tc           Tc         Length         Slope         Velocity           28.8         Direct Entry,           Subcatchment s03-2:           Runoff         =         16.85 cfs @ 12.02 hrs, Volume=         1.018 af, Depth= 4.04"           Runoff         prect Entry,         Subcatchment s03-2:           Runoff         Subcatchment span= 0.00-48.00 hrs, dt= 0.01 hrs           Type III 24-hr 100-yr         Rainfall=7.00"           Area (ac)         CN         Description           3.021         74         Tc           Tc         Length         Slope           Subcatchment s03-2(IC): s03-2 Impervious Cover           Runoff         =         13.41 cfs @ 12.02 hrs, Volume=         0.937 af, Depth= 6.76"           Runoff         =         13.41 cfs @ 12.02 hrs, Volume= </td <td></td> <td></td>		
1.6Direct Entry,Subcatchment s03-1:Runoff = 25.44 cfs @ 12.39 hrs, Volume= 3.146 af, Depth= 3.62"Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrsType III 24-hr 100-yr Rainfall=7.00"Area (ac) CN Description(Interst and the second se		Description
$ \begin{array}{rcl} { { Runoff } & = & 25.44 \ cfs @ 12.39 \ hrs, \ Volume= & 3.146 \ af, \ Depth= 3.62'' \\ { { Runoff } by SCS TR-20 \ method, UH=SCS, \ Time \ Span= 0.00-48.00 \ hrs, \ dt= 0.01 \ hrs \\ { Type III 24-hr 100-yr \ Rainfall=7.00'' \\ \hline \hline \hline \\ \hline \\$		Direct Entry,
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs         Type III 24-hr 100-yr Rainfall=7.00"         Area (ac) CN Description         (feet) Slope Velocity Capacity Description         (min) (feet) (ft/ft) (ft/sec) (cfs)         28.8         Direct Entry,         Subcatchment s03-2:         Runoff = 16.85 cfs @ 12.02 hrs, Volume= 1.018 af, Depth= 4.04"         Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs         Type III 24-hr 100-yr Rainfall=7.00"         Area (ac) CN Description         3.021 74         To Length Slope Velocity Capacity Description         (min) (feet) Slope Velocity Capacity Description         (min) (feet) Slope Velocity Capacity Description         Minet Entry,         Subcatchment s03-2(IC): s03-2 Impervious Cover         Runoff = 13.41 cfs @ 12.02 hrs, Volume= 0.937 af, Depth= 6.76"         Runoff = 13.41 cfs @ 12.02 hrs, Volume= 0.937 af, Depth= 6.76"         Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs         Type III 24-hr 100-yr Rainfall=7.00"         Area (ac) CN Description         1.663 98 <td>Subcate</td> <td>chment s03-1:</td>	Subcate	chment s03-1:
Type III 24-hr 100-yr Rainfall=7.00" Area (ac) CN Description 10.435 70 Tc Length Slope Velocity Capacity Description (min) (feet) (tt/ft) (ft/sec) (cfs) 28.8 Direct Entry, Subcatchment s03-2: Runoff = 16.85 cfs @ 12.02 hrs, Volume= 1.018 af, Depth= 4.04" Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=7.00" Area (ac) CN Description 3.021 74 Tc Length Slope Velocity Capacity Description (min) (feet) (tt/ft) (ft/sec) (cfs) 1.5 Direct Entry, Subcatchment s03-2(IC): s03-2 Impervious Cover Runoff = 13.41 cfs @ 12.02 hrs, Volume= 0.937 af, Depth= 6.76" Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=7.00" Area (ac) CN Description 1.663 98 Tc Length Slope Velocity Capacity Description (min) (feet) (tt/ft) (ft/sec) (cfs)	Runoff = 25.44 cfs @ 12.39 hrs, Volun	ne= 3.146 af, Depth= 3.62"
10.435       70         Tc       Length       Slope       Velocity       Capacity       Description         (min)       (feet)       (ft/ft)       (ft/sec)       (cfs)         28.8       Direct Entry,         Subcatchment s03-2:         Runoff       =       16.85 cfs @       12.02 hrs, Volume=       1.018 af, Depth=       4.04"         Runoff by SCS TR-20 method, UH=SCS, Time Span=       0.00-48.00 hrs, dt=       0.01 hrs         Type III 24-hr 100-yr Rainfall=7.00"         Area (ac)       CN       Description         3.021       74       Tc       Length       Slope       Velocity       Capacity       Description         (min)       (feet)       (ft/ft)       (ft/sec)       (cfs)       Intert Entry,         Subcatchment s03-2(IC): s03-2 Impervious Cover         Runoff       =       13.41 cfs @       12.02 hrs, Volume=       0.937 af, Depth=       6.76"         Runoff by SCS TR-20 method, UH=SCS, Time Span=       0.00-48.00 hrs, dt=       0.01 hrs         Type III 24-hr 100-yr       Rainfall=7.00"       Area (ac)       CN       Description         1.663       98       Tc       Length       Slope       Vel		pan= 0.00-48.00 hrs, dt= 0.01 hrs
Tc       Length (feet)       Slope Velocity (cfs)       Description         28.8       Direct Entry,         Subcatchment s03-2:         Runoff       =       16.85 cfs @ 12.02 hrs, Volume=       1.018 af, Depth= 4.04"         Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs       Tree (ac)       CN         Area (ac)       CN       Description         3.021       74       Tc       Length         Subcatchment s03-2(IC): s03-2 Impervious Cover         Runoff       =       13.41 cfs @ 12.02 hrs, Volume=         0.937 af, Depth= 6.76"         Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs         Tc       Length       Slope         Velocity       Capacity       Description         (min)       (feet)       (ft/ft)         0.937 af, Depth= 6.76"       Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs         Type III 24-hr 100-yr       Rainfall=7.00"         Area (ac)       CN       Description         1.663       98       Tc         Tc       Length       Slope       Velocity         Mathematical Slope       Velocity       Capacity          0.01 (ft/ft)       (ft/sec	Area (ac) CN Description	
$\begin{array}{rcl} (\mbox{min}) & (\mbox{feet}) & (\mbox{ft/ft}) & (\mbox{ft/sec}) & (\mbox{cfs}) \\ \hline 28.8 & \mbox{Direct Entry,} \\ \hline & \mbox{Subcatchment s03-2:} \\ \hline & \mbox{Runoff} = 16.85 \mbox{cfs} @ 12.02 \mbox{ hrs, Volume} = 1.018 \mbox{ af, Depth} = 4.04'' \\ \hline & \mbox{Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 \mbox{ hrs, dt} = 0.01 \mbox{ hrs} \\ \hline & \mbox{Type III 24-hr 100-yr Rainfall=7.00''} \\ \hline & \mbox{Area (ac)} & \mbox{CN} & \mbox{Description} \\ \hline & \mbox{a.021} & 74 \\ \hline & \mbox{Tc Length Slope Velocity Capacity Description} \\ \hline & \mbox{(min)} & (\mbox{feet}) & (\mbox{ft/ft}) & (\mbox{ft/sec}) & \mbox{core} \\ \hline \hline & \mbox{Runoff} = 13.41 \mbox{ cfs} & \mbox{12.02 \ hrs, Volume} & 0.937 \mbox{ af, Depth} = 6.76'' \\ \hline & \mbox{Runoff} & \mbox{secs TR-20 method, UH=SCS, Time Span= 0.00-48.00 \ hrs, dt= 0.01 \ hrs \\ \hline & \mbox{Type III 24-hr 100-yr Rainfall=7.00''} \\ \hline & \mbox{Area (ac)} & \mbox{CN} & \mbox{Description} \\ \hline & \mbox{1.663} & \mbox{98} \\ \hline & \mbox{Tc Length Slope Velocity Capacity Description} \\ \hline & \mbox{1.663} & \mbox{98} \\ \hline & \mbox{Tc Length Slope Velocity Capacity Description} \\ \hline & \mbox{(min)} & (\mbox{feet}) & \mbox{(ft/ft)} & (\mbox{ft/sec}) & \mbox{(cfs)} \\ \hline \end{array}$	10.435 70	
Subcatchment s03-2:         Runoff       =       16.85 cfs @       12.02 hrs, Volume=       1.018 af, Depth= 4.04"         Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs       Type III 24-hr 100-yr Rainfall=7.00"         Area (ac)       CN       Description         3.021       74         Tc       Length       Slope         Velocity       Capacity       Description         1.5       Direct Entry,         Subcatchment s03-2(IC): s03-2 Impervious Cover         Runoff       =       13.41 cfs @       12.02 hrs, Volume=       0.937 af, Depth= 6.76"         Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs       Type III 24-hr 100-yr Rainfall=7.00"         Area (ac)       CN       Description         1.663       98       Tc       Length       Slope       Velocity       Capacity       Description         (min)       (feet)       Slope       Velocity       Capacity       Description		Description
$\begin{array}{rcl} \operatorname{Runoff} &=& 16.85 \ cfs @ 12.02 \ hrs, \ Volume= & 1.018 \ af, \ Depth= 4.04" \\ Runoff by SCS TR-20 \ method, UH=SCS, \ Time \ Span= 0.00-48.00 \ hrs, \ dt= 0.01 \ hrs \\ Type \ III \ 24-hr \ 100-yr \ Rainfall=7.00" \\ \hline \\ \hline \\ \underline{Area (ac)  CN  Description \\ 3.021  74 \\ \hline \\ \hline \\ Tc \ Length  Slope \ Velocity \ Capacity \ Description \\ \hline \\ \hline \\ (fin)  (feet)  (ft/ft)  (ft/sec)  (cfs) \\ \hline \\ $	28.8	Direct Entry,
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs         Type III 24-hr 100-yr Rainfall=7.00"         Area (ac) CN Description         3.021       74         Tc Length Slope Velocity Capacity Description         (min) (feet) (ft/ft) (ft/sec) (cfs)         1.5       Direct Entry,         Subcatchment s03-2(IC): s03-2 Impervious Cover         Runoff       = 13.41 cfs @ 12.02 hrs, Volume=         Non-48.00 hrs, dt= 0.01 hrs         Type III 24-hr 100-yr Rainfall=7.00"         Area (ac) CN Description         1.663 98         Tc Length Slope Velocity Capacity Description         Tc Length Slope Velocity Capacity Description         (refs)	Subcate	chment s03-2:
Type III 24-hr 100-yr Rainfall=7.00"         Area (ac)       CN       Description         3.021       74         Tc Length Slope Velocity Capacity Description         (min)       (feet)       (ft/ft)       (ft/sec)       (cfs)         1.5       Direct Entry,         Subcatchment s03-2(IC): s03-2 Impervious Cover         Runoff       =       13.41 cfs @       12.02 hrs, Volume=       0.937 af, Depth= 6.76"         Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs         Type III 24-hr 100-yr Rainfall=7.00"         Area (ac)       CN       Description         1.663       98         Tc Length Slope Velocity Capacity Description         (min)       (feet)       (ft/ft)       (ft/sec)       (cfs)	Runoff = 16.85 cfs @ 12.02 hrs, Volum	ne= 1.018 af, Depth= 4.04"
3.021       74         Tc       Length       Slope       Velocity       Capacity       Description         (min)       (feet)       (ft/ft)       (ft/sec)       (cfs)         1.5       Direct Entry,         Subcatchment s03-2(IC): s03-2 Impervious Cover         Runoff       =       13.41 cfs @       12.02 hrs, Volume=       0.937 af, Depth= 6.76"         Runoff by SCS TR-20 method, UH=SCS, Time Span=       0.00-48.00 hrs, dt= 0.01 hrs         Type III 24-hr 100-yr       Rainfall=7.00"         Area (ac)       CN       Description         1.663       98         Tc       Length       Slope       Velocity       Capacity         Min       (feet)       (ft/ft)       (ft/sec)       (cfs)		pan= 0.00-48.00 hrs, dt= 0.01 hrs
Tc       Length (feet)       Slope (ft/ft) (ft/sec)       Capacity (cfs)       Description         1.5       Direct Entry,         Subcatchment s03-2(IC): s03-2 Impervious Cover         Runoff       =       13.41 cfs @       12.02 hrs, Volume=       0.937 af, Depth= 6.76"         Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs       Type III 24-hr 100-yr Rainfall=7.00"         Area (ac)       CN       Description         1.663       98         Tc       Length       Slope       Velocity       Capacity       Description         (min)       (feet)       (ft/ft)       (ft/sec)       (cfs)	Area (ac) CN Description	
(min)       (feet)       (ft/ft)       (ft/sec)       (cfs)         1.5       Direct Entry,         Subcatchment s03-2(IC): s03-2 Impervious Cover         Runoff       =       13.41 cfs @       12.02 hrs, Volume=       0.937 af, Depth=       6.76"         Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt=       0.01 hrs         Type III 24-hr 100-yr       Rainfall=7.00"         Area (ac)       CN       Description         1.663       98         Tc       Length       Slope       Velocity       Capacity       Description         (min)       (feet)       (ft/ft)       (ft/sec)       (cfs)	3.021 74	
Subcatchment s03-2(IC): s03-2 Impervious Cover         Runoff = 13.41 cfs @ 12.02 hrs, Volume= 0.937 af, Depth= 6.76"         Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs         Type III 24-hr 100-yr Rainfall=7.00"         Area (ac)       CN         Description         1.663       98         Tc       Length         Slope       Velocity       Capacity         Description         (min)       (ft/ft)         (ft/ft)       (ft/sec)		Description
Runoff       =       13.41 cfs @       12.02 hrs, Volume=       0.937 af, Depth= 6.76"         Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs         Type III 24-hr 100-yr Rainfall=7.00"         Area (ac)       CN       Description         1.663       98         Tc       Length       Slope         Velocity       Capacity       Description         (min)       (feet)       (ft/ft)	1.5	Direct Entry,
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=7.00" <u>Area (ac) CN Description</u> 1.663 98 <u>Tc Length Slope Velocity Capacity Description</u> <u>(min) (feet) (ft/ft) (ft/sec) (cfs)</u>	Subcatchment s03-2	(IC): s03-2 Impervious Cover
Type III 24-hr 100-yr Rainfall=7.00"         Area (ac)       CN       Description         1.663       98         Tc       Length       Slope       Velocity       Capacity       Description         (min)       (feet)       (ft/ft)       (ft/sec)       (cfs)	Runoff = 13.41 cfs @ 12.02 hrs, Volum	me= 0.937 af, Depth= 6.76"
1.663 98 Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		pan= 0.00-48.00 hrs, dt= 0.01 hrs
1.663 98 Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	Area (ac) CN Description	
(min) (feet) (ft/ft) (ft/sec) (cfs)		
		Description
		Direct Entry,

Proposed Conditions_10454-01Type III 24-hr 100-yrRainfall=7.0Prepared by The Chazen CompaniesPage 2HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems4/10/2006 3:16:19 F				
Subcatchment s03-2(OW): s03-2 Open Water				
Runoff = 0.45 cfs @ 12.00 hrs, Volume= 0.031 af, Depth= 7.00"				
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=7.00"				
Area (ac) CN Description 0.054 100				
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)				
0.0 Direct Entry,				
Subcatchment s04-1:				
Runoff = 29.99 cfs @ 12.09 hrs, Volume= 2.145 af, Depth= 3.41"				
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=7.00"				
Area (ac) CN Description				
7.549 68				
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)				
6.1 Direct Entry,				
Subcatchment s05-1:				
Runoff = 16.18 cfs @ 12.21 hrs, Volume= 1.541 af, Depth= 2.70"				
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=7.00"				
Area (ac) CN Description				
6.842 61				
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)				
14.4 Direct Entry,				
Subcatchment s06-0:				
Runoff = 20.64 cfs @ 12.25 hrs, Volume= 2.102 af, Depth= 2.80"				
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=7.00"				

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Area (ac) CN Description		
9.007 62		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
17.3 Direct Entry,		
Subcatchment s06-0(OW): s06 Open Water		
Runoff = 3.60 cfs @ 12.00 hrs, Volume= 0.250 af, Depth= 7.00"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=7.00"		
Area (ac) CN Description 0.428 100		
Subcatchment s07-1:		
Subcatchment S07-1.		
Runoff = 14.43 cfs @ 12.14 hrs, Volume= 1.165 af, Depth= 3.00"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=7.00"		
Area (ac) CN Description 4.656 64		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
9.3 Direct Entry,		
Subcatchment s07-1(OW): s07 Open Water		
Runoff = 4.25 cfs @ 12.00 hrs, Volume= 0.295 af, Depth= 7.00"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=7.00"		
Area (ac) CN Description 0.506 100		
0.000 100		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
0.0 Direct Entry,		

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Subcatchment s08-1:		
Runoff = 42.17 cfs @ 12.36 hrs, Volume= 5.018 af, Depth= 2.60"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=7.00"		
Area (ac) CN Description 23.126 60		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
24.5Direct Entry,		
Subcatchment s08-2:		
Runoff = 24.99 cfs @ 12.17 hrs, Volume= 2.165 af, Depth= 2.90"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=7.00"		
Area (ac) CN Description 8.958 63		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
11.4Direct Entry,		
Subcatchment s08-2(IC):		
Runoff = 42.39 cfs @ 12.04 hrs, Volume= 3.112 af, Depth= 6.76"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=7.00"		
Area (ac) CN Description 5.524 98		
Tc       Length       Slope       Velocity       Capacity       Description         (min)       (feet)       (ft/ft)       (ft/sec)       (cfs)		
2.9 Direct Entry,		
Subcatchment s08-2(OW):		
Runoff = 1.61 cfs @ 12.00 hrs, Volume= 0.112 af, Depth= 7.00"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=7.00"		

Proposed Conditions_10454-01 Prepared by The Chazen Companies HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer System	Type III 24-hr 100-yr         Rainfall=7.00"           Page 261           tems         4/10/2006         3:16:19 PM	
Area (ac) CN Description		
0.192 100		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
0.0 Direct Entry,		
Subcatchment s08-3:		
Runoff = 4.11 cfs @ 12.20 hrs, Volume= 0.383	af, Depth= 2.70"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hr Type III 24-hr 100-yr Rainfall=7.00"	s, dt= 0.01 hrs	
Area (ac) CN Description		
1.700 61		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
13.6Direct Entry,		
Subcatchment s08-3(IC): s08-3 Impe	rvious Cover	
Runoff = 8.89 cfs @ 12.01 hrs, Volume= 0.612	af, Depth= 6.76"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hr Type III 24-hr 100-yr Rainfall=7.00"	rs, dt= 0.01 hrs	
Area (ac) CN Description		
1.086 98		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
1.0 Direct Entry,		
Subcatchment s08-3(OW): s08-3 O	pen Water	
Runoff = 0.35 cfs @ 12.00 hrs, Volume= 0.024 a	af, Depth= 7.00"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hr Type III 24-hr 100-yr Rainfall=7.00"	rs, dt= 0.01 hrs	
Area (ac) CN Description		
0.042 100		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
0.0 Direct Entry,		

Proposed Conditions_10454-01Type III 24-hr 100-yr Rainfall=7.00"Prepared by The Chazen CompaniesPage 262HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems4/10/2006 3:16:19 PM		
Subcatchment s09-1:		
Runoff = 6.94 cfs @ 12.13 hrs, Volume= 0.565 af, Depth= 2.60"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=7.00"		
Area (ac) CN Description 2.604 60		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
9.0 Direct Entry,		
Subcatchment s09-2:		
Runoff = 47.76 cfs @ 12.29 hrs, Volume= 5.128 af, Depth= 3.31"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=7.00"		
Area (ac) CN Description		
18.608 67		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
20.6 Direct Entry,		
Subcatchment s09-2(IC): s09-2 Impervious Cover		
Runoff = 18.18 cfs @ 12.04 hrs, Volume= 1.316 af, Depth= 6.76"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=7.00"		
Area (ac) CN Description 2.336 98		
Tc       Length       Slope       Velocity       Capacity       Description         (min)       (feet)       (ft/ft)       (ft/sec)       (cfs)		
2.5 Direct Entry,		
Subcatchment s09-2(OW): s09-2 Open Water		
Runoff = 1.98 cfs @ 12.00 hrs, Volume= 0.138 af, Depth= 7.00"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=7.00"		

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HydroCAD® 7.00         s/n         000927         ©         1986-2003         Applied         Microcomputer         Systems         4/10/2006         3:16:19         PM		
Area (ac) CN Description		
0.236 100		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
0.0 Direct Entry,		
Subcatchment s09-3:		
Runoff = 12.91 cfs @ 12.15 hrs, Volume= 1.085 af, Depth= 3.41"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=7.00"		
Area (ac) CN Description		
3.818 68		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
10.9 Direct Entry,		
Subcatchment s10-1:		
Runoff = 18.67 cfs @ 12.40 hrs, Volume= 2.284 af, Depth= 3.41"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=7.00"		
Area (ac) CN Description		
8.038 68		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
27.9 Direct Entry,		
Subcatchment s10-1(OW): s10 Open Water		
Runoff = 6.97 cfs @ 12.00 hrs, Volume= 0.484 af, Depth= 7.00"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=7.00"		
Area (ac) CN Description 0.830 100		

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Subcatchment s	13-1:	
Runoff = 12.34 cfs @ 12.05 hrs, Volume=	0.800 af, Depth= 2.70"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=7.00"		
Area (ac) CN Description		
3.555 61		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	1	
2.8 Direct Entr	y,	
Subcatchment s13-1(IC): s13-1 Impervious Cover		
Runoff = 48.98 cfs @ 12.04 hrs, Volume=	3.583 af, Depth= 6.76"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=7.00"		
Area (ac) CN Description		
6.360 98		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	1	
2.8 Direct Entr	ſy,	
Subcatchment s13-1(OW): s	13-1 Open Water	
Runoff = 1.10 cfs @ 12.00 hrs, Volume=	0.076 af, Depth= 7.00"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=7.00"		
Area (ac) CN Description		
0.131 100		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	1	
0.0 Direct Entr	ſy,	
Subcatchment s14-1:		
Runoff = 32.88 cfs @ 12.40 hrs, Volume=	4.020 af, Depth= 3.51"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs		

Type III 24-hr 100-yr Rainfall=7.00"

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HydroCAD® 7.00         s/n         000927         ©         1986-2003         Applied         Microcomputer         Systems         4/10/2006         3:16:20         PM
Area (ac) CN Description
13.727 69
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
28.1 Direct Entry,
Subcatchment s14-1(IC): s14-1 Impervious Cover
Runoff = 14.43 cfs @ 12.03 hrs, Volume= 1.037 af, Depth= 6.76"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=7.00"
Area (ac) CN Description
1.840 98
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
2.3Direct Entry,
Subcatchment s14-1(OW): s14 Open Water
Runoff = 4.35 cfs @ 12.00 hrs, Volume= 0.302 af, Depth= 7.00"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=7.00"
Area (ac) CN Description
0.518 100
Subcatchment s14-2:
Runoff = 1.54 cfs @ 12.10 hrs, Volume= 0.113 af, Depth= 2.70"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=7.00"
Area (ac) CN Description
0.504 61
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
6.4     Direct Entry,

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Subcatchment s14-2(OW): s14-2 Open Water		
Runoff = 1.48 cfs @ 12.00 hrs, Volume= 0.103 af, Depth= 7.00"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=7.00"		
Area (ac) CN Description 0.176 100		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
0.0 Direct Entry,		
Subcatchment s14-3:		
Runoff = 25.63 cfs @ 12.12 hrs, Volume= 1.989 af, Depth= 3.51"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=7.00"		
Area (ac) CN Description		
6.794 69		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
8.5 Direct Entry,		
Subcatchment s14-3(IC): s14-3 Impervious Cover		
Runoff = 64.92 cfs @ 12.04 hrs, Volume= 4.766 af, Depth= 6.76"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=7.00"		
Area (ac) CN Description		
8.460 98		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
2.9Direct Entry,		
Subcatchment s16-1:		
Runoff = 105.25 cfs @ 12.27 hrs, Volume= 10.935 af, Depth= 3.31"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=7.00"		

Type III 24-hr 100-yr Rainfall=7.00"

Proposed Conditions_10454-01	Type III 24-hr 100-yr Rainfall=7.00"	
Prepared by The Chazen Companies HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied	Page 267 d Microcomputer Systems 4/10/2006 3:16:20 PM	
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Area (ac) CN Description		
39.680 67		
Tc Length Slope Velocity Capacity (min) (feet) (ft/ft) (ft/sec) (cfs)	Description	
19.0	Direct Entry,	
Subcatchment s16-1(OW): s16-1 Open Water		
Runoff = 44.97 cfs @ 12.00 hrs, Volu	me= 3.121 af, Depth= 7.00"	
Runoff by SCS TR-20 method, UH=SCS, Time S Type III 24-hr 100-yr Rainfall=7.00"	span= 0.00-48.00 hrs, dt= 0.01 hrs	
Area (ac) CN Description		
5.351 100		
Tc Length Slope Velocity Capacity	Description	
(min) (feet) (ft/ft) (ft/sec) (cfs)	Direct Fature	
0.0	Direct Entry,	
Subcat	tchment s16-2:	
Runoff = 8.04 cfs @ 12.21 hrs, Volu	me= 0.772 af, Depth= 4.26"	
Runoff by SCS TR-20 method, UH=SCS, Time S Type III 24-hr 100-yr Rainfall=7.00"	span= 0.00-48.00 hrs, dt= 0.01 hrs	
Area (ac) CN Description		
2.176 76		
Tc Length Slope Velocity Capacity (min) (feet) (ft/ft) (ft/sec) (cfs)	Description	
15.8	Direct Entry,	
Subcat	tchment s17-1:	
Dunoff 11.22 of a 12.50 bro Value	ma 1.528 of Donth 2.00"	
Runoff = $11.32 \text{ cfs} @ 12.50 \text{ hrs}$ , Volu	me= 1.528 af, Depth= 3.00"	
Runoff by SCS TR-20 method, UH=SCS, Time S Type III 24-hr 100-yr Rainfall=7.00"	Span= 0.00-48.00 hrs, dt= 0.01 hrs	
Area (ac) CN Description		
6.110 64		
Tc Length Slope Velocity Capacity	Description	
(min) (feet) (ft/ft) (ft/sec) (cfs)		
33.8	Direct Entry,	

Proposed Conditions_10454-01	Type III 24-hr 100-yr Rainfall=7.00"	
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Subcatchment s17-1(OW): s17-1 Open Water		
Runoff = 1.38 cfs @ 12.00 hrs, Volume=	0.096 af, Depth= 7.00"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-44 Type III 24-hr 100-yr Rainfall=7.00"	3.00 hrs, dt= 0.01 hrs	
Area (ac) CN Description		
0.164 100		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
0.0 Direct Entry	Ι,	
Subcatchment s1	7-2:	
Runoff = 82.52 cfs @ 13.29 hrs, Volume= 2	0.318 af, Depth= 3.20"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48 Type III 24-hr 100-yr Rainfall=7.00"	3.00 hrs, dt= 0.01 hrs	
Area (ac) CN Description		
76.086 66		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
95.7 Direct Entry	Ι,	
Subcatchment s1	7-3:	
Runoff = 62.26 cfs @ 12.45 hrs, Volume=	7.979 af, Depth= 3.20"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48 Type III 24-hr 100-yr Rainfall=7.00"	3.00 hrs, dt= 0.01 hrs	
Area (ac) CN Description		
29.880 66		
TcLengthSlopeVelocityCapacityDescription(min)(feet)(ft/ft)(ft/sec)(cfs)		
30.7 Direct Entry	1,	
Subcatchment s1	8-1:	
Runoff = 21.34 cfs @ 12.23 hrs, Volume=	2.108 af, Depth= 3.00"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48 Type III 24-hr 100-yr Rainfall=7.00"	3.00 hrs, dt= 0.01 hrs	

Proposed Conditions_10454-01	Type III 24-hr 100-yr Rainfall=7.00"
Prepared by The Chazen Companies HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer S	Page 269 Systems 4/10/2006 3:16:20 PM
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Area (ac) CN Description	
8.429 64	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
16.4 <b>Direct Entry</b> ,	
Subcatchment s18-1(OW): s18-1	Open Water
Runoff = 3.97 cfs @ 12.00 hrs, Volume= 0.2	75 af, Depth= 7.00"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 Type III 24-hr 100-yr Rainfall=7.00"	) hrs, dt= 0.01 hrs
Area (ac) CN Description	
0.472 100	
Subcatchment s18-2	).
	-
Runoff = 31.00 cfs @ 12.26 hrs, Volume= 3.13	39 af, Depth= 3.51"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 Type III 24-hr 100-yr Rainfall=7.00"	) hrs, dt= 0.01 hrs
Area (ac) CN Description	
10.721 69	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
18.0 Direct Entry,	
Subcatchment s19-0	):
Runoff = 22.44 cfs @ 12.61 hrs, Volume= 3.30	67 af, Depth= 2.60"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 Type III 24-hr 100-yr Rainfall=7.00"	) hrs, dt= 0.01 hrs
Area (ac) CN Description	
15.520 60	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
40.4 Direct Entry,	

Proposed Conditions_10454-01 Prepared by The Chazen Companies HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer S	Type III 24-hr 100-yr         Rainfall=7.00"           Page 270           Systems         4/10/2006 3:16:20 PM		
Subcatchment s20-1	Subcatchment s20-1:		
Runoff = 20.85 cfs @ 12.30 hrs, Volume= 2.28	86 af, Depth= 3.20"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=7.00"			
Area (ac) CN Description			
8.559 66			
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)			
21.5 Direct Entry,			
Subcatchment s20-1(OW): s20-1	l Open Water		
Runoff = 16.54 cfs @ 12.00 hrs, Volume= 1.14	48 af, Depth= 7.00"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=7.00"			
Area (ac) CN Description			
1.968 100			
Subcatchment s20-2			
Runoff = 35.59 cfs @ 12.12 hrs, Volume= 2.74	48 af, Depth= 4.04"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 Type III 24-hr 100-yr Rainfall=7.00"	) hrs, dt= 0.01 hrs		
Area (ac) CN Description			
8.157 74			
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)			
8.4 Direct Entry,			
Subcatchment s20-2(IC): s20-2 Impervious Cover			
Runoff = 36.75 cfs @ 12.07 hrs, Volume= 2.88	80 af, Depth= 6.76"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=7.00"			
Area (ac) CN Description 5.112 98			

Proposed Conditions_10454-01 Type III 24-hr 100-yr Rainfall=7.00		
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Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
4.7 Direct Entry,		
Subcatchment s20-2(OW): s20-2 Open Water		
Runoff = 2.03 cfs @ 12.00 hrs, Volume= 0.141 af, Depth= 7.00"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=7.00"		
Area (ac) CN Description		
0.242 100		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
0.0 Direct Entry,		
Subcatchment s20-3:		
Runoff = 18.89 cfs @ 12.30 hrs, Volume= 2.076 af, Depth= 3.62"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=7.00"		
Area (ac) CN Description		
6.886 70		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
22.0   Direct Entry,		
Subcatchment s21-1:		
Runoff = 185.20 cfs @ 12.23 hrs, Volume= 18.171 af, Depth= 3.41"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=7.00"		
Area (ac) CN Description		
63.942 68		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
16.6 <b>Direct Entry</b> ,		

Proposed Conditions_10454-01Type III 24-hr 100-yrRainfall=7.00"Prepared by The Chazen CompaniesPage 272HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems4/10/2006 3:16:20 PM			
Subcatchment s21-1(OW):			
Runoff = 102.81 cfs @ 12.00 hrs, Volume= 7.137 af, Depth= 7.00"			
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=7.00"			
Area (ac) CN Description			
12.235 100			
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)			
0.0 Direct Entry,			
Subcatchment s21-2:			
Runoff = 49.11 cfs @ 12.45 hrs, Volume= 6.314 af, Depth= 3.62"			
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=7.00"			
Area (ac) CN Description			
20.941 70			
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)			
31.4 Direct Entry,			
Subcatchment s21-3:			
Runoff = 34.19 cfs @ 12.16 hrs, Volume= 2.886 af, Depth= 4.04"			
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=7.00"			
Area (ac) CN Description			
8.567 74			
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)			
11.2Direct Entry,			
Subcatchment s21-4:			
Runoff = 10.21 cfs @ 12.19 hrs, Volume= 0.935 af, Depth= 3.31"			
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=7.00"			

Type III 24-hr 100-yr Rainfall=7.00"

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Area (ac) CN Description		
3.392 67		
Tc Length Slope Velocity Capacity Description		
(min) (feet) (ft/ft) (ft/sec) (cfs)		
13.8Direct Entry,		
Subcatchment s21-4(IC): s21-4 Impervious Cover		
Runoff = 13.20 cfs @ 12.02 hrs, Volume= 0.926 af, Depth= 6.76"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=7.00"		
Area (ac) CN Description		
1.643 98		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
1.6 Direct Entry,		
Subcatchment s21-4(OW): s21-4 Open Water		
Runoff = 0.98 cfs @ 12.00 hrs, Volume= 0.068 af, Depth= 7.00"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=7.00"		
Area (ac) CN Description		
0.117 100		
Subcatchment s21-5:		
Runoff = 9.09 cfs @ 12.19 hrs, Volume= 0.829 af, Depth= 4.15"		
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=7.00"		
Area (ac) CN Description		
2.398 75		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
13.9 Direct Entry,		

Proposed Conditions_10454-01Type III 24-hr 100-yrRainfall=7.00"Prepared by The Chazen CompaniesPage 274HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems4/10/2006 3:16:21 PM			
Subcatchment s21-6:			
Runoff = 19.43 cfs @ 12.23 hrs, Volume= 1.938 af, Depth= 4.26"			
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=7.00"			
Area (ac) CN Description			
5.463 76			
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)			
17.5 Direct Entry,			
Subcatchment s21-6(IC): s21-6 Impervious Cover			
Runoff = 5.18 cfs @ 12.02 hrs, Volume= 0.362 af, Depth= 6.76"			
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=7.00"			
Area (ac) CN Description			
0.643 98			
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)			
1.5     Direct Entry,			
Subcatchment s21-6(OW): s21-6 Open Water			
Runoff = 0.64 cfs @ 12.00 hrs, Volume= 0.044 af, Depth= 7.00"			
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=7.00"			
Area (ac) CN Description			
0.076 100			
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)			
0.0 Direct Entry,			
Subcatchment s21-7:			
Runoff = 12.07 cfs @ 12.18 hrs, Volume= 1.094 af, Depth= 3.00"			
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=7.00"			

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Area (ac) CN Description		
4.375 64		
Tc Length Slope Velocity Capacity Descript (min) (feet) (ft/ft) (ft/sec) (cfs)	ion	
13.1Direct E	ntry,	
Subcatchment s21-7(IC): s2	1-7 Impervious Cover	
Runoff = 30.27 cfs @ 12.04 hrs, Volume=	2.192 af, Depth= 6.76"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.0 Type III 24-hr 100-yr Rainfall=7.00"	10-48.00 hrs, dt= 0.01 hrs	
Area (ac) CN Description		
3.890 98		
Tc Length Slope Velocity Capacity Descript (min) (feet) (ft/ft) (ft/sec) (cfs)	ion	
2.5 Direct E	ntry,	
Subcatchment s21-7(OW)	: s21-7 Open Water	
Runoff = 0.76 cfs @ 12.00 hrs, Volume=	0.053 af, Depth= 7.00"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=7.00"		
Area (ac) CN Description		
0.090 100		
Tc Length Slope Velocity Capacity Descript (min) (feet) (ft/ft) (ft/sec) (cfs)	ion	
0.0 Direct E	ntry,	
Subcatchment s22-1:		
Runoff = 61.27 cfs @ 12.20 hrs, Volume=	5.705 af, Depth= 3.83"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.0 Type III 24-hr 100-yr Rainfall=7.00"	0-48.00 hrs, dt= 0.01 hrs	
Area (ac) CN Description		
17.878 72		
Tc Length Slope Velocity Capacity Descript (min) (feet) (ft/ft) (ft/sec) (cfs)	ion	
14.7 Direct E	ntry,	

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Subcatchment s22-1(OW): s22-1 Open Water			
Runoff = 1.14 cfs @ 12.00 hrs, Volume= 0.079 af, Depth= 7.00"			
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=7.00"			
Area (ac) CN Description 0.136 100			
Subcatchment s22-2:			
Runoff = 111.37 cfs @ 12.34 hrs, Volume= 12.745 af, Depth= 3.41"			
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=7.00"			
Area (ac) CN Description 44.848 68			
Tc Length Slope Velocity Capacity Description			
(min) (feet) (ft/ft) (ft/sec) (cfs) 24.0 Direct Entry,			
Subcatchment s23-1:			
Runoff = 65.76 cfs @ 12.53 hrs, Volume= 9.293 af, Depth= 3.83" Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=7.00"			
Area (ac) CN Description			
29.123 72			
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)			
38.5 Direct Entry,			
Subcatchment s23-2:			
Runoff = 47.93 cfs @ 12.06 hrs, Volume= 3.180 af, Depth= 4.37"			
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=7.00"			
Area (ac) CN Description 8.741 77			

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Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)			
4.0 Direct Entry,			
Subcatchment s23-2(IC): s23-2 Impervious Cover			
Runoff = 52.97 cfs @ 12.06 hrs, Volume= 4.048 af, Depth= 6.76"			
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=7.00"			
Area (ac) CN Description			
7.185 98			
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)			
4.0 Direct Entry,			
Subcatchment s23-2(OW): s23-2 Open Water			
Runoff = 1.41 cfs @ 12.00 hrs, Volume= 0.098 af, Depth= 7.00"			
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=7.00"			
Area (ac) CN Description			
0.168 100			
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)			
0.0 Direct Entry,			
Subcatchment s24-0:			
Runoff = 67.08 cfs @ 12.45 hrs, Volume= 8.541 af, Depth= 3.62"			
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=7.00"			
Area (ac) CN Description			
28.325 70			
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)			
30.7 Direct Entry,			

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Subcatchment s25-0:			
Runoff = 33.89 cfs @ 12.28 hrs, Volume= 3.622 af, Depth= 3.20"			
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=7.00"			
Area (ac) CN Description			
13.562 66			
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)			
20.2 Direct Entry,			
Reach 25R:			
Overland Flow Reach			
Inflow Area =       15.520 ac, Inflow Depth =       2.60" for 100-yr event         Inflow =       15.19 cfs @       12.96 hrs, Volume=       3.366 af         Outflow =       15.10 cfs @       13.02 hrs, Volume=       3.366 af, Atten= 1%, Lag= 3.2 min			
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 2.5 fps, Min. Travel Time= 4.2 min Avg. Velocity = 0.7 fps, Avg. Travel Time= 14.7 min			
Peak Depth= 0.32' @ 13.02 hrs Capacity at bank full= 175.17 cfs Inlet Invert= 560.00', Outlet Invert= 512.00' 50.00' x 1.00' deep Parabolic Channel, n= 0.060 Length= 620.0' Slope= 0.0774 '/'			
Reach r03-1:			
Overland Flow Reach Requires more survey			
Inflow Area =       11.485 ac, Inflow Depth =       3.41" for 100-yr event         Inflow =       21.78 cfs @       12.60 hrs, Volume=       3.264 af         Outflow =       21.68 cfs @       12.62 hrs, Volume=       3.264 af, Atten= 0%, Lag= 1.4 min			
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 6.0 fps, Min. Travel Time= 2.2 min Avg. Velocity = 2.2 fps, Avg. Travel Time= 5.8 min			
Peak Depth= 0.76' @ 12.62 hrs Capacity at bank full= 92.14 cfs Inlet Invert= 845.00', Outlet Invert= 728.00' 10.00' x 1.50' deep Parabolic Channel, n= 0.060 Length= 785.0' Slope= 0.1490 '/'			

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# Reach r04-1:

Channel

Inflow Area = Inflow = Outflow =	26.658 ac, Inflow Depth = 3.78" 51.27 cfs @ 12.46 hrs, Volume= 51.26 cfs @ 12.47 hrs, Volume=	= 8.392 af	
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 8.6 fps, Min. Travel Time= 0.6 min Avg. Velocity = 2.2 fps, Avg. Travel Time= 2.5 min			
		Length= 330.0' Slope= 0.1621 '/'	

## Reach r08-1a:

Man Made Ditch Inverts of pipe to be surveyed

Inflow Area	a =	93.367 ac, Inflow Depth = 2.12"	for 100-yr event
Inflow	=	36.22 cfs @ 12.82 hrs, Volume=	16.479 af
Outflow	=	36.22 cfs @ 12.82 hrs, Volume=	16.479 af, Atten= 0%, Lag= 0.3 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 9.6 fps, Min. Travel Time= 0.4 min Avg. Velocity = 5.6 fps, Avg. Travel Time= 0.7 min

Peak Depth= 0.68' @ 12.82 hrs Capacity at bank full= 81.88 cfs Inlet Invert= 607.00', Outlet Invert= 587.00' 10.00' x 1.00' deep Parabolic Channel, n= 0.027 Length= 226.0' Slope= 0.0885 '/'

## Reach r08-1b:

24" HDPE Inverts to be surveyed

Inflow Area = 93.367 ac, Inflow Depth = 2.12" for 100-yr event Inflow = 36.22 cfs @ 12.82 hrs, Volume= 16.479 af Outflow = 36.22 cfs @ 12.83 hrs, Volume= 16.479 af, Atten= 0%, Lag= 0.2 min Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 24.2 fps, Min. Travel Time= 0.2 min Avg. Velocity = 14.7 fps, Avg. Travel Time= 0.3 min Peak Depth= 0.96' @ 12.83 hrs Capacity at bank full= 77.17 cfs Inlet Invert= 587.00', Outlet Invert= 557.75' 24.0" Diameter Pipe n= 0.012 Length= 295.0' Slope= 0.0992 '/'

# Reach r08-1c:

Ditch Pipe inverts to be surveyed

Inflow Are	a =	93.367 ac, Inflow Depth = 2.12" for 100-yr event	
Inflow	=	36.22 cfs @ 12.83 hrs, Volume= 16.479 af	
Outflow	=	36.22 cfs @ 12.84 hrs, Volume= 16.479 af, Atten= 0%	, Lag= 1.1 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 9.2 fps, Min. Travel Time= 1.1 min Avg. Velocity = 5.1 fps, Avg. Travel Time= 1.9 min

Peak Depth= 0.71' @ 12.84 hrs Capacity at bank full= 76.65 cfs Inlet Invert= 557.75', Outlet Invert= 512.00' 10.00' x 1.00' deep Parabolic Channel, n= 0.027 Length= 590.0' Slope= 0.0775 '/

## Reach r08-1d: Amenia Creek/Cascade Brook

(Steam) parallel to Route 22 Base Flow estimated from field observation (see field note 21)

Inflow Area = 114.957 ac, Inflow Depth = 19.08" for 100-yr event Inflow = 123.55 cfs @ 12.26 hrs, Volume= 182.738 af, Incl. 40.00 cfs Base Flow Outflow = 122.30 cfs @ 12.32 hrs, Volume= 182.493 af, Atten= 1%, Lag= 3.4 min Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 4.1 fps, Min. Travel Time= 3.3 min Avg. Velocity = 3.1 fps, Avg. Travel Time= 4.3 min

Peak Depth= 4.34' @ 12.32 hrs Capacity at bank full= 104.49 cfs Inlet Invert= 512.00', Outlet Invert= 504.00' 10.00' x 4.00' deep Parabolic Channel, n= 0.060 Length= 805.0' Slope= 0.0099 '/'

# Reach r13-1:

 Inflow Area =
 2.176 ac, Inflow Depth = 4.26" for 100-yr event

 Inflow =
 8.04 cfs @ 12.21 hrs, Volume=
 0.772 af

 Outflow =
 7.98 cfs @ 12.24 hrs, Volume=
 0.772 af, Atten= 1%, Lag= 1.3 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 9.8 fps, Min. Travel Time= 1.5 min Avg. Velocity = 3.6 fps, Avg. Travel Time= 4.2 min

#### Proposed Conditions\_10454-01 Type II Prepared by The Chazen Companies HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems

Peak Depth= 0.70' @ 12.24 hrs Capacity at bank full= 17.79 cfs Inlet Invert= 546.00', Outlet Invert= 524.00' 18.0" Diameter Pipe n= 0.012 Length= 900.0' Slope= 0.0244 '/'

#### Reach r14-3a:

30" HDPE Under Main Entrance Road

Inflow Area = 6.422 ac, Inflow Depth =  $3.08^{"}$  for 100-yr event Inflow = 19.67 cfs @ 12.16 hrs, Volume= 1.650 afOutflow = 19.65 cfs @ 12.16 hrs, Volume= 1.650 af, Atten= 0%, Lag= 0.4 min Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrsMax. Velocity= 15.2 fps, Min. Travel Time= 0.5 minAvg. Velocity = 5.6 fps, Avg. Travel Time= 1.3 min

Peak Depth= 0.77' @ 12.16 hrs Capacity at bank full= 94.91 cfs Inlet Invert= 526.00', Outlet Invert= 505.70' 30.0" Diameter Pipe n= 0.012 Length= 445.0' Slope= 0.0456 '/'

#### Reach r14-3b:

Grass lined channel

Inflow Area = 6.422 ac, Inflow Depth =  $3.08^{"}$  for 100-yr event 19.77 cfs @ 12.15 hrs, Volume= Inflow 1.650 af = Outflow = 19.67 cfs @ 12.16 hrs, Volume= 1.650 af, Atten= 0%, Lag= 0.6 min Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 7.3 fps, Min. Travel Time= 0.8 min Avg. Velocity = 2.6 fps, Avg. Travel Time= 2.3 min Peak Depth= 0.79' @ 12.16 hrs Capacity at bank full= 325.42 cfs Inlet Invert= 542.00', Outlet Invert= 526.00'

Inlet Invert=  $542.00^{\circ}$ , Outlet Invert=  $526.00^{\circ}$ 10.00' x 3.00' deep Parabolic Channel, n= 0.027 Length=  $360.0^{\circ}$  Slope= 0.0444 '/

#### Reach r17-1:

 Inflow Area =
 76.086 ac, Inflow Depth =
 3.20" for 100-yr event

 Inflow =
 82.52 cfs @
 13.29 hrs, Volume=
 20.318 af

 Outflow =
 82.25 cfs @
 13.32 hrs, Volume=
 20.318 af, Atten= 0%, Lag= 1.8 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 9.0 fps, Min. Travel Time= 2.6 min Avg. Velocity = 3.5 fps, Avg. Travel Time= 6.7 min

Proposed Conditions_10454-01 Prepared by The Chazen Companies HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer	Type III 24-hr 100-yr Rainfall=7.00" Page 282 Systems 4/10/2006 3:16:21 PM							
Peak Depth= 1.38' @ 13.32 hrs Capacity at bank full= 181.28 cfs Inlet Invert= 646.00', Outlet Invert= 524.00' 12.00' x 2.00' deep Parabolic Channel, n= 0.045 Length= 1,390.0' Slope= 0.0878 '/'								
Reach r18-2:								
Overland Flow Reach								
- /	000 af 000 af, Atten= 0%, Lag= 0.0 min							
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, df Max. Velocity= 0.0 fps, Min. Travel Time= 0.0 min Avg. Velocity = 0.0 fps, Avg. Travel Time= 0.0 min	t= 0.01 hrs							
Peak Depth= 0.00' @ 0.00 hrs Capacity at bank full= 434.91 cfs Inlet Invert= 973.60', Outlet Invert= 630.00' 50.00' x 1.00' deep Parabolic Channel, n= 0.060 Length= 72	20.0' Slope= 0.4772 '/'							
Reach r21-1a:								
Man Made Ditch								
- ,	vr event 326 af 312 af, Atten= 0%, Lag= 1.0 min							
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt Max. Velocity= 8.0 fps, Min. Travel Time= 1.4 min Avg. Velocity = 3.4 fps, Avg. Travel Time= 3.2 min	t= 0.01 hrs							
Peak Depth= 2.18' @ 14.15 hrs Capacity at bank full= 191.76 cfs Inlet Invert= 504.00', Outlet Invert= 494.00' 10.00' x 3.00' deep Parabolic Channel, n= 0.027 Length= 648.0' Slope= 0.0154 '/'								
Reach r21-1b:								
Overland Flow Reach								
- ,	vr event 578 af 578 af, Atten= 0%, Lag= 0.4 min							

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 4.7 fps, Min. Travel Time= 0.6 min Avg. Velocity = 1.8 fps, Avg. Travel Time= 1.5 min Proposed Conditions 10454-01 Prepared by The Chazen Companies HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems

Peak Depth= 0.56' @ 12.55 hrs Capacity at bank full= 227.81 cfs Inlet Invert= 506.70', Outlet Invert= 485.75' 50.00' x 1.00' deep Parabolic Channel, n= 0.060 Length= 160.0' Slope= 0.1309 '/'

### Reach r22-2:

**Overland Flow Reach** 

Inflow Area = Inflow = Outflow =	15.520 ac, Inflow Depth =2.60"for100-yr event15.21 cfs @12.94 hrs, Volume=3.366 af15.19 cfs @12.96 hrs, Volume=3.366 af, Atten= 0%, Lag= 1.6 min	
Max. Velocity= 4	Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs 4.9 fps, Min. Travel Time= 2.1 min 1.5 fps, Avg. Travel Time= 7.2 min	
1 2	21' @ 12.96 hrs k full= 469.25 cfs .00', Outlet Invert= 620.00'	

50.00' x 1.00' deep Parabolic Channel, n= 0.060 Length= 630.0' Slope= 0.5556 '/'

#### Reach r25-0a:

Ditch Pipe inverts need to be surveyed

Inflow Area =	67.391 ac, Inf	low Depth = 3.46"	for 100-yr event
Inflow =	60.63 cfs @ 1	2.31 hrs, Volume=	19.437 af
Outflow =	60.58 cfs @ 1	2.35 hrs, Volume=	19.434 af, Atten= 0%, Lag= 2.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 10.8 fps, Min. Travel Time= 1.7 min Avg. Velocity = 3.7 fps, Avg. Travel Time= 4.9 min

Peak Depth= 1.12' @ 12.35 hrs Capacity at bank full= 205.50 cfs Inlet Invert= 570.00', Outlet Invert= 504.00' 10.00' x 2.00' deep Parabolic Channel, n= 0.027 Length= 1,090.0' Slope= 0.0606 '/'

### Reach r25-0b: Wetland Reach

Wetland Reach Has wetland vegetation within reach

Inflow Area	a =	9.435 ac, li	nflow Depth = 2.95"	for 100-yr event	
Inflow	=	20.41 cfs @	12.31 hrs, Volume=	2.323 af	
Outflow	=	18.91 cfs @	12.39 hrs, Volume=	2.322 af, Atten= 7%	%, Lag= 5.0 min

Routing by Dvn-Stor-Ind method. Time Span= 0.00-48.00 hrs. dt= 0.01 hrs. Max. Velocity= 2.1 fps, Min. Travel Time= 6.0 min Avg. Velocity = 0.5 fps, Avg. Travel Time= 26.4 min

Peak Depth= 1.12' @ 12.39 hrs Capacity at bank full= 156.51 cfs Inlet Invert= 504.00', Outlet Invert= 499.50' 20.00' x 3.00' deep Parabolic Channel, n= 0.045 Length= 750.0' Slope= 0.0060 '/'

# Reach r25-0c: Amenia Creek/Cascade Brook

(Steam) parallel to Route 22 Base Flow estimated from field observation (see field note 21)

138.083 ac, Inflow Depth = 30.09" for 100-yr event Inflow Area = Inflow 204.19 cfs @ 12.34 hrs, Volume= 346.221 af, Incl. 40.00 cfs Base Flow = 198.76 cfs @ 12.43 hrs, Volume= Outflow = 345.537 af, Atten= 3%, Lag= 5.8 min Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 3.0 fps, Min. Travel Time= 5.3 min Avg. Velocity = 2.7 fps, Avg. Travel Time= 6.1 min Peak Depth= 7.89' @ 12.43 hrs Capacity at bank full= 67.14 cfs Inlet Invert= 504.00', Outlet Invert= 500.00' 10.00' x 4.00' deep Parabolic Channel, n= 0.060 Length= 975.0' Slope= 0.0041 '/' Pond 8P: No field note. Water spills over cart path; no storage. Inflow Area = 41.049 ac, Inflow Depth = 3.53" for 100-yr event Inflow = 48.76 cfs @ 12.27 hrs. Volume= 12.077 af Outflow 48.76 cfs @ 12.27 hrs, Volume= 12.077 af, Atten= 0%, Lag= 0.0 min = 48.76 cfs @ 12.27 hrs, Volume=

12.077 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 575.47' @ 12.27 hrs Flood Elev= 574.70' Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= (not calculated)

Primary

=

#	Routing	Invert	Outlet Devices
1	Primary	574.70'	177.0 deg Sharp-Crested Vee/Trap Weir C= 2.46

Primary OutFlow Max=48.76 cfs @ 12.27 hrs HW=575.47' TW=571.11' (Dynamic Tailwater) **1=Sharp-Crested Vee/Trap Weir** (Weir Controls 48.76 cfs @ 2.2 fps)

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# Pond p02-2:

Proposed culvert under proposed road at intersection with 44.

	Inflow Inflow Outflo Prima	w =	16.49 cfs 16.49 cfs	<ul> <li>inflow Depth = 3.20"</li> <li>12.42 hrs, Volume=</li> <li>12.42 hrs, Volume=</li> <li>12.42 hrs, Volume=</li> </ul>	= 2.076 af = 2.076 af, Atten= 0%, Lag= 0.0 min		
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 642.19' @ 12.42 hrs Flood Elev= 645.00' Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= (not calculated)							
	#	Routing	Invert	Outlet Devices			
	1	Primary	640.00'	24.0" x 100.0' long Cul	Ivert CPP and section conforming to fill Ka-	0 500	

 1 Primary
 640.00'
 24.0" x 100.0' long Culvert
 CPP, end-section conforming to fill, Ke= 0.500

 Outlet Invert= 638.00'
 S= 0.0200 '/' n= 0.012
 Cc= 0.900

Primary OutFlow Max=16.49 cfs @ 12.42 hrs HW=642.19' TW=625.91' (Dynamic Tailwater) ↓ 1=Culvert (Inlet Controls 16.49 cfs @ 5.2 fps)

# Pond p02-3:

Simulates last DMH at bottom of small road, at intersection with 44. This culvert is only used to size the drain pipe under 44.

Inflow Inflow Outflo Prima	ow =	29.15 cfs 29.15 cfs	c, Inflow Depth = 5.37"       for 100-yr event         @ 12.02 hrs, Volume=       1.828 af         @ 12.02 hrs, Volume=       1.828 af, Atten= 0%, Lag= 0.0 min         @ 12.02 hrs, Volume=       1.828 af				
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 641.96' @ 12.02 hrs Flood Elev= 645.00' Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= (not calculated)							
#	Routing	Invert	Outlet Devices				
1	Primary	635.00'	<b>24.0</b> " x <b>100.0</b> ' long Culvert CPP, projecting, no headwall, Ke= 0.900				

1	Primary	635.00	<b>24.0</b> " <b>x 100.0 long Culvert</b> CPP, projecting, no headwall, Ke= 0.900
			Outlet Invert= 634.00' S= 0.0100 '/' n= 0.012 Cc= 0.900

Primary OutFlow Max=28.99 cfs @ 12.02 hrs HW=641.89' TW=555.88' (Dynamic Tailwater)

#### Pond p03-2:

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Inflow Are	ea =	4.738 ac, Inflow Depth = $5.03$	3" for 100-yr event
Inflow	=	30.65 cfs @ 12.02 hrs, Volume	e= 1.986 af
Outflow	=	8.15 cfs @ 12.34 hrs, Volume	= 1.982 af, Atten= 73%, Lag= 18.8 min
Primary	=	8.15 cfs @ 12.34 hrs, Volume	e= 1.982 af
-			

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 774.00' Surf.Area= 2,315 sf Storage= 4,095 cf Peak Elev= 779.00' @ 12.34 hrs Surf.Area= 9,995 sf Storage= 41,432 cf (37,337 cf above start) Flood Elev= 779.00' Surf.Area= 9,991 sf Storage= 41,391 cf (37,296 cf above start) Plug-Flow detention time= 298.0 min calculated for 1.888 af (95% of inflow) Center-of-Mass det. time= 243.1 min (1,022.0 - 779.0)

#	Invert	Avail.St	orage	Storage Des	scription	
1	768.00'	51,	363 cf	Custom Sta	ige Data (Conic) L	isted below
Elevatio		Surf.Area	,	Inc.Store	Cum.Store	Wet.Area
(fee	et)	(sq-ft)	(0	cubic-feet)	(cubic-feet)	(sq-ft)
768.0	00	67		0	0	67
770.0	00	345		376	376	361
772.0	00	729		1,050	1,426	777
772.	50	842		392	1,819	901
774.0	00	2,315		2,277	4,095	2,388
774.	50	5,704		1,942	6,037	5,779
776.0	00	6,996		9,509	15,546	7,138
778.0	00	8,917		15,874	31,420	9,160
780.0	00	11,064		19,942	51,363	11,421
# R	outing	Invert	Outlet	Devices		
	rimary	774.00'			rate C= 0.600	

2 Primary 776.20' **6.0" Vert. Orifice/Grate X 2.00** C= 0.600

778.50' 3 Primary **4.0' long x 6.0' high Sharp-Crested Rectangular Weir** 2 End Contraction(s)

Primary OutFlow Max=8.15 cfs @ 12.34 hrs HW=779.00' TW=722.87' (Dynamic Tailwater)

-1=Orifice/Grate (Orifice Controls 0.52 cfs @ 10.6 fps)

-2=Orifice/Grate (Orifice Controls 3.02 cfs @ 7.7 fps)

-3=Sharp-Crested Rectangular Weir (Weir Controls 4.61 cfs @ 2.3 fps)

#### Pond p04-1:

Storage, inverts and culvert length based on assumed grading, check when final grading becomes available

Inflow Area =	34.207 ac, Inflow Depth = 3.70"	for 100-yr event
Inflow =	60.36 cfs @ 12.42 hrs, Volume=	10.537 af
Outflow =	41.99 cfs @ 12.79 hrs, Volume=	10.537 af, Atten= 30%, Lag= 22.3 min
Primary =	41.99 cfs @ 12.79 hrs, Volume=	10.537 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

# Proposed Conditions\_10454-01

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Peak Elev= 646.71' @ 12.79 hrs Surf.Area= 12,716 sf Storage= 48,817 cf Flood Elev= 648.00' Surf.Area= 15,680 sf Storage= 66,062 cf Plug-Flow detention time= 7.7 min calculated for 10.535 af (100% of inflow) Center-of-Mass det. time= 7.7 min (895.7 - 888.1)

#	Invert	Avail.St	torage	Storage Des	scription		
1	638.00'	66,	062 cf	Custom Sta	ge Data (Conic) L	isted below	
Eleva	ation	Surf.Area		Inc.Store	Cum.Store	Wet.Area	
(1	feet)	(sq-ft)	(0	cubic-feet)	(cubic-feet)	(sq-ft)	
63	8.00	0		0	0	0	
64	0.00	1,300		867	867	1,306	
64	2.00	6,180		6,876	7,743	6,203	
64	4.00	7,270		13,435	21,178	7,438	
64	6.00	11,100		18,235	39,414	11,327	
64	8.00	15,680		26,648	66,062	15,980	
#	Routing	Invert	Outlet	Devices			
1	Primary	638.00'	24.0"	x 685.0' long	Culvert CPP, er	nd-section conform	ming to fill, Ke= 0.500

Outlet Invert= 598.00' S= 0.0584 '/' n= 0.012 Cc= 0.900

Primary OutFlow Max=41.99 cfs @ 12.79 hrs HW=646.71' TW=575.45' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 41.99 cfs @ 13.4 fps)

#### Pond p06-0:

Field Note #22 Geometry to be confirmed by survey.

Inflow Area =	9.435 ac, Inflow Depth = $2.99"$	for 100-yr event
Inflow =	21.68 cfs @ 12.24 hrs, Volume=	2.352 af
Outflow =	20.41 cfs @ 12.31 hrs, Volume=	2.323 af, Atten= 6%, Lag= 3.9 min
Primary =	20.41 cfs @ 12.31 hrs, Volume=	2.323 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 506.80' Surf.Area= 18,600 sf Storage= 42,160 cf Peak Elev= 507.55' @ 12.31 hrs Surf.Area= 21,985 sf Storage= 58,063 cf (15,903 cf above start) Flood Elev= 507.10' Surf.Area= 19,958 sf Storage= 48,537 cf (6,377 cf above start) Plug-Flow detention time= 344.2 min calculated for 1.355 af (58% of inflow) Center-of-Mass det. time= 87.5 min (932.0 - 844.5 )

#	Invert	Avail.Stora	age Storage Des	cription		
1	500.00'	67,669	ef Custom Sta	<b>ge Data (Conic)</b> Li	sted below	
Elevat (fe	tion eet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
500 506 508	6.80	0 18,600 24,030	0 42,160 25,509	0 42,160 67,669	0 18,672 24,138	

FIO	Josed Col	iailions_	
Prep	ared by Th	e Chazen	Companies Page 288
Hydro	DCAD® 7.00	s/n 000927	© 1986-2003 Applied Microcomputer Systems 4/10/2006 3:16:22 PM
#	Routing	Invert	Outlet Devices
1	Primary	506.80'	<b>12.0" x 20.0' long Culvert</b> CMP, projecting, no headwall, Ke= 0.900
			Outlet Invert= 506.00' S= 0.0400 '/' n= 0.024 Cc= 0.900
2	Primary	507.10'	178.0 deg Sharp-Crested Vee/Trap Weir C= 2.46

Primary OutFlow Max=20.41 cfs @ 12.31 hrs HW=507.55' TW=505.07' (Dynamic Tailwater) -1=Culvert (Inlet Controls 1.47 cfs @ 2.3 fps) -2=Sharp-Crested Vee/Trap Weir (Weir Controls 18.94 cfs @ 1.6 fps)

#### Pond p07-1:

Field Note # 29 Outlet geometry to be confirmed by survey.

Inflow Area =	=	26.342 ac, Inflow Depth = 3.42"	for 100-yr event
Inflow =	=	18.07 cfs @ 12.14 hrs, Volume=	7.507 af
Outflow =	=	12.15 cfs @ 12.33 hrs, Volume=	7.360 af, Atten= 33%, Lag= 11.6 min
Primary =	=	12.15 cfs @ 12.33 hrs, Volume=	7.360 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 572.80' Surf.Area= 21,640 sf Storage= 56,264 cf Peak Elev= 573.79' @ 12.33 hrs Surf.Area= 26,321 sf Storage= 80,532 cf (24,268 cf above start) Flood Elev= 573.50' Surf.Area= 24,936 sf Storage= 73,351 cf (17,087 cf above start) Plug-Flow detention time= 322.9 min calculated for 6.068 af (81% of inflow) Center-of-Mass det. time= 64.4 min (1,191.1 - 1,126.7)

#	Invert	Avail.St	torage	Storage Description			
1	565.00'	85,	557 cf	Custom Sta	i <b>ge Data (Conic)</b> Li	sted below	
Elevat (fe	tion eet)	Surf.Area (sq-ft)		Inc.Store cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
565	,	0		0	0	0	
572	.80	21,640		56,264	56,264	21,735	
574	.00	27,290		29,293	85,557	27,424	
# F	Routing	Invert	Outlet	Devices			
1 F	Primary	572.80'	<b>18.0</b> "	x 20.0' long (	Culvert CMP, proj	ecting, no headwall,	Ke= 0.900
2 F	Primary	573.50'				eir X 2.00 C= 2.46	

Primary OutFlow Max=12.15 cfs @ 12.33 hrs HW=573.79' TW=571.12' (Dynamic Tailwater) -1=Culvert (Inlet Controls 3.33 cfs @ 2.7 fps)

-2=Sharp-Crested Vee/Trap Weir (Weir Controls 8.82 cfs @ 1.3 fps)

#### Pond p08-2:

Inflow Area	a =	18.762 ac, Inflow Depth = 4.62"	' for 100-yr event	
Inflow	=	85.03 cfs @ 12.04 hrs, Volume=	= 7.218 af	
Outflow	=	45.84 cfs @ 12.25 hrs, Volume=	e 6.530 af, Atten= 46%, Lag= 12.5 mi	n
Primary	=	45.84 cfs @ 12.25 hrs, Volume=	= 6.530 af	

Proposed Conditions_10454-01	Type III 24-hr 100-yr Rainfall=7.00"
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Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 550.00' Surf.Area= 8,558 sf Storage= 24,834 cf Peak Elev= 556.95' @ 12.25 hrs Surf.Area= 23,262 sf Storage= 146,533 cf (121,699 cf above start) Flood Elev= 557.00' Surf.Area= 23,344 sf Storage= 147,597 cf (122,763 cf above start) Plug-Flow detention time= 353.3 min calculated for 5.960 af (83% of inflow) Center-of-Mass det. time= 245.2 min (1,030.6 - 785.4)

#	Invert	Avail.Stora	ige Storage Des	cription		
1	544.00'	170,918	B cf Custom Sta	<b>ge Data (Conic)</b> Li	sted below	
	et)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
544 546		1,962 3,155	0 5.070	0 5,070	1,962 3,207	
548		4,454	7,572	12,642	4,577	
548		4,796	2,312	14,954	4,940	
550 550		8,558 12,948	9,880 5,339	24,834 30,173	8,726 13,120	
552 554		15,129 18,234	21,037 33,315	51,209 84,524	15,390 18,627	
556	.00	21,565	39,752	124,277	22,105	
558	.00	25,122	46,642	170,918	25,823	

#	Routing	Invert	Outlet Devices
1	Primary	550.00'	3.0" Vert. Orifice/Grate C= 0.600
2	Primary	554.09'	12.0" Vert. Orifice/Grate X 2.00 C= 0.600
3	Primary	556.00'	11.0' long x 6.0' high Sharp-Crested Rectangular Weir

2 End Contraction(s)

Primary OutFlow Max=45.83 cfs @ 12.25 hrs HW=556.95' TW=516.27' (Dynamic Tailwater) -1=Orifice/Grate (Orifice Controls 0.62 cfs @ 12.6 fps)

-2=Orifice/Grate (Orifice Controls 11.63 cfs @ 7.4 fps)

-3=Sharp-Crested Rectangular Weir (Weir Controls 33.59 cfs @ 3.3 fps)

# Pond p08-3:

Inflow Area	a =	2.828 ac, Inflow Depth = $4$ .	.32" for 100-yr event	1
Inflow	=	10.87 cfs @ 12.02 hrs, Volur	me= 1.019 af	
Outflow	=	4.90 cfs @ 12.36 hrs, Volur	me= 1.018 af,	Atten= 55%, Lag= 20.9 min
Primary	=	4.90 cfs @ 12.36 hrs, Volur	me= 1.018 af	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 528.00' Surf.Area= 1,849 sf Storage= 2,615 cf Peak Elev= 531.88' @ 12.36 hrs Surf.Area= 5,125 sf Storage= 15,730 cf (13,115 cf above start) Flood Elev= 533.00' Surf.Area= 6,389 sf Storage= 22,602 cf (19,987 cf above start) Plug-Flow detention time= 188.1 min calculated for 0.958 af (94% of inflow) Center-of-Mass det. time= 125.0 min (907.9 - 782.9)

# Proposed Conditions\_10454-01

Type III 2	24-hr 100-yr Rainfall=7.00"
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#	Invert	Avail.St	torage	Storage De	escription			
1	524.00'	28,	956 cf	Custom St	age Data (Co	nic) Li	isted below	
Elevat	tion eet)	Surf.Area (sq-ft)		Inc.Store ubic-feet)	Cum.Sto (cubic-fe		Wet.Area (sq-ft)	
524	1.00	178		0		0	178	
526	6.00	500		651	6	51	524	
526	6.50	548		262	9	13	587	
528	3.00	1,849		1,702	2,6	15	1,900	
530	0.00	3,344		5,120	7,7	34	3,437	
532	2.00	5,240		8,513	16,2	48	5,388	
534	1.00	7,538		12,709	28,9	56	7,755	
	Routing Primary	Invert 528.00'	Outlet D		Grate C= 0.6	00		
	Primary	530.00'			<b>Grate</b> $C=0.0$			

Primary OutFlow Max=4.90 cfs @ 12.36 hrs HW=531.88' TW=516.32' (Dynamic Tailwater) -1=Orifice/Grate (Orifice Controls 0.46 cfs @ 9.3 fps) -2=Orifice/Grate (Orifice Controls 4.44 cfs @ 5.7 fps)

# Pond p09-2:

Inflow Area	a =	21.180 ac, Inflow Depth = 3.73	" for 100-yr event
Inflow	=	53.86 cfs @ 12.29 hrs, Volume	= 6.582 af
Outflow	=	9.70 cfs @ 13.16 hrs, Volume	= 6.048 af, Atten= 82%, Lag= 52.7 min
Primary	=	9.70 cfs @ 13.16 hrs, Volume	= 6.048 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 586.00' Surf.Area= 10,285 sf Storage= 36,340 cf Peak Elev= 592.95' @ 13.16 hrs Surf.Area= 27,511 sf Storage= 185,705 cf (149,365 cf above start) Flood Elev= 593.00' Surf.Area= 27,610 sf Storage= 187,200 cf (150,860 cf above start) Plug-Flow detention time= 527.9 min calculated for 5.212 af (79% of inflow) Center-of-Mass det. time= 374.4 min (1,200.4 - 826.0)

#	Invert	Avail.Storage	e Storage Des	cription		
1	580.00'	214,790 c	of Custom Sta	ge Data (Conic) Lis	sted below	
Elevat (fe	ion eet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft <u>)</u>	
580	.00	3,968	0	0	3,968	
582	.00	5,102	9,046	9,046	5,198	
584	.00	6,343	11,423	20,469	6,550	
584	.50	6,670	3,253	23,722	6,907	
586	.00	10,285	12,619	36,340	10,554	
586	.50	16,887	6,725	43,066	17,159	
588	.00	19,143	27,005	70,070	19,525	
590	.00	22,349	41,451	111,521	22,890	
592	.00	25,781	48,089	159,610	26,494	
594	.00	29,439	55,180	214,790	30,336	

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#	Routing	Invert	Outlet Devices	
1	Primary	586.00'	3.0" Vert. Orifice/Grate C= 0.600	
2	Primary	588.21'	8.0" Vert. Orifice/Grate C= 0.600	

3 Primary 592.00' 2.0' long x 6.0' high Sharp-Crested Rectangular Weir 2 End Contraction(s)

Primary OutFlow Max=9.70 cfs @ 13.16 hrs HW=592.95' TW=573.79' (Dynamic Tailwater)

-1=Orifice/Grate (Orifice Controls 0.62 cfs @ 12.6 fps)

-2=Orifice/Grate (Orifice Controls 3.53 cfs @ 10.1 fps)

-3=Sharp-Crested Rectangular Weir (Weir Controls 5.55 cfs @ 3.2 fps)

# Pond p10:

Field Note #25 Need to get full story on how this pond works

Inflow Area = Inflow = Outflow =	59.531 ac, Inflow Depth = 2.04"for 100-yr event36.34 cfs @ 12.46 hrs, Volume=10.140 af0.00 cfs @ 0.00 hrs, Volume=0.000 af, Atten= 100%, Lag= 0.0 min	
Starting Elev= 49 Peak Elev= 505.9 Plug-Flow detent	tor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs 3.40' Surf.Area= 36,110 sf Storage= 101,108 cf 9' @ 48.00 hrs Surf.Area= 103,129 sf Storage= 542,824 cf (441,716 cf above sta on time= (not calculated) et. time= (not calculated)	rt)

#	Invert	Avail.Stora	ge Storage Des	cription		
1	490.00'	581,029	cf Custom Stag	ge Data (Conic) Li	sted below	
Elevatio		Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft <u>)</u>	
490.0	00	0	0	0	0	
498.4	40	36,110	101,108	101,108	36,221	
500.0	00	42,400	62,741	163,849	42,610	
502.0	00	54,880	97,012	260,861	55,187	
504.0	00	78,730	132,895	393,755	79,107	
506.0	00	109,382	187,274	581,029	109,836	

#### Pond p13-1:

No Field Note Natural depression.

Inflow Area	a =	12.222 ac, Inflow Depth =	5.14" for 100-yr event	
Inflow	=	65.83 cfs @ 12.04 hrs, Vol	blume= 5.232 af	
Outflow	=	60.24 cfs @ 12.07 hrs, Vol	blume= 5.216 af, Atten= 8%, Lag= 1.8 mir	n
Primary	=	60.24 cfs @ 12.07 hrs, Vol	blume= 5.216 af	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 524.00' Surf.Area= 5,894 sf Storage= 16,480 cf Peak Elev= 526.98' @ 12.07 hrs Surf.Area= 10,038 sf Storage= 40,670 cf (24,190 cf above start) Flood Elev= 527.00' Surf.Area= 10,067 sf Storage= 40,862 cf (24,383 cf above start) Plug-Flow detention time= 161.5 min calculated for 4.837 af (92% of inflow) Center-of-Mass det. time= 94.9 min (864.3 - 769.4)

#	Invert	Avail.S	torage	Storage Des	scription		
1	518.00'	50,	891 cf	Custom Sta	ge Data (Conic) L	isted below	
	vation (feet)	Surf.Area (sq-ft)	(0	Inc.Store cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
5	18.00	1,331		0	0	1,331	
5	20.00	2,048		3,353	3,353	2,104	
5	22.00	2,912		4,935	8,288	3,037	
5	22.50	3,150		1,515	9,803	3,294	
5	24.00	5,894		6,676	16,480	6,061	
5	26.00	8,542		14,354	30,834	8,776	
5	28.00	11,592		20,057	50,891	11,908	
<u>#</u>	Routing Primary	Invert 524.00'		Devices	rate C= 0.600		
2	Primary	525.90'	15.0' lo	Vert. Orifice/Grate C= 0.600 I long x 1.3' high Sharp-Crested Rectangular Weir d Contraction(s)			

Primary OutFlow Max=60.17 cfs @ 12.07 hrs HW=526.98' TW=502.21' (Dynamic Tailwater) 1=Orifice/Grate (Orifice Controls 0.40 cfs @ 8.1 fps) 2=Sharp-Crested Rectangular Weir (Weir Controls 59.77 cfs @ 3.7 fps)

# Pond p14-1:

Field Note #26 Need to figure out how this pond works

Inflow Area = Inflow = Outflow = Primary =	50.663 ac, Inflow Depth = 4.54"for 100-yr event178.60 cfs @ 12.08 hrs, Volume=19.168 af21.54 cfs @ 13.35 hrs, Volume=7.372 af, Atten= 88%, Lag= 75.9 min21.54 cfs @ 13.35 hrs, Volume=7.372 af				
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 497.40' Surf.Area= 22,200 sf Storage= 54,760 cf Peak Elev= 505.59' @ 48.00 hrs Surf.Area= 90,486 sf Storage= 568,606 cf (513,846 cf above start) Plug-Flow detention time= 322.4 min calculated for 6.114 af (32% of inflow) Center-of-Mass det. time= 102.8 min ( 949.0 - 846.3 )					

#	Invert	Avail.Storage	Storage Description
1	490.00'	805,062 cf	Custom Stage Data (Conic) Listed below

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Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
490.00	0	0	0	0
497.40	22,200	54,760	54,760	22,286
498.00	25,330	14,249	69,009	25,433
500.00	52,810	76,476	145,485	52,948
502.00	73,360	125,608	271,093	73,574
504.00	84,070	157,308	428,402	84,467
506.00	92,130	176,139	604,540	92,797
508.00	108,618	200,522	805,062	109,437

# Routing Invert Outlet Devices

1 Primary 500.00' **24.0" x 80.0' long Culvert** CPP, end-section conforming to fill, Ke= 0.500 Outlet Invert= 502.00' S= -0.0250 '/' n= 0.012 Cc= 0.900

Primary OutFlow Max=21.55 cfs @ 13.35 hrs HW=505.03' TW=501.89' (Dynamic Tailwater)

#### Pond p14-2:

Inflow Area	a =	15.934 ac, I	nflow Depth :	= 5.25"	for 100	)-yr event	İ	
Inflow	=	85.36 cfs @	12.05 hrs, \	Volume=	6	6.972 af		
Outflow	=	78.16 cfs @	12.09 hrs, \	Volume=	6	6.943 af,	Atten= 8%,	Lag= 2.3 min
Primary	=	78.16 cfs @	12.09 hrs, \	Volume=	6	6.943 af		

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 532.00' Surf.Area= 7,681 sf Storage= 23,903 cf Peak Elev= 534.95' @ 12.09 hrs Surf.Area= 12,297 sf Storage= 53,864 cf (29,961 cf above start) Flood Elev= 535.00' Surf.Area= 12,390 sf Storage= 54,538 cf (30,635 cf above start) Plug-Flow detention time= 152.0 min calculated for 6.395 af (92% of inflow) Center-of-Mass det. time= 80.8 min (849.6 - 768.8)

#	Invert	Avail.St	torage Storage De	escription			
1	526.00'	66,	889 cf Custom S	tage Data (Conic)	Listed below		
Eleva	ation feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)		
52	6.00	2,239	0	0	2,239		
52	8.00	3,156	5,369	5,369	3,227		
53	0.00	4,207	7,338	12,707	4,362		
53	0.50	4,491	2,174	14,881	4,669		
53	2.00	7,681	9,023	23,903	7,885		
53	4.00	10,686	18,285	42,188	10,966		
53	6.00	14,093	24,701	66,889	14,463		
#	Routing	Invert	Outlet Devices				
1 2	Primary Primary	532.00' 533.60'	<b>3.0" Vert. Orifice/Grate</b> C= 0.600 <b>14.0' long x 1.5' high Sharp-Crested Rectangular Weir</b> 2 End Contraction(s)				

Primary OutFlow Max=78.12 cfs @ 12.09 hrs HW=534.95' TW=502.35' (Dynamic Tailwater) -1=Orifice/Grate (Orifice Controls 0.40 cfs @ 8.1 fps) -2=Sharp-Crested Rectangular Weir (Weir Controls 77.72 cfs @ 4.2 fps)

# Pond p16-1:

Field Note # 49 Large pond with man-made island. Geometry to be verified by survey. In particular, we are making big guesses about the outlets. Also need to find out about valves...

Inflow Area = 176.893 ac, Inflow Depth = 3.36" for 100-yr event	
Inflow = 231.69 cfs @ 12.33 hrs, Volume= 49.496 af	
Outflow = 89.45 cfs @ 14.04 hrs, Volume= 37.676 af, Atten=	= 61%, Lag= 102.9 min
Primary = 89.45 cfs @ 14.04 hrs, Volume= 37.676 af	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 507.00' Surf.Area= 199,799 sf Storage= 878,320 cf Peak Elev= 511.38' @ 14.04 hrs Surf.Area= 310,348 sf Storage= 1,879,457 cf (1,001,137 cf above start) Flood Elev= 510.50' Surf.Area= 271,550 sf Storage= 1,623,217 cf (744,897 cf above start) Plug-Flow detention time= 856.5 min calculated for 17.512 af (35% of inflow) Center-of-Mass det. time= 324.5 min (1,220.2 - 895.8)

#	Invert	Avail.St	torage Sto	rage Des	cription		
1	500.00'	2,062,	087 cf Cu	stom Stag	ge Data (Conic)	Listed below	
Flouration	~	Curf Aree	Inc	Ctore	Cum Store	Mat Area	
Elevatio		Surf.Area		.Store	Cum.Store	Wet.Area	
(feet	t)	(sq-ft)	(cubic	c-feet)	(cubic-feet)	(sq-ft)	
500.0	0	0		0	0	0	
503.0	0	140,344	14	0,344	140,344	140,358	
509.20	0	232,500	1,14	3,862	1,284,206	232,994	
510.0	0	249,400	19	2,720	1,476,927	249,951	
512.0	0	338,000	58	5,160	2,062,087	338,634	
# Ro	outing	Invert	Outlet Devi	ices			
1 Pri	mary	509.00'	18.0" x 11	0.0' lona	Culvert CMP, p	rojecting, no hea	dwall, Ke= 0.900
	,				0' S= 0.0300 '/'	, .	
2 Pri	mary	500.00'	8.0" x 100	.0' long a	ssumed equalization	ation pipe w/ valv	ve X 0.00
				U U	headwall, Ke= 0		
				<b>U</b> .	0' S= 0.0000 '/'		0.900
3 Pri	mary	510.50'	175.0 dea 3	Sharp-Cro	ested Vee/Trap V	Veir X 2.00 C= 2	2.46
			5	•			

**Primary OutFlow** Max=89.45 cfs @ 14.04 hrs HW=511.38' TW=506.43' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 8.57 cfs @ 4.8 fps)

-2=assumed equalization pipe w/ valve (Controls 0.00 cfs)

-3=Sharp-Crested Vee/Trap Weir (Weir Controls 80.89 cfs @ 2.3 fps)

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# Pond p17-1:

Field Note #45 Golf pond Geometry to be confirmed by surveyed

Inflow Outflow	= =		s, Volume= s, Volume=	r 100-yr event 29.921 af 29.921 af, Atten= 0%, Lag= 0.4 29.921 af	min			
Deuting hu Dur Starlind method Time Spen 0.00.40.00 hrs. dt. 0.04 hrs.								

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 523.80' Surf.Area= 7,290 sf Storage= 9,234 cf Peak Elev= 525.63' @ 13.20 hrs Surf.Area= 11,495 sf Storage= 26,571 cf (17,337 cf above start) Flood Elev= 524.30' Surf.Area= 8,074 sf Storage= 13,623 cf (4,389 cf above start) Plug-Flow detention time= 14.4 min calculated for 29.702 af (99% of inflow) Center-of-Mass det. time= 8.0 min (914.1 - 906.1)

#	Invert	Avail.St	torage	ge Storage Description				
1	520.00'	30,	224 cf	Custom Sta	age Data (Conic) L	isted below		
<u> </u>	feet)	Surf.Area (sq-ft)	(c	Inc.Store ubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)		
-	0.00	0		0	0	0		
52	3.80	7,290		9,234	9,234	7,313		
52	4.00	7,300		1,459	10,693	7,374		
52	6.00	12,460		19,531	30,224	12,581		
#	Routing	Invert	Outlet [	Devices				
1	Primary	523.80'		•	adth Broad-Creste	0	Weir	
2 3	Primary Primary	524.30' 525.20'	Head (feet) 0.20 0.40 0.60 0.80 1.00 Coef. (English) 2.80 2.92 3.08 3.30 3.32 143.0 deg Sharp-Crested Vee/Trap Weir C= 2.47 178.0 deg x 60.0' long Sharp-Crested Vee/Trap Weir C= 2.46					

Primary OutFlow Max=100.92 cfs @ 13.20 hrs HW=525.63' TW=516.21' (Dynamic Tailwater) -1=Broad-Crested Rectangular Weir (Weir Controls 18.02 cfs @ 4.5 fps)

-2=Sharp-Crested Vee/Trap Weir (Weir Controls 14.94 cfs @ 2.8 fps)

-3=Sharp-Crested Vee/Trap Weir (Weir Controls 67.95 cfs @ 1.9 fps)

# Pond p18-1:

Field Note #46 Golf pond Geometry to be confirmed by surveyed

Inflow Area	a =	131.862 ac, Inflow Depth = 3.23	" for 100-yr event
Inflow	=	132.62 cfs @ 12.43 hrs, Volume	= 35.444 af
Outflow	=	131.91 cfs @ 12.47 hrs, Volume	= 35.439 af, Atten= 1%, Lag= 2.8 min
Primary	=	131.91 cfs @ 12.47 hrs, Volume	= 35.439 af

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Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 513.90' Surf.Area= 20,680 sf Storage= 26,884 cf Peak Elev= 516.32' @ 12.47 hrs Surf.Area= 30,579 sf Storage= 88,894 cf (62,010 cf above start) Flood Elev= 514.81' Surf.Area= 23,768 sf Storage= 48,709 cf (21,825 cf above start) Plug-Flow detention time= 38.1 min calculated for 34.822 af (98% of inflow) Center-of-Mass det. time= 22.7 min (925.5 - 902.8)

#	Invert	Avail.Storag	ge Storage Des	scription				
1	510.00'	148,288	148,288 cf Custom Stage Data (Conic) Listed below					
Elevat (fe	ion et)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)			
510		0	0	0	0			
513		20,680	26,884	26,884	20,704			
514		20,690	2,068	28,952	20,756			
516		28,290	48,782	77,735	28,436			
518	.00	42,760	70,554	148,288	42,967			

#	Routing	Invert	Outlet Devices
1	Primary	513.90'	2.0' long x 0.5' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00
			Coef. (English) 2.80 2.92 3.08 3.30 3.32
2	Primary	514.81'	143.0 deg Sharp-Crested Vee/Trap Weir C= 2.47
3	Primary	515.32'	175.0 deg x 10.0' long Sharp-Crested Vee/Trap Weir C= 2.46

Primary OutFlow Max=131.90 cfs @ 12.47 hrs HW=516.32' TW=509.42' (Dynamic Tailwater) -1=Broad-Crested Rectangular Weir (Weir Controls 24.94 cfs @ 5.2 fps) -2=Sharp-Crested Vee/Trap Weir (Weir Controls 20.56 cfs @ 3.0 fps)

-3=Sharp-Crested Vee/Trap Weir (Weir Controls 86.40 cfs @ 2.6 fps)

# Pond p19-0:

Wetland Geometry to be confirmed by survey Based off aerial topo, and assumed topo contour

Pond Unchanged from existing to proposed conditions

Inflow Area = Inflow = Outflow = Primary = Secondary =	22.44 cfs @ 15.21 cfs @ 15.21 cfs @	nflow Depth = 2.60 12.61 hrs, Volume 12.94 hrs, Volume 12.94 hrs, Volume 0.00 hrs, Volume	= 3.366 af, Atte = 3.366 af	en= 32%, Lag= 19.9 min			
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 972.00' Surf.Area= 86,000 sf Storage= 57,333 cf Peak Elev= 972.29' @ 12.94 hrs Surf.Area= 93,885 sf Storage= 89,433 cf (32,099 cf above start) Plug-Flow detention time= 278.9 min calculated for 2.049 af (61% of inflow) Center-of-Mass det. time= 57.4 min (942.9 - 885.4)							

# Proposed Conditions 10454-01

Prepared by Th	Page 297				
HydroCAD® 7.00	4/10/2006 3:16:23 PM				
# Invert	Avail.Stora	ge Storage Des	cription		
1 970.00'	282,329	cf Custom Sta	<b>ge Data (Conic)</b> Li	sted below	
Elevation	Surf.Area	Inc.Store	Cum.Store	Wet.Area	
(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)	(sq-ft)	
970.00	0	0	0	0	
972.00	86,000	57,333	57,333	86,006	
974.00	141,270	224,996	282,329	141,327	

_	#	Routing	Invert	Outlet Devices
	1	Secondary	973.60'	178.0 deg x 51.0' long Sharp-Crested Vee/Trap Weir C= 2.46
	2	Primary	972.00'	35.0' long x 0.5' breadth Broad-Crested Rectangular Weir
		-		Head (feet) 0.20 0.40 0.60 0.80 1.00
				Coef. (English) 2.80 2.92 3.08 3.30 3.32

Primary OutFlow Max=15.21 cfs @ 12.94 hrs HW=972.29' TW=970.21' (Dynamic Tailwater) 2=Broad-Crested Rectangular Weir (Weir Controls 15.21 cfs @ 1.5 fps)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=972.00' TW=973.60' (Dynamic Tailwater) -1=Sharp-Crested Vee/Trap Weir (Controls 0.00 cfs)

#### Pond p20-1:

Field Note #50 Spring Fed Pond Geometry to be confirmed by surveyed

Inflow Area =	207.817 ac, Inflow Depth = 2.73"	for 100-yr event
Inflow =	99.91 cfs @ 14.02 hrs, Volume=	47.279 af
Outflow =	98.56 cfs @ 14.14 hrs, Volume=	46.326 af, Atten= 1%, Lag= 7.3 min
Primary =	98.56 cfs @ 14.14 hrs, Volume=	46.326 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 505.10' Surf.Area= 89,370 sf Storage= 138,524 cf Peak Elev= 506.43' @ 14.15 hrs Surf.Area= 91,508 sf Storage= 259,519 cf (120,996 cf above start) Plug-Flow detention time= 196.3 min calculated for 43.137 af (91% of inflow) Center-of-Mass det. time= 48.0 min (1,222.4 - 1,174.4)

#	Invert	Avail.Storag	e Storage Des	Storage Description			
1	502.00' 615,682 cf		cf Custom Sta	Custom Stage Data (Prismatic) Listed below			
Elevatio		Surf.Area	Inc.Store	Cum.Store			
(fee	et)	(sq-ft)	(cubic-feet)	(cubic-feet)			
502.0	00	0	0	0			
505.1	10	89,370	138,524	138,524			
506.0	00	89,380	80,437	218,961			
508.00		99,280	188,660	407,621			
510.00		108,781	208,061	615,682			

Type III 24-hr 100-yr Rainfall=7.00"

### Proposed Conditions 10454-01

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Type III 24-hr 100-yr Rainfall=7.00" Page 298 HydroCAD® 7.00 s/n 000927 © 1986-2003 Applied Microcomputer Systems 4/10/2006 3:16:23 PM

#	Routing	Invert	Outlet Devices
1	Primary	505.10'	3.0' long x 1.5' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50
			3.00
			Coef. (English) 2.62 2.64 2.64 2.68 2.75 2.86 2.92 3.07 3.07 3.03 3.28
			3.32
2	Primary	506.20'	6.5' long x 1.5' breadth Broad-Crested Rectangular Weir
	-		Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50
			3.00
			Coef. (English) 2.62 2.64 2.64 2.68 2.75 2.86 2.92 3.07 3.07 3.03 3.28
			3.32
~			
3	Primary	506.00'	176.0 deg x 97.0' long Sharp-Crested Vee/Trap Weir C= 2.46

Primary OutFlow Max=98.54 cfs @ 14.14 hrs HW=506.43' TW=506.18' (Dynamic Tailwater)

-1=Broad-Crested Rectangular Weir (Weir Controls 8.07 cfs @ 2.0 fps)

-2=Broad-Crested Rectangular Weir (Weir Controls 1.88 cfs @ 1.3 fps)

-3=Sharp-Crested Vee/Trap Weir (Weir Controls 88.59 cfs @ 1.9 fps)

# Pond p20-2:

Inflow Area =		13.511 ac, Inflow Depth = 5.12"	for 100-yr event
Inflow	=	69.74 cfs @ 12.09 hrs, Volume=	5.769 af
Outflow	=	14.79 cfs @ 12.54 hrs, Volume=	4.093 af, Atten= 79%, Lag= 27.0 min
Primary	=	14.79 cfs @ 12.54 hrs, Volume=	4.093 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 552.00' Surf.Area= 10,535 sf Storage= 35,913 cf Peak Elev= 559.00' @ 12.54 hrs Surf.Area= 25,656 sf Storage= 174,066 cf (138,153 cf above start) Flood Elev= 559.00' Surf.Area= 25,653 sf Storage= 174,016 cf (138,102 cf above start) Plug-Flow detention time= 689.6 min calculated for 3.268 af (57% of inflow) Center-of-Mass det. time= 445.6 min (1,226.0 - 780.5)

#	Invert	Avail.St	orage Storage D	escription		
1	546.00'	199,	647 cf Custom S	Stage Data (Conic)	Listed below	
	ration (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
54	46.00 48.00	3,714 4,960	0 8,644	0 8,644	3,714 5,044	
	50.00 50.50	6,308 6,661	11,241 3,242	19,885 23,127	6,493 6,874	
	52.00 52.50	10,535 15,037	12,786 6,360	35,913 42,273	10,779 15,285	
	54.00 56.00	17,268 20,441	24,209 37,664	66,483 104,147	17,616 20,935	
55	58.00 50.00	23,840 27,465	44,237 51,262	148,384 199,647	24,494 28,292	
#	Routing	Invert	Outlet Devices	100,041	20,202	
1 2	Primary Primary	552.00' 558.20'	3.0" Vert. Orifice/ 6.1' long x 6.2' hig		Rectangular Weir	2 End Contraction(s)

Primary OutFlow Max=14.79 cfs @ 12.54 hrs HW=559.00' TW=506.20' (Dynamic Tailwater) 1=Orifice/Grate (Orifice Controls 0.62 cfs @ 12.6 fps) 2=Sharp-Crested Rectangular Weir (Weir Controls 14.17 cfs @ 3.0 fps)

# Pond p21-1:

Inflow Area =	459.188 ac, Inflow Depth = 3.29"	for 100-yr event
Inflow =	578.35 cfs @ 12.27 hrs, Volume=	125.867 af
Outflow =	37.64 cfs @ 20.82 hrs, Volume=	99.293 af, Atten= 93%, Lag= 513.2 min
Primary =	37.64 cfs @ 20.82 hrs, Volume=	99.293 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 486.65' @ 20.82 hrs Surf.Area= 1,348,222 sf Storage= 3,374,375 cf Plug-Flow detention time= 898.0 min calculated for 99.292 af (79% of inflow) Center-of-Mass det. time= 727.8 min (1,734.1 - 1,006.3)

# Invert Avail.Storage Storage Description									
1	480.40'		5,244,885 cf Custom Stage Data (Conic) Listed below						
				•	-				
Elevat	tion	Surf.Area	Inc.Sto	ore Cur	n.Store	Wet.Area			
(fe	eet)	(sq-ft)	(cubic-fe	et) (cub	oic-feet)	(sq-ft)			
480	).40	0		0	0	0			
482	2.00	202,230	107,8	56 1	07,856	202,234			
484	I.00	485,198	667,1	14 7	74,970	485,231			
486	6.00	1,275,481	1,698,2	37 2,4	73,208	1,275,541			
488	8.00	1,499,208	2,771,6	78 5,2	244,885	1,499,423			
щ І	Douting	lun vout							
#	Routing	Invert	Outlet Devices						
1 1	Primary	480.40'	30.0" x 70.0' l	ong Culvert	CMP, pr	rojecting, no head	lwall, Ke= 0.900		

1 Primary 480.40' **30.0" x 70.0' long Culvert** CMP, projecting, no headwall, Ke= 0.900 Outlet Invert= 480.40' S= 0.0000 '/' n= 0.024 Cc= 0.900

Primary OutFlow Max=37.64 cfs @ 20.82 hrs HW=486.65' TW=0.00' (Dynamic Tailwater) -1=Culvert (Barrel Controls 37.64 cfs @ 7.7 fps)

#### Pond p21-4:

Inflow Area =		5.152 ac, Inflow Depth = 4.49"	for 100-yr event
Inflow	=	18.73 cfs @ 12.03 hrs, Volume=	1.929 af
Outflow	=	14.59 cfs @ 12.23 hrs, Volume=	1.917 af, Atten= 22%, Lag= 12.0 min
Primary	=	14.59 cfs @ 12.23 hrs, Volume=	1.917 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 496.00' Surf.Area= 5,112 sf Storage= 14,306 cf Peak Elev= 499.00' @ 12.23 hrs Surf.Area= 8,847 sf Storage= 35,620 cf (21,314 cf above start) Flood Elev= 499.00' Surf.Area= 8,847 sf Storage= 35,622 cf (21,317 cf above start) Plug-Flow detention time= 354.4 min calculated for 1.589 af (82% of inflow) Center-of-Mass det. time= 211.5 min (1,001.0 - 789.5)

# Proposed Conditions 10454-01

Type III 2	24-hr 100-yr Rainfall=7.00"
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#	Invert	Avail.S	torage	Storage De	scription				
1	490.00'	44,	433 cf	Custom Sta	Custom Stage Data (Conic) Listed below				
Eleva (f	ation eet)	Surf.Area (sq-ft)		Inc.Store cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)			
49	0.00	1,146		0	0	1,146			
492	2.00	1,784		2,907	2,907	1,839			
494	4.00	2,530		4,292	7,199	2,654			
494	4.50	2,733		1,315	8,514	2,876			
49	6.00	5,112		5,791	14,306	5,278			
49	8.00	7,468		12,506	26,812	7,699			
50	0.00	10,226		17,622	44,433	10,536			
	Routing	Invert		Devices					
1	Primary	496.00'			rate C= 0.600				
2	Primary	498.10'	5.0' loi	ng x 2.0' higł	n Sharp-Crested F	Rectangular Weir	2 End Contraction(s)		

Primary OutFlow Max=14.59 cfs @ 12.23 hrs HW=499.00' TW=483.78' (Dynamic Tailwater) -1=Orifice/Grate (Orifice Controls 0.40 cfs @ 8.2 fps)

-2=Sharp-Crested Rectangular Weir (Weir Controls 14.19 cfs @ 3.3 fps)

# Pond p21-5:

Inflow Area =	2.398 ac, 1	nflow Depth	= 4.15"	for 1	100-yr event		
Inflow =	9.09 cfs @	12.19 hrs,	Volume=		0.829 af		
Primary =	9.09 cfs @	12.19 hrs,	Volume=		0.829 af,	Atten= 0%,	Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

### Pond p21-6:

Inflow Area	a =	6.182 ac, Inflow Depth = 4.55"	for 100-yr event
Inflow	=	21.31 cfs @ 12.23 hrs, Volume=	= 2.345 af
Outflow	=	16.61 cfs @ 12.38 hrs, Volume=	= 2.334 af, Atten= 22%, Lag= 8.8 min
Primary	=	16.61 cfs @ 12.38 hrs, Volume=	= 2.334 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 492.00' Surf.Area= 3,323 sf Storage= 4,847 cf Peak Elev= 494.98' @ 12.38 hrs Surf.Area= 13,794 sf Storage= 34,214 cf (29,367 cf above start) Flood Elev= 495.00' Surf.Area= 13,824 sf Storage= 34,456 cf (29,609 cf above start) Plug-Flow detention time= 267.7 min calculated for 2.223 af (95% of inflow) Center-of-Mass det. time= 218.9 min (1,031.2 - 812.2)

#	Invert	Avail.Storage	Storage Description
1	488.00'	48,245 cf	Custom Stage Data (Conic) Listed below

# Proposed Conditions\_10454-01

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Wet.Area (sq-ft)	Cum.Store (cubic-feet)	Inc.Store (cubic-feet)	Surf.Area (sq-ft)	Elevation (feet)
296	0	0	296	488.00
946	1,162	1,162	924	490.00
1,141	1,670	508	1,110	490.50
3,367	4,847	3,177	3,323	492.00
6,212	7,182	2,336	6,166	492.50
12,214	20,666	13,484	12,147	494.00
15,669	48,245	27,579	15,500	496.00

Routing Invert Outlet Devices
 Primary 492.00' 3.0" Vert. Orifice/Grate

Primary 492.00' **3.0" Vert. Orifice/Grate** C= 0.600

2 Primary 494.00' 5.0' long x 2.0' high Sharp-Crested Rectangular Weir 2 End Contraction(s)

**Primary OutFlow** Max=16.61 cfs @ 12.38 hrs HW=494.98' TW=484.26' (Dynamic Tailwater) **1=Orifice/Grate** (Orifice Controls 0.40 cfs @ 8.1 fps)

**2=Sharp-Crested Rectangular Weir** (Weir Controls 16.21 cfs @ 3.4 fps)

# Pond p21-7:

Inflow Area	a =	8.355 ac, Inflow Depth = $4.79$ "	for 100-yr event
Inflow	=	36.88 cfs @ 12.04 hrs, Volume=	3.338 af
Outflow	=	17.82 cfs @ 12.32 hrs, Volume=	3.314 af, Atten= 52%, Lag= 16.8 min
Primary	=	17.82 cfs @ 12.32 hrs, Volume=	3.314 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 492.00' Surf.Area= 3,941 sf Storage= 8,984 cf Peak Elev= 498.92' @ 12.32 hrs Surf.Area= 13,242 sf Storage= 66,293 cf (57,309 cf above start) Flood Elev= 499.00' Surf.Area= 13,379 sf Storage= 67,369 cf (58,385 cf above start) Plug-Flow detention time= 369.8 min calculated for 3.108 af (93% of inflow) Center-of-Mass det. time= 296.3 min (1,072.1 - 775.8)

#	Invert	Avail.St	orage	Storage Des	scription		
1	486.00'	80,7	712 cf	<b>Custom Sta</b>	ige Data (Conic)	Listed below	
Eleva (f	ation feet)	Surf.Area (sq-ft)	(c	Inc.Store ubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
48	6.00	478		0	0	478	
48	8.00	964		1,414	1,414	999	
49	0.00	1,601		2,538	3,952	1,684	
49	0.50	1,782		845	4,797	1,879	
492	2.00	3,941		4,187	8,984	4,056	
494	4.00	6,120		9,981	18,965	6,292	
49	6.00	8,702		14,746	33,712	8,944	
49	8.00	11,686		20,315	54,027	12,012	
50	0.00	15,071		26,685	80,712	15,495	
	Routing	Invert	Outlet E				
1 2 3	Primary Primary Primary	496.05'	6.0" Ve	rt. Orifice/G	rate C= 0.600 rate X 2.00 C= Sharp-Crested		2 End Contraction(s)

Primary OutFlow Max=17.82 cfs @ 12.32 hrs HW=498.92' TW=484.13' (Dynamic Tailwater) -1=Orifice/Grate (Orifice Controls 0.62 cfs @ 12.6 fps) -2=Orifice/Grate (Orifice Controls 3.06 cfs @ 7.8 fps) -3=Sharp-Crested Rectangular Weir (Weir Controls 14.14 cfs @ 3.2 fps)

# Pond p22-1:

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Field Note #54 Golf Pond Geometry to be confirmed by survey

Pond unchanged from existing to proposed conditions

Inflow Area =	78.382 ac, 1	Inflow Depth = 3.35"	for 100-yr event
Inflow =	163.25 cfs @	12.29 hrs, Volume=	21.895 af
Outflow =	161.62 cfs @	12.32 hrs, Volume=	21.596 af, Atten= 1%, Lag= 1.8 min
Primary =	161.62 cfs @	12.32 hrs, Volume=	21.596 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 498.10' Surf.Area= 6,520 sf Storage= 10,106 cf Peak Elev= 502.47' @ 12.32 hrs Surf.Area= 12,237 sf Storage= 50,332 cf (40,226 cf above start) Plug-Flow detention time= 31.5 min calculated for 21.359 af (98% of inflow) Center-of-Mass det. time= 14.3 min ( 876.7 - 862.4 )

#	Invert	Avail.St	orage Sto	orage Des	cription	
1	495.00'	143,	770 cf <b>Cu</b>	stom Stag	ge Data (Prisma	atic) Listed below
		<b>-</b>		-		
Elevati		Surf.Area	-	.Store	Cum.Store	
(fe	eet)	(sq-ft)	(cubio	c-feet)	(cubic-feet)	
495.	.00	0		0	0	
498.	.10	6,520	1	0,106	10,106	
500.	.00	8,390	1	4,164	24,270	
502.	.00	11,530	1	9,920	44,190	
504.	.00	14,530	2	26,060	70,250	
506.	.00	18,340	3	32,870	103,120	
508.	.00	22,310	4	10,650	143,770	
#F	Routing	Invert	Outlet Dev	ices		
1 F	Primary	499.75'	18.0" x 21	.0' long C	ulvert CMP, p	rojecting, no headwall, Ke= 0.900
	-		<b>Outlet Inve</b>	ert= 499.75	5' S= 0.0000 '/'	n= 0.024 Cc= 0.900
2 F	Primary	500.50'	1.0' long >	( 15.0' bre	adth Broad-Cro	ested Rectangular Weir
	-		Head (feet)	) 0.20 0.4	40 0.60 0.80 1	.00 1.20 1.40 1.60
			Coef. (Eng	, lish) 2.68	2.70 2.70 2.6	4 2.63 2.64 2.64 2.63
3 F	Primary	500.50'	20.0' long	x 13.5' br	eadth Broad-C	rested Rectangular Weir
	-		Head (feet)	) 0.20 0.4	40 0.60 0.80 1	.00 1.20 1.40 1.60
			Coef. (Eng	lish) 2.62	2.66 2.70 2.6	6 2.65 2.66 2.65 2.63

Proposed Conditions_10454-01	Type III 24-h
Prepared by The Chazen Companies	
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Primary OutFlow Max=161.61 cfs @ 12.32 hrs HW=502.47' TW=484.13' (Dynamic Tailwater) -1=Culvert (Barrel Controls 8.75 cfs @ 5.0 fps)

-2=Broad-Crested Rectangular Weir (Weir Controls 7.28 cfs @ 3.7 fps)

-3=Broad-Crested Rectangular Weir (Weir Controls 145.58 cfs @ 3.7 fps)

# Pond p23-1:

Inflow Area	a =	29.123 ac, In	flow Depth :	= 3.83"	for '	100-yr event		
Inflow	=	65.76 cfs @	12.53 hrs, \	Volume=		9.293 af		
Outflow	=	65.71 cfs @	12.54 hrs, \	Volume=		8.578 af,	Atten= 0%,	Lag= 0.5 min
Primary	=	65.71 cfs @	12.54 hrs, \	Volume=		8.578 af		

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 507.93' @ 12.54 hrs Surf.Area= 14,476 sf Storage= 33,912 cf Plug-Flow detention time= 56.7 min calculated for 8.576 af (92% of inflow) Center-of-Mass det. time= 17.2 min (873.8 - 856.6)

#	Invert	Avail.Stor	rage Storage Des	scription	
1	503.50'	68,91	5 cf Custom Sta	i <b>ge Data (Conic)</b> Li	sted below
Elevat (fe	tion eet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
503	5.50	0	0	0	0
504	.00	2,390	398	398	2,390
506	6.00	9,090	10,761	11,159	9,110
508	.00	14,660	23,529	34,688	14,732
510	.00	19,690	34,227	68,915	19,847
#_ F	Routing	Invert C	Outlet Devices		

1 Primary 507.70' **178.0 deg x 178.0' long Sharp-Crested Vee/Trap Weir** C= 2.46

Primary OutFlow Max=65.69 cfs @ 12.54 hrs HW=507.93' TW=507.26' (Dynamic Tailwater) ←1=Sharp-Crested Vee/Trap Weir (Weir Controls 65.69 cfs @ 1.5 fps)

#### Pond p23-2:

Inflow Are	ea =	16.094 ac, Inflow Depth = 5.46" for 100-yr eve	nt
Inflow	=	101.80 cfs @ 12.06 hrs, Volume= 7.326 af	
Outflow	=	69.09 cfs @ 12.13 hrs, Volume= 6.478 af	, Atten= 32%, Lag= 4.4 min
Primary	=	69.09 cfs @ 12.13 hrs, Volume= 6.478 af	-

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Starting Elev= 508.00' Surf.Area= 7,318 sf Storage= 15,927 cf Peak Elev= 514.92' @ 12.13 hrs Surf.Area= 24,621 sf Storage= 139,976 cf (124,049 cf above start) Flood Elev= 515.00' Surf.Area= 24,788 sf Storage= 141,986 cf (126,059 cf above start) Plug-Flow detention time= 356.3 min calculated for 6.113 af (83% of inflow) Center-of-Mass det. time= 261.6 min (1,033.7 - 772.1)

# Proposed Conditions\_10454-01

Type III 24-hr 100-yr Rainfall=7.00" Page 304 4/10/2006 3:16:23 PM าร

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t Avail.Storage	<ul> <li>Storage Des</li> </ul>	cription				
			sted below			
Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)			
826	0	0	826			
1,667	2,444	2,444	1,702			
		-				
		-				
20,010	10,010	100,110	21,120			
Invert Outle	et Devices					
512.55' <b>12.0</b> 514.00' <b>20.0</b>	" Vert. Orifice/G ' long x 6.0' hig	irate X 2.00 C= 0. h Sharp-Crested R				
rate (Orifice Continents) rate (Orifice Continents)	ols 0.62 cfs @ <sup>.</sup> ols 10.34 cfs @	12.5 fps) 6.6 fps)				
	Pond zDP	1: Design Point	1			
ons to be confirme	ed by survey.					
51.27 cfs @ 1 51.27 cfs @ 1	2.46 hrs, Volun 2.46 hrs, Volun	ne= 8.392 a ne= 8.392 a	af af, Atten= 0%, Lag= 0.0 min			
Primary = $51.27 \text{ cfs} @ 12.46 \text{ hrs}$ , Volume= $8.392 \text{ af}$ Routing by Dyn-Stor-Ind method, Time Span= $0.00-48.00 \text{ hrs}$ , dt= $0.01 \text{ hrs}$ Peak Elev= $723.08' @ 12.46 \text{ hrs}$ Surf.Area= $225 \text{ sf}$ Storage= $275 \text{ cf}$ Flood Elev= $727.00'$ Surf.Area= $1,105 \text{ sf}$ Storage= $2,619 \text{ cf}$ Plug-Flow detention time= $0.1 \text{ min}$ calculated for $8.390 \text{ af}$ ( $100\%$ of inflow) Center-of-Mass det. time= $0.1 \text{ min} (900.4 - 900.3)$						
	Surf.Area (sq-ft)         826         1,667         2,788         3,112         7,318         12,618         15,208         18,859         22,736         26,840         Invert Outle         508.00'       3.0"         512.55'       12.0         514.00'       20.0         2 En         ow Max=69.06 cfs         rate (Orifice Contrate (Orifice (Orific	Surf.Area (sq-ft)         Inc.Store (cubic-feet)           826         0           1,667         2,444           2,788         4,407           3,112         1,474           7,318         7,601           12,618         4,924           15,208         20,839           18,859         34,002           22,736         41,535           26,840         49,519           Invert         Outlet Devices           508.00'         3.0" Vert. Orifice/Gr           512.55'         12.0" Vert. Orifice/Gr           514.00'         20.0' long x 6.0' higl           2 End Contraction(s)           Ow Max=69.06 cfs @ 12.13 hrs H           rate (Orifice Controls 0.62 cfs @ 12.46 hrs           rate (Orifice Controls 10.34 cfs @           ested Rectangular Weir (Weir Controls 0.62 cfs @ 12.46 hrs, Volum           51.27 cfs @ 12.46 hrs, Volum	Surf.Area (sq-ft)         Inc.Store (cubic-feet)         Cum.Store (cubic-feet)           826         0         0           1,667         2,444         2,444           2,788         4,407         6,852           3,112         1,474         8,326           7,318         7,601         15,927           12,618         4,924         20,851           15,208         20,839         41,690           18,859         34,002         75,692           22,736         41,535         117,227           26,840         49,519         166,746           Invert         Outlet Devices         508.00'           508.00'         3.0" Vert. Orifice/Grate         C= 0.600           512.55'         12.0" Vert. Orifice/Grate X 2.00         C= 0.           514.00'         20.0' long x 6.0' high Sharp-Crested R           2 End Contraction(s)         20w         Max=69.06 cfs @ 12.13 hrs           w Max=69.06 cfs @ 12.13 hrs         HW=514.92'         TW=4.           rate (Orifice Controls 0.62 cfs @ 12.5 fps)         125 fps)           rate (Orifice Controls 10.34 cfs @ 6.6 fps)         ested Rectangular Weir         (Weir Controls 58.11 cfs @           ions to be confirmed by survey.         26.658 ac, Inflow	Surf.Area         Inc.Store         Cum.Store         Wet.Area           (sq-ft)         (cubic-feet)         (sq-ft)           826         0         0         826           1.667         2.444         2.444         1.702           2.788         4.407         6.852         2.872           3.112         1.474         8.326         3.210           7,318         7,601         15.927         7.432           12,618         4.924         20.851         12.735           15,208         20.839         41.690         15.400           18,859         34,002         75,692         19,166           22,736         41.535         117.227         23,175           26,840         49,519         166,746         27,428           Invert Outlet Devices           508.00         3.0" Vert. Orifice/Grate X 2.00 C= 0.600         512.55         12.0" Vert. Orifice/Grate X 2.00 C= 0.600           514.00'         20.0' long x 6.0' high Sharp-Crested Rectangular Weir         2 End Contraction(s)           w Max=69.06 cfs @ 12.13 hrs HW=514.92' TW=483.23' (Dynamic Tailwater)           rate (Orifice Controls 10.34 cfs @ 6.6 fps)            10.34 cfs @ 6.6 fps)         <		

#	Invert	Avail.Storage	Storage Description
1	720.10'	3,706 cf	Custom Stage Data (Conic) Listed below

# Proposed Conditions 10454-01

Prepared by The Chazen	Companies
HydroCAD® 7.00 s/n 000927	© 1986-2003 Applied Microcomputer Syste

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft <u>)</u>
720.10	0	0	0	0
722.00	90	57	57	96
724.00	340	403	460	364
726.00	760	1,072	1,533	815
728.00	1,450	2,173	3,706	1,543

#	Routing	Invert	Outlet Devices
1	Primary	720.10'	<b>42.0" x 120.0' long Culvert</b> CMP, square edge headwall, Ke= 0.500
	-		Outlet Invert= 700.00' S= 0.1675 '/' n= 0.024 Cc= 0.900
2	Primary	727.00'	155.0 deg Sharp-Crested Vee/Trap Weir C= 2.47

Primary OutFlow Max=51.26 cfs @ 12.46 hrs HW=723.08' TW=686.81' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 51.26 cfs @ 5.9 fps)

-2=Sharp-Crested Vee/Trap Weir (Controls 0.00 cfs)

# Pond zDP2: Design Point 2

Field Note #15 Culvert dimensions to be confirmed by survey Overflow to ditch is currently discarded... We may have to model that area...

Inflow Area =	93.367 ac, Inflow Depth = 3.11"	for 100-yr event
Inflow =	127.33 cfs @ 12.81 hrs, Volume=	24.206 af
Outflow =	127.31 cfs @ 12.82 hrs, Volume=	24.206 af, Atten= 0%, Lag= 0.4 min
Discarded =	91.09 cfs @ 12.82 hrs, Volume=	7.727 af
Primary =	36.22 cfs @ 12.82 hrs, Volume=	16.479 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 626.33' @ 12.82 hrs Surf.Area= 1,613 sf Storage= 3,993 cf Flood Elev= 624.50' Surf.Area= 925 sf Storage= 1,728 cf Plug-Flow detention time= 0.7 min calculated for 24.206 af (100% of inflow) Center-of-Mass det. time= 0.5 min (891.1 - 890.5)

#	Invert	Avail.S	torage Sto	Storage Description					
1	619.60'	7,	280 cf Cu	istom Sta	ige Data (Conic)	Listed below			
	ation (feet)	Surf.Area (sq-ft)	-	c.Store c-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)			
61	19.60	0		0	0	0			
62	20.00	10		1	1	10			
62	22.00	260		214	215	269			
62	24.00	760		976	1,192	793			
62	26.00	1,420		2,146	3,338	1,492			
62	28.00	2,580		3,943	7,280	2,694			
#	Routing	Invert	Outlet Dev	rices					
1	Primary	619.60'		-		nd-section confo n= 0.012 Cc= (	rming to fill, Ke= 0.500 0.900		
2	Discarded	624.50'	166.0 deg	Sharp-Cr	ested Vee/Trap	<b>Weir</b> C= 2.46			

Discarded OutFlow Max=91.09 cfs @ 12.82 hrs HW=626.33' (Free Discharge) ←2=Sharp-Crested Vee/Trap Weir (Weir Controls 91.09 cfs @ 3.3 fps)

Primary OutFlow Max=36.22 cfs @ 12.82 hrs HW=626.33' TW=607.68' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 36.22 cfs @ 11.5 fps)

# Pond zDP3: Design Point 3

Inflow Are	a =	228.471 ac, Inflow Depth = 19.48"	for	100-yr event
Inflow	=	307.06 cfs @ 12.39 hrs, Volume=		370.914 af
Primary	=	307.06 cfs @ 12.39 hrs, Volume=		370.914 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

# Pond zDP4: Design Point 4

Inflow Area	a =	459.188 ac, Inflow Depth = 2.59"	for 100-yr event
Inflow	=	37.64 cfs @ 20.82 hrs, Volume=	99.293 af
Primary	=	37.64 cfs @ 20.82 hrs, Volume=	99.293 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

### Pond zDP5: Design Point 5

Inflow Area	1 =	28.325 ac, I	nflow Depth = 3.62	" for 100-yr event
Inflow	=	67.08 cfs @	12.45 hrs, Volume	= 8.541 af
Primary	=	67.08 cfs @	12.45 hrs, Volume	= 8.541 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

# Appendix L: Design Calculations

The Chazen Companies April 10, 2006

Subcatchment #			s01	s02	s03	s04	s05	s06	s06(OW)	s07
Total Area (sq.ft.)			500,284	4,256,334	660,980	496,709	650,587	392,339	18,652	296,676
Total Area (Ac.)			11.485	97.712	15.174	11.403	14.935	9.007	0.428	6.811
Composite CN			68	65	69	65	61	62	100	64
Tc (min.)			42.8	61.3	28.8	6.5	17.3	17.3	0.0	13.9
			Area	Area	Area	Area	Area	Area	Area	Area
Land Use	HSG	CN	(sq.ft.)	(sq.ft.)	(sq.ft.)	(sq.ft.)	(sq.ft.)	(sq.ft.)	(sq.ft.)	(sq.ft.)
Impervious	n/a	98	27,212	154,201	48,276	26,764	7,763	0	0	0
Open Water	n/a	100	0	0	0	0	0	0	18,652	0
Open Space	А	39	0	0	0	0	0	0	0	0
(good condition)	В	61	0	89,888	5,413	0	247,015	369,612	0	156,861
	С	74	0	163,946	0	0	0	0	0	32,368
	D	80	0	39,509	0	0	0	22,727	0	0
Brush	А	30	0	0	0	0	0	0	0	0
(good condition)	В	48	0	342,297	0	100,402	113,197	0	0	0
	С	65	69,152	354,706	356,613	158,627	270,880	0	0	107,448
	D	73	0	9,429	0	0	0	0	0	0
Woods	А	30	0	0	0	0	0	0	0	0
(good condition)	В	55	82,999	1,328,763	21,986	20,395	0	0	0	0
	С	70	320,920	1,566,601	228,693	190,521	11,731	0	0	0
	D	77	0	206,994	0	0	0	0	0	0

#### SHEET FLOW

SHEET FLOW								
Flow length (ft.)	150	150	20		100	100		100
Slope (ft./ft.)	0.03	0.07	0.03		0.09	0.12		0.11
Manning's n for sheet flow	0.6	0.6	0.011		0.24	0.24		0.24
2-year, 24-hour rainfall (in.)	3.4	3.4	3.4	3.4	3.4	3.4	3.4	3.4
Travel time (min.)	33.9	24.1	0.3	0.0	7.6	6.8	0.0	7.0

1480

0.12

2.5

28.5

420

0.29

2.5

5.2

2100

0.15

2.5

36.1

#### SHALLOW CONCENTRATED

Flow length (ft.)
Slope (ft./ft.)
Kv (fps)
Travel time (min.)

Flow length (ft.) Slope (ft./ft.) Kv (fps)

Travel time (min.)

#### SHALLOW CONCENTRATED

340					630		
0.09					0.10		
7.0					7.0		
2.7	0.0	0.0	0.0	0.0	4.7	0.0	0.0

1030

0.16

7.0

6.1

1470

0.13

7.0

9.7

490

0.04

7.0

5.8

0.0

1130

0.15

7.0

6.9

OPEN CHANNEL OR PIPE								
Flow length (ft.)	650	1700		210				
Slope (ft./ft.)	0.08	0.12		0.08				
Manning's n for channel flow	0.027	0.04		0.045				
Open Channel - Bottom width (ft.)	2	5		4				
Open Channel - Side slopes	3	2		5				
Open Channel - Bank full depth (ft.)	1	6		2				
Pipe - Diameter (in.)								
Travel time (min.)	1.0	1.0	0.0	0.3	0.0	0.0	0.0	0.0
Time of Concentration (min.)	42.8	61.3	28.8	6.5	17.3	17.3	0.0	13.9

Subcatchment #			s07(OW)	s08	s09	s10	s10(OW)	s11	s11(IC)	s12
Total Area (sq.ft.)			22,053	1,381,698	368,154	354,148	36,160	102,963	101,851	279,676
Total Area (Ac.)			0.506	31.719	8.452	8.130	0.830	2.364	2.338	6.420
Composite CN			100	58	65	67	100	69	98	55
Tc (min.)			0.0	21.8	16.4	27.9	0.0	20.5	2.8	42.0
			Area	Area	Area	Area	Area	Area	Area	Area
Land Use	HSG	CN	(sq.ft.)	(sq.ft.)	(sq.ft.)	(sq.ft.)	(sq.ft.)	(sq.ft.)	(sq.ft.)	(sq.ft.)
Impervious	n/a	98	0	25,133	0	4,456	0	0	101,851	13,714
Open Water	n/a	100	22,053	0	0	0	36,160	0	0	0
Open Space	Α	39	0	0	0	0	0	0	0	0
(good condition)	В	61	0	827,510	175,170	47,339	0	30,880	0	0
	С	74	0	0	63,353	0	0	36,519	0	0
	D	80	0	6,225	0	152,794	0	0	0	0
Brush	Α	30	0	0	0	0	0	0	0	0
(good condition)	В	48	0	390,480	0	0	0	0	0	108,715
	С	65	0	56,290	6,166	0	0	3,551	0	21,152
	D	73	0	0	0	0	0	0	0	0
Woods	Α	30	0	0	0	0	0	0	0	0
(good condition)	В	55	0	72,256	42,952	145,968	0	0	0	<mark>136,095</mark>
	С	70	0	0	80,513	0	0	32,014	0	0
	D	77	0	3,804	0	3,591	0	0	0	0

SHEET FLOW								
Flow length (ft.)		100	90	130		150	50	150
Slope (ft./ft.)		0.16	0.21	0.09		0.26	0.03	0.10
Manning's n for sheet flow		0.24	0.6	0.6		0.6	0.011	0.24
2-year, 24-hour rainfall (in.)	3.4	3.4	3.4	3.4	3.4	3.4	3.4	3.4
Travel time (min.)	0.0	6.0	10.3	19.5	0.0	14.3	0.6	10.1

210

0.36

2.5

2.3

280

0.17

7.0

1.6

0.0

SHA	LLOW CONCENTRATED	
Flow	length (ft.)	
Slop	e (ft./ft.)	

	•	•	,	
Κv	(fps	5)		
Tra	ave	l time	e (min.)	

#### SHALLOW CONCENTRATED

Flow length (ft.)		1550	560	600		360		562
Slope (ft./ft.)		0.07	0.13	0.05		0.10		0.01
Kv (fps)		7.0	7.0	7.0		7.0		2.5
Travel time (min.)	0.0	13.9	3.7	6.4	0.0	2.7	0.0	31.4

150

0.23

2.5

2.1

0.0

200

0.30

2.5

2.4

240

0.03

20.3

1.1

64

0.14

7.0

0.4

OPEN CHANNEL OR PIPE								
Flow length (ft.)		65				520	520	
Slope (ft./ft.)		0.020				0.01	0.01	
Manning's n for channel flow		0.06				0.012	0.012	
Open Channel - Bottom width (ft.)		8						
Open Channel - Side slopes		1						
Open Channel - Bank full depth (ft.)		4						
Pipe - Diameter (in.)						24	24	
Travel time (min.)	0.0	0.2	0.0	0.0	0.0	1.1	1.1	0.0
Time of Concentration (min.)	0.0	21.8	16.4	27.9	0.0	20.5	2.8	42.0

Subcatchment #			s13	s14	s14(IC)	s14(OW)	s15	s16	s16(IC)	s16(OW)
Total Area (sq.ft.)			15,256	786,944	103,664	22,563	46,503	2,961,820	114,515	233,105
Total Area (Ac.)			0.350	18.066	2.380	0.518	1.068	67.994	2.629	5.351
Composite CN			55	64	98	100	51	66	98	100
Tc (min.)			8.6	28.1	2.3	0.0	13.1	22.3	2.9	0.0
			Area	Area	Area	Area	Area	Area	Area	Area
Land Use	HSG	CN	(sq.ft.)	(sq.ft.)	(sq.ft.)	(sq.ft.)	(sq.ft.)	(sq.ft.)	(sq.ft.)	(sq.ft.)
Impervious	n/a	98	0	0	103,664	0	0	0	114,515	0
Open Water	n/a	100	0	0	0	22,563	0	0	0	233,105
Open Space	A	39	0	0	0	0	0	0	0	0
(good condition)	В	61	0	60,933	0	0	0	848,071	0	0
	С	74	0	0	0	0	0	414,618	0	0
	D	80	0	55,295	0	0	0	263,145	0	0
			-		-					
Brush	A	30	0	0	0	0	0	0	0	0
(good condition)	В	48	0	83,704	0	0	24,787	175,490	0	0
	С	65	0	0	0	0	0	994,221	0	0
	D	73	0	94,487	0	0	0	5,762	0	0
Woods	А	30	0	0	0	0	0	0	0	0
(good condition)	В	55	15,256	290,388	0	0	21,716	33,458	0	0
	С	70	0	0	0	0	0	94,539	0	0
	D	77	0	202,136	0	0	0	132,517	0	0

SHEET FLOW								
Flow length (ft.)	90	150	20		150	150	80	
Slope (ft./ft.)	0.33	0.25	0.04		0.11	0.15	0.04	
Manning's n for sheet flow	0.6	0.6	0.011		0.24	0.24	0.011	
2-year, 24-hour rainfall (in.)	3.4	3.4	3.4	3.4	3.4	3.4	3.4	3.4
Travel time (min.)	8.6	14.5	0.2	0.0	9.6	8.6	0.7	0.0

Flow length (ft.) Slope (ft./ft.) Kv (fps)

Travel time (min.)

	0.0	5.5	1.9	0.0	1.0	2.7	0.6	0.0
		2.5	20.3		7.0	7.0	20.3	
		0.13	0.01		0.11	0.17	0.04	
		295	235		144	470	150	
(IEB								

#### SHALLOW CONCENTRATED

Flow length (ft.)		760			131	890		
Slope (ft./ft.)		0.05			0.12	0.04		
Kv (fps)		7.0			2.5	7.0		
Travel time (min.)	0.0	8.1	0.0	0.0	2.5	10.6	0.0	0.0

OPEN CHANNEL OR PIPE								
Flow length (ft.)			130			400	2070	
Slope (ft./ft.)			0.03			0.02	0.05	
Manning's n for channel flow			0.012			0.012	0.012	
Open Channel - Bottom width (ft.)								
Open Channel - Side slopes								
Open Channel - Bank full depth (ft.)								
Pipe - Diameter (in.)			24			36	36	
Travel time (min.)	0.0	0.0	0.2	0.0	0.0	0.5	1.5	0.0
Time of Concentration (min.)	8.6	28.1	2.3	0.0	13.1	22.3	2.9	0.0

Subcatchment #			s17	s17(OW)	s18	s18(OW)	s19	s20	s20(OW)	s21
Total Area (sq.ft.)			5,045,436	7,156	986,815	20,560	676,035	812,610	85,729	4,184,190
Total Area (Ac.)			<b>115.827</b>	0.164	22.654	0.472	15.520	18.655	1.968	96.056
Composite CN			66	100	67	100	60	69	100	68
Tc (min.)			125.2	0.0	24.4	0.0	40.4	33.9	0.0	26.7
			Area	Area	Area	Area	Area	Area	Area	Area
Land Use	HSG	CN	(sq.ft.)	(sq.ft.)	(sq.ft.)	(sq.ft.)	(sq.ft.)	(sq.ft.)	(sq.ft.)	(sq.ft.)
Impervious	n/a	98	48,140	0	0	0	0	0	0	88,484
Open Water	n/a	100	0	7,156	0	20,560	0	0	85,729	0
						-	-		-	
Open Space	A	39	0	0	0	0	0	0	0	0
(good condition)	В	61	<mark>26,475</mark>	0	196,514	0	0	113,046	0	308,999
	С	74	<mark>75,456</mark>	0	63,398	0	0	224,042	0	427,921
	D	80	0	0	17,033	0	0	0	0	46,080
Brush	A	30	0	0	0	0	0	0	0	0
(good condition)	В	48	0	0	22,245	0	0	0	0	457,135
	С	65	<mark>324,679</mark>	0	64,022	0	0	3,473	0	68,092
	D	73	0	0	0	0	0	0	0	401,560
Woods	Α	30	0	0	0	0	0	0	0	0
(good condition)	В	55	1,275,839	0	68,490	0	448,248	71,428	0	773,186
	С	70	3,294,848	0	555,112	0	227,787	400,621	0	226,467
	D	77	0	0	0	0	0	0	0	1,386,265

SHEET FLOW								
Flow length (ft.)	150		150		150	150		150
Slope (ft./ft.)	0.10		0.41		0.07	0.06		0.30
Manning's n for sheet flow	0.6		0.6		0.6	0.6		0.6
2-year, 24-hour rainfall (in.)	3.4	3.4	3.4	3.4	3.4	3.4	3.4	3.4
Travel time (min.)	20.9	0.0	11.9	0.0	24.1	25.7	0.0	13.5

1120

0.43

2.5

11.4

0.0

0.0

770

0.52

2.5

7.1

33.9

0.0

0.0

810

0.11

2.5

16.3

40.4

206

0.25

2.5

2.7

26.7

Flow length (ft.)	
Slope (ft./ft.)	
Kv (fps)	
Travel time (min.)	

1700

0.14

2.5 30.3

125.2

0.0

0.0

#### SHALLOW CONCENTRATED

Flow length (ft.)	3330		140			160		1206
Slope (ft./ft.)	0.09		0.09			0.13		0.07
Kv (fps)	2.5		7.0			7.0		7.0
Travel time (min.)	74.0	0.0	1.1	0.0	0.0	1.1	0.0	10.5
OPEN CHANNEL OR PIPE			-					
Flow length (ft.)								
Slope (ft./ft.)								
Manning's n for channel flow								
Open Channel - Bottom width (ft.)								
Open Channel - Side slopes								
Open Channel - Bank full depth (ft.)								
Pipe - Diameter (in.)								
Travel time (min.)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

Time of Concentration (min.)

The Chazen Companies S:\1\10400-10499\10454.00\ENG\Stormwater\Existing Subcatchment Stats\_10454-01.xls 4/10/2006 3:45 PM

Subcatchment #			s21(OW)	s22	s22(OW)	s23	s24	s25
Total Area (sq.ft.)			532,953	3,584,440	5,908	1,811,550	1,233,858	590,760
Total Area (Ac.)			12.235	82.287	0.136	41.587	28.325	13.562
Composite CN			100	69	100	72	70	66
Tc (min.)			0.0	31.3	0.0	47.0	30.7	20.2
			Area	Area	Area	Area	Area	Area
Land Use	HSG	CN	(sq.ft.)	(sq.ft.)	(sq.ft.)	(sq.ft.)	(sq.ft.)	(sq.ft.)
Impervious	n/a	98	0	0	0	0	51,416	827
Open Water	n/a	100	532,953	0	5,908	0	0	0
Open Space	A	39	0	0	0	0	0	0
(good condition)	В	61	0	52,966	0	0	0	297,744
	С	74	0	374,915	0	0	0	0
	D	80	0	0	0	60,413	0	149,068
Brush	Α	30	0	0	0	0	0	0
(good condition)	В	48	0	0	0	17,593	65,641	0
	С	65	0	0	0	0	0	0
	D	73	0	0	0	627,888	624,972	43,255
Woods	Α	30	0	0	0	0	0	0
(good condition)	В	55	0	418,537	0	31,436	268,274	80,367
	С	70	0	2,738,021	0	899,506	0	0
	D	77	0	0	0	174,714	223,556	19,499

#### SHEET FLOW

Flow length (ft.) Slope (ft./ft.) Kv (fps)

Travel time (min.)

Flow length (ft.)		150		150	150	150
Slope (ft./ft.)		0.23		0.13	0.13	0.07
Manning's n for sheet flow		0.6		0.6	0.6	0.24
2-year, 24-hour rainfall (in.)	3.4	3.4	3.4	3.4	3.4	3.4
Travel time (min.)	0.0	15.0	0.0	18.9	18.8	11.6

#### SHALLOW CONCENTRATED Flow length

Flow length (ft.)		1310		1290
Slope (ft./ft.)		0.42		0.39
Kv (fps)		2.5		2.5
Travel time (min.)	0.0	13.5	0.0	13.8

#### SHALLOW CONCENTRATED

	430		1480	1084	225
	0.15		0.06	0.07	0.04
	7.0		7.0	7.0	7.0
0.0	2.6	0.0	14.4	9.4	2.7

285

0.16

7.0

1.7

152

0.17

2.5

2.4

OPEN CHANNEL OR PIPE						
Flow length (ft.)		210				575
Slope (ft./ft.)		0.06				0.01
Manning's n for channel flow		0.012				0.045
Open Channel - Bottom width (ft.)						15
Open Channel - Side slopes						7.5
Open Channel - Bank full depth (ft.)						1
Pipe - Diameter (in.)		36				
Travel time (min.)	0.0	0.1	0.0	0.0	0.0	4.2
Time of Concentration (min.)	0.0	31.3	0.0	47.0	30.7	20.2

Subcatchment #			s01-0	s02-1	s02-2	s02-3	s03-1	s03-2	s03-2(IC)	s03-2(OW)
Total Area (sq.ft.)			500,284	3,728,339	338,709	178,094	454,566	131,602	72,450	2,362
Total Area (Ac.)			11.485	85.591	7.776	4.088	10.435	3.021	1.663	0.054
Composite CN			68	65	66	86	70	74	98	100
Tc (min.)			42.8	61.3	29.4	1.6	28.8	1.5	1.5	0.0
			Area	Area	Area	Area	Area	Area	Area	Area
Land Use	HSG	CN	(sq.ft.)	(sq.ft.)	(sq.ft.)	(sq.ft.)	(sq.ft.)	(sq.ft.)	(sq.ft.)	(sq.ft.)
Impervious	n/a	98	27,212	124,310	29,794	81,609	38,120	0	72,450	0
				_						
Open Water	n/a	100	0	0	0	0	0	0	0	2,362
Open Space	A	39	0	0	0	0	0	0	0	0
(good condition)	В	61	0	59,500	30,292	0	5,413	0	0	0
	С	74	0	163,945	0	0	0	131,602	0	0
	D	80	0	39,509	33,422	80,809	0	0	0	0
				_						
Brush	A	30	0	0	0	0	0	0	0	0
(good condition)	В	48	0	148,070	75,018	5,299	0	0	0	0
	С	65	69,152	105,460	146,030	10,378	160,355	0	0	0
	D	73	0	9,429	0	0	0	0	0	0
				_						
Woods	А	30	0	0	0	0	0	0	0	0
(good condition)	В	55	82,999	1,328,759	0	0	21,986	0	0	0
	С	70	320,921	1,542,365	24,152	0	228,692	0	0	0
	D	77	0	206,993	0	0	0	0	0	0

Subcatchment #	s01-0	s02-1	s02-2	s02-3	<mark>s03-1</mark>	<mark>s03-2</mark>	s03-2(IC)	s03-2(OW)
SHEET FLOW								
Flow length (ft.)	150	150	150	100	20	50	50	
Slope (ft./ft.)	0.03	0.07	0.13	0.17	0.03	0.04	0.04	
Manning's n for sheet flow	0.6	0.6	0.6	0.011	0.011	0.011	0.011	

Slope (ft./ft.)	0.03	0.07	0.13	0.17	0.03	0.04
Manning's n for sheet flow	0.6	0.6	0.6	0.011	0.011	0.011
2-year, 24-hour rainfall (in.)	3.4	3.4	3.4	3.4	3.4	3.4
Travel time (min.)	33.9	24.1	18.9	0.5	0.3	0.5

SHALLOW CONCENTRATED								
Flow length (ft.)	420	2100	570	180	1480	80	80	
Slope (ft./ft.)	0.29	0.15	0.22	0.05	0.12	0.13	0.13	
Kv (fps)	2.5	2.5	2.5	20.3	2.5	7.0	7.0	
Travel time (min.)	5.2	36.1	8.1	0.7	28.5	0.5	0.5	0.0

3.4

0.5

3.4

0.0

#### SHALLOW CONCENTRATED

Flow length (ft.)	340		240					
Slope (ft./ft.)	0.09		0.15					
Kv (fps)	7.0		7.0					
Travel time (min.)	2.7	0.0	1.5	0.0	0.0	0.0	0.0	0.0

OPEN CHANNEL OR PIPE								
Flow length (ft.)	650	1700	640	730		600	600	
Slope (ft./ft.)	0.08	0.12	0.08	0.11		0.08	0.08	
Manning's n for channel flow	0.027	0.04	0.027	0.012		0.012	0.012	
Open Channel - Bottom width (ft.)	2	5	2					
Open Channel - Side slopes	3	2	3					
Open Channel - Bank full depth (ft.)	1	6	1					
Pipe - Diameter (in.)				30		24	24	
Travel time (min.)	1.0	1.0	1.0	0.4	0.0	0.5	0.5	0.0
Time of Concentration (min.)	42.8	61.3	29.4	1.6	28.8	1.5	1.5	0.0

Subcatchment #			s04-1	s05-1	s06-0	s06-0(OW)	s07-1	s07-1(OW)	s08-1	s08-2
Total Area (sq.ft.)			328,852	298,028	392,339	18,652	202,800	22,053	1,007,378	390,196
Total Area (Ac.)			7.549	6.842	9.007	0.428	4.656	0.506	23.126	8.958
Composite CN			68	61	62	100	64	100	60	63
Tc (min.)			6.1	14.4	17.3	0.0	9.3	0.0	24.5	11.4
			Area	Area	Area	Area	Area	Area	Area	Area
Land Use	HSG	CN	(sq.ft.)	(sq.ft.)	(sq.ft.)	(sq.ft.)	(sq.ft.)	(sq.ft.)	(sq.ft.)	(sq.ft.)
Impervious	n/a	98	0	0	0	0	0	0	6,637	0
Open Water	n/a	100	0	0	0	18,652	0	22,053	0	0
Open Space	Α	39	0	0	0	0	0	0	0	0
(good condition)	В	61	0	267,689	369,612	0	160,287	0	836,719	180,706
	С	74	13,811	13,094	0	0	42,513	0	0	48,324
	D	80	0	0	22,727	0	0	0	6,256	0
Brush	Α	30	0	0	0	0	0	0	0	0
(good condition)	В	48	0	17,245	0	0	0	0	81,324	26,228
	С	65	129,457	0	0	0	0	0	0	<mark>118,030</mark>
	D	73	0	0	0	0	0	0	0	0
Woods	Α	30	0	0	0	0	0	0	0	0
(good condition)	В	55	0	0	0	0	0	0	72,619	0
	С	70	185,584	0	0	0	0	0	0	<mark>16,909</mark>
	D	77	0	0	0	0	0	0	3,823	0

Subcatchment #	s04-1	s05-1	s06-0	s06-0(OW)	<mark>s07-1</mark>	s07-1(OW)	s08-1	s08-2
SHEET FLOW								
Flow length (ft.)		100	100		95		150	120
Slope (ft./ft.)		0.14	0.12		0.14		0.21	0.08
Manning's n for sheet flow		0.24	0.24		0.24		0.24	0.24

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2-year, 24-hour rainfall (in.)	3.4	3.4	3.4	3.4	3.4	3.4	3.4	3.4
Travel time (min.)	0.0	6.4	6.8	0.0	6.1	0.0	7.5	9.2
SHALLOW CONCENTRATED								
Flow length (ft.)	990	830	490		485		1105	25
Slope (ft./ft.)	0.15	0.06	0.04		0.13		0.08	0.04
Kv (fps)	7.0	7.0	7.0		7.0		7.0	20.3

Slope (ft./ft.) Kv (fps) Travel time (min.)

#### SHALLOW CONCENTRATED

6.1

8.1

Flow length (ft.)			630				450	
Slope (ft./ft.)			0.10				0.02	
Kv (fps)			7.0				7.0	
Travel time (min.)	0.0	0.0	4.7	0.0	0.0	0.0	7.6	0.0
OPEN CHANNEL OR PIPE								
Flow length (ft.)							65	2160

5.8

Flow length (ft.)							65	2160
Slope (ft./ft.)							0.020	0.050
Manning's n for channel flow							0.06	0.012
Open Channel - Bottom width (ft.)							8	
Open Channel - Side slopes							1	
Open Channel - Bank full depth (ft.)							4	
Pipe - Diameter (in.)								24
Travel time (min.)	0.0	0.0	0.0	0.0	0.0	0.0	0.2	2.1
Time of Concentration (min.)	6.1	14.4	17.3	0.0	9.3	0.0	24.5	11.4

0.0

3.2

0.0

9.3

0.1

Subcatchment #			s08-2(IC)	s08-2(OW)		s08-3(IC)	s08-3(OW)	s09-1	s09-2	s09-2(IC)
Total Area (sq.ft.)			240,632	8,377	74,072	47,304	1,849	113,411	810,566	101,750
Total Area (Ac.)			5.524	0.192	1.700	1.086	0.042	2.604	18.608	2.336
Composite CN			98	100	61	98	100	60	67	98
Tc (min.)			2.9	0.0	13.6	1.0	0.0	9.0	20.6	2.5
			Area	Area	Area	Area	Area	Area	Area	Area
Land Use	HSG	CN	(sq.ft.)	(sq.ft.)	(sq.ft.)	(sq.ft.)	(sq.ft.)	(sq.ft.)	(sq.ft.)	(sq.ft.)
Impervious	n/a	98	240,632	0	0	47,304	0	0	0	101,750
Open Water	n/a	100	0	8,377	0	0	1,849	0	0	0
Open Space	Α	39	0	0	0	0	0	0	0	0
(good condition)	В	61	0	0	74,072	0	0	88,876	16,371	0
	С	74	0	0	0	0	0	0	149,060	0
	D	80	0	0	0	0	0	0	0	0
Brush	Α	30	0	0	0	0	0	0	0	0
(good condition)	В	48	0	0	0	0	0	0	0	0
	С	65	0	0	0	0	0	0	547,008	0
	D	73	0	0	0	0	0	0	0	0
Woods	Α	30	0	0	0	0	0	0	0	0
(good condition)	В	55	0	0	0	0	0	24,535	0	0
	С	70	0	0	0	0	0	0	98,126	0
	D	77	0	0	0	0	0	0	0	0

Subcatchment #	s08-2(IC)	s08-2(OW)	s08-3	s08-3(IC)	s08-3(OW)	s09-1	s09-2	s09-2(IC)
SHEET FLOW								
Flow length (ft.)	35		150	70		105	150	150
Slope (ft./ft.)	0.03		0.09	0.08		0.10	0.16	0.03

Flow length (ft.)	35		150	70		105	150	150
Slope (ft./ft.)	0.03		0.09	0.08		0.10	0.16	0.03
Manning's n for sheet flow	0.011		0.24	0.011		0.24	0.24	0.011
2-year, 24-hour rainfall (in.)	3.4	3.4	3.4	3.4	3.4	3.4	3.4	4.4
Travel time (min.)	0.4	0.0	10.5	0.5	0.0	7.6	8.3	1.2

130		410			50	995	100
0.03		0.10			0.24	0.13	0.09
20.3		7.0			7.0	7.0	20.3
0.6	0.0	3.1	0.0	0.0	0.2	6.6	0.3

# SHALLOW CONCENTRATED

SHALLOW CONCENTRATED

Flow length (ft.) Slope (ft./ft.) Kv (fps)

Travel time (min.)

Flow length (ft.) Slope (ft./ft.) Kv (fps)							400 0.03 7.0	
Travel time (min.)	0.0	0.0	0.0	0.0	0.0	0.0	5.5	0.0
OPEN CHANNEL OR PIPE								
Flow length (ft.)	2105			400		540	310	940
Slope (ft./ft.)	0.06			0.080		0.05	0.09	0.06
Manning's n for channel flow	0.012			0.012		0.027	0.012	0.012
Open Channel - Bottom width (ft.)						15		
Open Channel - Side slopes						3		

Time of Concentration (min.)

Pipe - Diameter (in.)

Travel time (min.)

#### Open Channel - Bank full depth (ft.) 0.5 24 12 24 0.0 0.0 0.5 0.0 1.2 0.2 1.8 2.9 0.0 13.6 1.0 0.0 9.0 20.6

18

1.0

Subcatchment #			s09-2(OW)	s09-3	s10-1	s10-1(OW)	s13-1	s13-1(IC)	s13-1(OW)	s14-1
Total Area (sq.ft.)			10,285	166,298	350,123	36,160	154,850	277,041	5,725	597,966
Total Area (Ac.)			0.236	3.818	8.038	0.830	3.555	6.360	0.131	13.727
Composite CN			100	68	68	100	61	98	100	<u>69</u>
Tc (min.)			0.0	10.9	27.9	0.0	2.8	2.8	0.0	<b>28.1</b>
			Area	Area	Area	Area	Area	Area	Area	Area
Land Use	HSG	CN	(sq.ft.)	(sq.ft.)	(sq.ft.)	(sq.ft.)	(sq.ft.)	(sq.ft.)	(sq.ft.)	(sq.ft.)
Impervious	n/a	98	0	1,491	4,456	0	0	277,041	0	0
Open Water	n/a	100	10,285	0	0	36,160	0	0	5,725	0
Open Space	Α	39	0	0	0	0	0	0	0	0
(good condition)	В	61	0	75,342	52,781	0	154,850	0	0	<mark>113,977</mark>
	С	74	0	74,024	0	0	0	0	0	0
	D	80	0	0	152,794	0	0	0	0	55,324
Brush	Α	30	0	0	0	0	0	0	0	0
(good condition)	В	48	0	0	0	0	0	0	0	0
	С	65	0	0	0	0	0	0	0	0
	D	73	0	0	0	0	0	0	0	94,535
Woods	Α	30	0	0	0	0	0	0	0	0
(good condition)	В	55	0	0	136,501	0	0	0	0	<u>131,871</u>
	С	70	0	15,442	0	0	0	0	0	0
	D	77	0	0	3,591	0	0	0	0	202,259

Subcatchment #	s09-2(OW)	<mark>s09-3</mark>	<mark>s10-1</mark>	s10-1(OW)	<mark>s13-1</mark>	s13-1(IC)	s13-1(OW)	<mark>s14-1</mark>
SHEET FLOW								
Flow length (ft.)		150	130		12	12		150
Slope (ft./ft.)		0.17	0.09		0.02	0.02		0.25

0.6

3.4

19.5

150

0.23

2.5

2.1

3.4

0.0

0.0

0.011

3.4

0.2

300

0.02

20.3

1.7

0.011

3.4

0.2

300

0.02

20.3

1.7

3.4

0.0

0.0

0.6

3.4

14.5

295

0.13

2.5

5.5

0.24

3.4

8.1

325

0.10

7.0

2.4

3.4

0.0

0.0

Slope (ft./ft.) Slope (ft./ft.) Manning's n for sheet flow 2-year, 24-hour rainfall (in.) Travel time (min.)

#### SHALLOW CONCENTRATED Flow length (ft.) Slope (ft./ft.) Kv (fps) Travel time (min.)

#### SHALLOW CONCENTRATED

	_							
Flow length (ft.)			600					760
Slope (ft./ft.)			0.05					0.05
Kv (fps)			7.0					7.0
Travel time (min.)	0.0	0.0	6.4	0.0	0.0	0.0	0.0	8.1

OPEN CHANNEL OR PIPE								
Flow length (ft.)		170			840	840		
Slope (ft./ft.)		0.09			0.03	0.03		
Manning's n for channel flow		0.027			0.012	0.012		
Open Channel - Bottom width (ft.)		15						
Open Channel - Side slopes		3						
Open Channel - Bank full depth (ft.)		0.5						
Pipe - Diameter (in.)					30	30		
Travel time (min.)	0.0	0.3	0.0	0.0	0.9	0.9	0.0	0.0
Time of Concentration (min.)	0.0	10.9	27.9	0.0	2.8	2.8	0.0	28.1

Subcatchment #			s14-1(IC)	s14-1(OW)	s14-2	s14-2(OW)	s14-3	s14-3(IC)	s16-1	s16-1(OW)
Total Area (sq.ft.)			80,151	22,563	21,960	7,669	295,959	368,525	1,728,453	233,105
Total Area (Ac.)			1.840	0.518	0.504	0.176	6.794	8.460	39.680	5.351
Composite CN			98	100	61	100	69	98	67	100
Tc (min.)			2.3	0.0	6.4	0.0	8.5	2.9	19.0	0.0
			Area	Area	Area	Area	Area	Area	Area	Area
Land Use	HSG	CN	(sq.ft.)	(sq.ft.)	(sq.ft.)	(sq.ft.)	(sq.ft.)	(sq.ft.)	(sq.ft.)	(sq.ft.)
Impervious	n/a	98	80,151	0	0	0	0	368,525	0	0
	<u> </u>									
Open Water	n/a	100	0	22,563	0	7,669	0	0	0	233,105
			1							
Open Space	A	39	0	0	0	0	0	0	0	0
(good condition)	В	61	0	0	21,960	0	90,323	0	901,475	0
	С	74	0	0	0	0	174,097	0	392,889	0
	D	80	0	0	0	0	0	0	235,994	0
Brush	A	30	0	0	0	0	0	0	0	0
(good condition)	В	48	0	0	0	0	0	0	0	0
	С	65	0	0	0	0	31,539	0	198,095	0
	D	73	0	0	0	0	0	0	0	0
Woods	A	30	0	0	0	0	0	0	0	0
(good condition)	В	55	0	0	0	0	0	0	0	0
	С	70	0	0	0	0	0	0	0	0
	D	77	0	0	0	0	0	0	0	0

Subcatchment #	s14-1(IC)	s14-1(OW)	s14-2	s14-2(OW)	s14-3	s14-3(IC)	s16-1	s16-1(OW)
							-	

20		130		150	40	100	
0.04		0.23		0.31	0.05	0.16	
0.011		0.24		0.24	0.011	0.24	
3.4	3.4	3.4	3.4	3.4	3.4	3.4	3.4
0.2	0.0	6.4	0.0	6.4	0.4	6.0	0.0
	0.04 0.011 3.4	0.04 0.011 3.4 3.4	0.04         0.23           0.011         0.24           3.4         3.4	0.04         0.23           0.011         0.24           3.4         3.4         3.4	0.04         0.23         0.31           0.011         0.24         0.24           3.4         3.4         3.4         3.4	0.04         0.23         0.31         0.05           0.011         0.24         0.24         0.011           3.4         3.4         3.4         3.4         3.4	0.04         0.23         0.31         0.05         0.16           0.011         0.24         0.24         0.011         0.24           3.4         3.4         3.4         3.4         3.4         3.4

235				30	130	330	
0.01				0.03	0.02	0.17	
20.3				7.0	20.3	7.0	
1.9	0.0	0.0	0.0	0.4	0.8	1.9	0.0

# SHALLOW CONCENTRATED

SHALLOW CONCENTRATED

Flow length (ft.) Slope (ft./ft.) Kv (fps)

Travel time (min.)

Flow length (ft.)					150		890	
Slope (ft./ft.)					0.05		0.04	
Kv (fps)					20.3		7.0	
Travel time (min.)	0.0	0.0	0.0	0.0	0.6	0.0	10.6	0.0
OPEN CHANNEL OR PIPE								
Flow length (ft.)	130				1415	2095	400	
Slope (ft./ft.)	0.03				0.05	0.05	0.02	
Manning's n for channel flow	0.012				0.012	0.012	0.012	
Open Channel - Bottom width (ft.)								
Open Channel - Side slopes								
Open Channel - Bank full depth (ft.)								
Pipe - Diameter (in.)	24				30	30	36	
Travel time (min.)	0.2	0.0	0.0	0.0	1.2	1.7	0.5	0.0
· ·			•			•	•	

0.0

2.3

0.0

8.5

2.9

19.0

0.0

Time of Concentration (min.)

Subcatchment #			s16-2	s17-1	s17-1(OW)	s17-2	s17-3	s18-1	s18-1(OW)	s18-2
Total Area (sq.ft.)			94,766	266,146	7,156	3,314,288	1,301,580	367,146	20,560	467,023
Total Area (Ac.)			2.176	6.110	0.164	76.086	29.880	8.429	0.472	10.721
Composite CN			76	64	100	66	66	64	100	69
Tc (min.)			15.8	33.8	0.0	95.7	30.7	16.4	0.0	18.0
			Area	Area	Area	Area	Area	Area	Area	Area
Land Use	HSG	CN	(sq.ft.)	(sq.ft.)	(sq.ft.)	(sq.ft.)	(sq.ft.)	(sq.ft.)	(sq.ft.)	(sq.ft.)
Impervious	n/a	98	0	0	0	46,821	0	0	0	0
					-					
Open Water	n/a	100	0	0	7,156	0	0	0	20,560	0
				r					,	
Open Space	A	39	0	0	0	0	0	0	0	0
(good condition)	В	61	2,152	30,811	0	0	0	199,079	0	0
	С	74	0	0	0	84,708	43,360	86,894	0	22,599
	D	80	8,119	0	0	0	0	16,165	0	0
Brush	Α	30	0	0	0	0	0	0	0	0
(good condition)	В	48	0	10,726	0	0	0	22,235	0	0
	С	65	0	38,075	0	524,806	0	9,704	0	0
	D	73	0	0	0	0	0	0	0	0
Woods	Α	30	0	0	0	0	0	0	0	0
(good condition)	В	55	1,946	57,903	0	888,814	318,092	24,699	0	43,778
	С	70	0	128,631	0	1,769,139	940,128	8,369	0	400,646
	D	77	82,549	0	0	0	0	0	0	0

Subcatchment #	s16-2	s17-1	s17-1(OW)	s17-2	s17-3	s18-1	s18-1(OW)	s18-2

SHEET FLOW								
Flow length (ft.)	150	60		150	150	100		150
Slope (ft./ft.)	0.31	0.17		0.10	0.08	0.15		0.41
Manning's n for sheet flow	0.6	0.24		0.6	0.6	0.24		0.6
2-year, 24-hour rainfall (in.)	3.4	3.4	3.4	3.4	3.4	3.4	3.4	3.4
Travel time (min.)	13.3	3.9	0.0	20.9	22.9	6.2	0.0	<b>11.9</b>

0.0

1700

0.14

2.5

30.3

815

0.52

2.5

7.5

1140

0.07

7.0

10.3

0.0

635

0.58

2.5

5.6

SHALLOW CONCENTRATED		
Flow length (ft.)	140	360
Slope (ft./ft.)	0.14	0.11
Kv (fps)	2.5	7.0
Travel time (min.)	2.5	2.6

# SHALLOW CONCENTRATED

SHALLOW CONCENTRATED								
Flow length (ft.)		1160		2000				
Slope (ft./ft.)		0.08		0.09				
Kv (fps)		2.5		2.5				
Travel time (min.)	0.0	27.3	0.0	44.4	0.0	0.0	0.0	0.0
OPEN CHANNEL OR PIPE					440			400
Flow length (ft.)					410			490
Slope (ft./ft.)					0.12			0.06
Manning's n for channel flow					0.027			0.027
Open Channel - Bottom width (ft.)					4			4
Open Channel - Side slopes					2			2
Open Channel - Bank full depth (ft.)					2			2
Pipe - Diameter (in.)								
Travel time (min.)	0.0	0.0	0.0	0.0	0.3	0.0	0.0	0.5
Time of Concentration (min.)	15.8	33.8	0.0	95.7	30.7	16.4	0.0	18.0

Subcatchment #			s19-0	s20-1	s20-1(OW)	s20-2	s20-2(IC)	s20-2(OW)	s20-3	s21-1
Total Area (sq.ft.)			676,035	372,811	85,729	355,306	222,700	10,535	299,956	2,785,332
Total Area (Ac.)			15.520	8.559	1.968	8.157	5.112	0.242	6.886	63.942
Composite CN			60	66	100	74	98	100	70	68
Tc (min.)			40.4	21.5	0.0	8.4	4.7	0.0	22.0	16.6
			Area	Area	Area	Area	Area	Area	Area	Area
Land Use	HSG	CN	(sq.ft.)	(sq.ft.)	(sq.ft.)	(sq.ft.)	(sq.ft.)	(sq.ft.)	(sq.ft.)	(sq.ft.)
Impervious	n/a	98	0	0	0	0	222,700	0	0	<mark>96,930</mark>
Open Water	n/a	100	0	0	85,729	0	0	10,535	0	0
Open Space	Α	39	0	0	0	0	0	0	0	0
(good condition)	В	61	0	113,045	0	0	0	0	0	458,230
	С	74	0	161,003	0	355,306	0	0	22,335	36,657
	D	80	0	0	0	0	0	0	0	22,358
Brush	A	30	0	0	0	0	0	0	0	0
(good condition)	В	48	0	0	0	0	0	0	0	407,425
	С	65	0	0	0	0	0	0	0	40,120
	D	73	0	0	0	0	0	0	0	352,287
Woods	Α	30	0	0	0	0	0	0	0	0
(good condition)	В	55	448,248	67,504	0	0	0	0	3,398	265,999
	С	70	227,787	31,259	0	0	0	0	274,223	0
	D	77	0	0	0	0	0	0	0	1,105,325

Subcatchment #	s19-0	s20-1	s20-1(OW)	s20-2	s20-2(IC)	s20-2(OW)	s20-3	s21-1

150	150		50	40		150	150
0.07	0.02		0.10	0.04		0.20	0.11
0.6	0.24		0.24	0.011		0.6	0.24
3.4	3.4	3.4	3.4	3.4	3.4	3.4	3.4
24.1	19.1	0.0	4.2	0.4	0.0	15.9	9.7
	0.07 0.6 3.4	0.07         0.02           0.6         0.24           3.4         3.4	0.07         0.02           0.6         0.24           3.4         3.4	0.07         0.02         0.10           0.6         0.24         0.24           3.4         3.4         3.4	0.07         0.02         0.10         0.04           0.6         0.24         0.24         0.011           3.4         3.4         3.4         3.4	0.07         0.02         0.10         0.04           0.6         0.24         0.24         0.011           3.4         3.4         3.4         3.4         3.4	0.07         0.02         0.10         0.04         0.20           0.6         0.24         0.24         0.011         0.6           3.4         3.4         3.4         3.4         3.4         3.4

SHALLOW CONCENTRATED								
Flow length (ft.)	810	165		300	300		685	420
Slope (ft./ft.)	0.11	0.03		0.07	0.07		0.56	0.12
Kv (fps)	2.5	7.0		20.3	20.3		2.5	7.0
Travel time (min.)	16.3	2.3	0.0	0.9	0.9	0.0	6.1	2.9

0.5

4.0

16.6

#### SHALLOW CONCENTRATED

SHALLOW CONCLIMINATED								
Flow length (ft.)								
Slope (ft./ft.)								
Kv (fps)								
Travel time (min.)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
OPEN CHANNEL OR PIPE								
Flow length (ft.)		125		2700	2700			641
Slope (ft./ft.)		0.32		0.03	0.03			0.01
Manning's n for channel flow		0.027		0.00	0.012			0.027
Open Channel - Bottom width (ft.)		5						10
Open Channel - Side slopes		2.5						20

0.0

0.0

1

0.1

21.5

40.4

Manning's n for channel flow		
Open Channel - Bottom width (ft.)		
Open Channel - Side slopes		
Open Channel - Bank full depth (ft.)		
Pipe - Diameter (in.)		
Travel time (min.)	0.0	
		1

Time of Concentration (min.)

24

3.3

8.4

24

3.3

4.7

0.0

0.0

0.0

Subcatchment #			s21-1(OW)	s21-2	s21-3	s21-4	s21-4(IC)	s21-4(OW)	s21-5	s21-6
Total Area (sq.ft.)			532,953	912,179	373,187	147,753	71,581	5,112	104,457	237,956
Total Area (Ac.)			12.235	20.941	8.567	3.392	1.643	0.117	2.398	<b>5.463</b>
Composite CN			100	70	74	67	98	100	75	76
Tc (min.)			0.0	31.4	11.2	13.8	1.6	0.0	13.9	17.5
			Area	Area	Area	Area	Area	Area	Area	Area
Land Use	HSG	CN	(sq.ft.)	(sq.ft.)	(sq.ft.)	(sq.ft.)	(sq.ft.)	(sq.ft.)	(sq.ft.)	(sq.ft.)
Impervious	n/a	98	0	0	0	0	71,581	0	9,459	0
Open Water	n/a	100	532,953	0	0	0	0	5,112	0	0
Open Space	A	39	0	0	0	0	0	0	0	0
(good condition)	В	61	0	0	0	65,537	0	0	0	0
	С	74	0	62,906	216,942	69,978	0	0	0	0
	D	80	0	0	80,724	4,825	0	0	0	48,207
Brush	A	30	0	0	0	0	0	0	0	0
(good condition)	В	48	0	0	0	0	0	0	0	2,439
	С	65	0	0	9,400	0	0	0	0	0
	D	73	0	0	12,071	0	0	0	0	0
Woods	Α	30	0	0	0	0	0	0	0	0
(good condition)	В	55	0	39,608	0	7,413	0	0	19,232	9,137
	С	70	0	809,666	54,051	0	0	0	0	0
	D	77	0	0	0	0	0	0	75,766	178,173

Subcatchment #	s21-1(OW)	<mark>s21-2</mark>	s21-3	<mark>s21-4</mark>	s21-4(IC)	s21-4(OW)	s21-5	s21-6
SHEET FLOW								
Flow length (ft.)		150	150	150	12		150	80
Slope (ft /ft )		0.12	0.10	0.07	0.02		0.47	0.00

Flow length (ft.)		150	150	150	12		150	80
Slope (ft./ft.)		0.12	0.19	0.07	0.02		0.47	0.09
Manning's n for sheet flow		0.6	0.24	0.24	0.011		0.6	0.6
2-year, 24-hour rainfall (in.)	3.4	3.4	3.4	3.4	3.4	3.4	3.4	3.4
Travel time (min.)	0.0	19.4	7.8	11.6	0.2	0.0	11.3	13.2

495

0.13

7.0

3.3

630 0.30

2.5

7.6

Travel time (min.)	0.0
Slope (ft./ft.) Kv (fps)	
Slope (ft (ft))	
Flow length (ft.)	
SHALLOW CONCENTRATED	

#### SHALLOW CONCENTRATED Flow lengt

Flow length (ft.)		475		145				170
Slope (ft./ft.)		0.54		0.05				0.06
Kv (fps)		2.5		20.3				7.0
Travel time (min.)	0.0	4.3	0.0	0.5	0.0	0.0	0.0	1.7

130

0.14

7.0

0.8

145

0.05

20.3

0.5

170

0.19

2.5

2.6

0.0

250

0.40

2.5

OPEN CHANNEL OR PIPE								
Flow length (ft.)		170	150	585	585			
Slope (ft./ft.)		0.65	0.05	0.02	0.02			
Manning's n for channel flow		0.027	0.027	0.012	0.012			
Open Channel - Bottom width (ft.)		4	4					
Open Channel - Side slopes		2	2					
Open Channel - Bank full depth (ft.)		2	2					
Pipe - Diameter (in.)				24	24			
Travel time (min.)	0.0	0.1	0.2	0.9	0.9	0.0	0.0	0.0
Time of Concentration (min.)	0.0	31.4	11.2	13.8	1.6	0.0	13.9	17.5

Subcatchment #			s21-6(IC)	s21-6(OW)	s21-7	s21-7(IC)	s21-7(OW)	s22-1	s22-1(OW)	s22-2
Total Area (sq.ft.)			28,022	3,323	190,567	169,450	3,941	778,782	5,908	1,953,600
Total Area (Ac.)			0.643	0.076	4.375	3.890	0.090	17.878	0.136	44.848
Composite CN			98	100	64	98	100	72	100	68
Tc (min.)			1.5	0.0	13.1	2.5	0.0	14.7	0.0	24.0
			Area	Area	Area	Area	Area	Area	Area	Area
Land Use	HSG	CN	(sq.ft.)	(sq.ft.)	(sq.ft.)	(sq.ft.)	(sq.ft.)	(sq.ft.)	(sq.ft.)	(sq.ft.)
						-				
Impervious	n/a	98	28,022	0	0	169,450	0	0	0	0
			1			n				
Open Water	n/a	100	0	3,323	0	0	3,941	0	5,908	0
				1						
Open Space	Α	39	0	0	0	0	0	0	0	0
(good condition)	В	61	0	0	157,573	0	0	39,566	0	0
	С	74	0	0	0	0	0	576,195	0	92,776
	D	80	0	0	0	0	0	0	0	0
			1			n	· · · · · ·			
Brush	А	30	0	0	0	0	0	0	0	0
(good condition)	В	48	0	0	0	0	0	0	0	0
	С	65	0	0	0	0	0	0	0	0
	D	73	0	0	0	0	0	0	0	0
						-				
Woods	А	30	0	0	0	0	0	0	0	0
(good condition)	В	55	0	0	0	0	0	44,938	0	<mark>347,333</mark>
	С	70	0	0	0	0	0	118,082	0	<mark>1,513,491</mark>
	D	77	0	0	32,994	0	0	0	0	0

Subcatchment #	s21-6(IC)	s21-6(OW)	s21-7	s21-7(IC)	s21-7(OW)	s22-1	s22-1(OW)	s22-2

SHEET FLOW								
Flow length (ft.)	12		150	12		150		150
Slope (ft./ft.)	0.02		0.40	0.02		0.23		0.23
Manning's n for sheet flow	0.011		0.6	0.011		0.24		0.6
2-year, 24-hour rainfall (in.)	3.4	3.4	3.4	3.4	3.4	3.4	3.4	3.4
Travel time (min.)	0.2	0.0	12.0	0.2	0.0	7.2	0.0	15.0

SHALLOW CONCENTRATED				
Flow length (ft.)	300			300
Slope (ft./ft.)	0.06			0.04
Kv (fps)	20.3			20.3
Travel time (min.)	1.0	0.0	0.0	1.2

24

0.2

0.0

# SHALLOW CONCENTRATED

Flow length (ft.) Slope (ft./ft.) Kv (fps)						405 0.09 7.0		
Travel time (min.)	0.0	0.0	0.0	0.0	0.0	<b>3.2</b>	0.0	0.0
OPEN CHANNEL OR PIPE								
Flow length (ft.)	230		1100	1100		140		180
Slope (ft./ft.)	0.04		0.05	0.05		0.08		0.28
Manning's n for channel flow	0.012		0.012	0.012		0.024		0.027
Open Channel - Bottom width (ft.)								4

24

1.0

#### Open Channel - Bottom width (ft.) Open Channel - Side slopes Open Channel - Bank full depth (ft.) Pipe - Diameter (in.) Travel time (min.)

Time of Concentration (min.)

#### 1.5 0.0 13.1 2.5 0.0 14.7 0.0 The Chazen Companies S:\1\10400-10499\10454.00\ENG\Stormwater\Proposed Subcatchment Stats\_10454-01.xls

24

1.0

4/10/2006 3:38 PM

0.0

0.0

440

0.07

7.0

4.1

36

0.2

930

0.48

2.5

8.9

2

2

0.1

24.0

0.0

Subcatchment #			s23-1	s23-2	s23-2(IC)	s23-2(OW)	s24-0	s25-0
Total Area (sq.ft.)			1,268,592	380,757	312,986	7,318	1,233,858	590,760
Total Area (Ac.)			29.123	8.741	7.185	0.168	28.325	13.562
Composite CN			72	77	98	100	70	66
Tc (min.)			38.5	4.0	4.0	0.0	30.7	20.2
			Area	Area	Area	Area	Area	Area
Land Use	HSG	CN	(sq.ft.)	(sq.ft.)	(sq.ft.)	(sq.ft.)	(sq.ft.)	(sq.ft.)
Impervious	n/a	98	0	0	312,986	0	51,416	827
Open Water	n/a	100	0	0	0	7,318	0	0
Open Space	A	39	0	0	0	0	0	0
(good condition)	В	61	0	0	0	0	0	297,744
	С	74	0	169,563	0	0	0	0
	D	80	55,373	211,194	0	0	0	149,069
Brush	A	30	0	0	0	0	0	0
(good condition)	В	48	0	0	0	0	65,641	0
	С	65	17,593	0	0	0	0	0
	D	73	286,181	0	0	0	624,971	43,255
Woods	A	30	0	0	0	0	0	0
(good condition)	В	55	0	0	0	0	268,274	80,367
	С	70	816,466	0	0	0	0	0
	D	77	92,979	0	0	0	223,556	19,499

Subcatchment #	s23-1	s23-2	s23-2(IC)	s23-2(OW)	s24-0	s25-0

SHEET FLOW						
Flow length (ft.)	150	150	150		150	150
Slope (ft./ft.)	0.13	0.02	0.02		0.13	0.07
Manning's n for sheet flow	0.6	0.011	0.011		0.6	0.24
2-year, 24-hour rainfall (in.)	3.4	3.4	3.4	3.4	3.4	3.4
Travel time (min.)	18.9	1.6	1.6	0.0	18.8	11.6

88

0.03

20.3

0.4

1290

0.39

2.5

13.8

SHALLOW CONCENTRATED
Flow length (ft.)
Slope (ft./ft.)
Kv (fps)
Travel time (min.)

#### SHALLOW CONCENTRATED Flow length (ft.)

Slope (ft./ft.) Kv (fps)

Travel time (min.)

450				1084	225
0.06				0.07	0.04
7.0				7.0	7.0
4.3	0.0	0.0	0.0	9.4	2.7

88

0.03

20.3

0.4

285

0.16

7.0

1.7

152

0.17

2.5

2.4

0.0

OPEN CHANNEL OR PIPE						
Flow length (ft.)	1325	2400	2400			575
Slope (ft./ft.)	0.05	0.05	0.05			0.01
Manning's n for channel flow	0.027	0.012	0.012			0.045
Open Channel - Bottom width (ft.)	4					15
Open Channel - Side slopes	2					7.5
Open Channel - Bank full depth (ft.)	2					1
Pipe - Diameter (in.)		30	30			
Travel time (min.)	1.6	2.0	2.0	0.0	0.0	4.2
Time of Concentration (min.)	38.5	4.0	4.0	0.0	30.7	20.2

SI URIMWA IE SWM BASIN D	SWM BASIN DESIGN CALCULATIONS	LATIONS									
Stormwater Management Facility	NYSDEC Practice	Contributing Subcatchments	Drainage Area	Impervious Area	Impervious Cover	WQv Required	WQv Provided Permanent Pool	WQv Provided Extended Detention	WQv Provided TOTAL	Basin Surface Area Required	Basin Surface Area Provided
			(acres)	(acres)	(%)	(cu.ft.)	(cu.ft.)	(cu.ft.)	(cu.ft.)	(sq.ft.)	(sq.ft.)
p03-2	Micropool ED	TOTAL 803-2 803-2(IC) 803-2(OW)	4.739 3.021 1.663 0.054	1.663 0.000 1.663 0.000	35.1	6,923	4,095	13,076	17,171	2,064	2,315
p08-2	Micropool ED	TOTAL s02-3 s08-2 s08-2(IC) s08-2(OW)	18.763 4.088 8.958 5.524 0.192	7.398 1.873 0.000 5.524 0.000	39.4	30,331	24,834	61,488	86,322	8,173	8,558
p08-3	Micropool ED	TOTAL \$08-3 \$08-3(IC) \$08-3(OW)	2.829 1.700 1.086 0.042	1.086 0.000 0.000 0.000	38.4	4,467	2,615	5,108	7,723	1,232	1,849
p09-2	Wet Pond	TOTAL 809-2 809-2(IC) 809-2(OW)	21.180 18.608 2.336 0.236	2.336 0.000 2.336 0.000	11.0	16,914	36,340	38,169	74,509	9,226	10,285
p13-1	Micropool ED	TOTAL \$13-1 \$13-1(IC) \$13-1(OW) \$16-2	12.222 3.555 6.360 0.131 2.176	6.360 0.000 6.360 0.000 0.000	52.0	<mark>25,296</mark>	16,480	13,636	30,116	5,324	5,894
p14-2	Micropool ED	TOTAL s14-2 s14-2(OW) s14-3 s14-3	15.935 0.504 0.176 6.794 8.460	8.460 0.000 0.000 8.460	<b>5</b> 3.1	<mark>33,585</mark>	23,903	14,628	38,531	6,941	7,681
p20-2	Wet Pond	TOTAL s20-2	13.511 8.157	5.112 0.000	37.8	21,070	35,913	51,774	87,687	5,885	10,535

10454.01 SILO RIDGE COUNTRY GOLF RESORT COMMUNITY STORMWATER - PROPOSED CONDITIONS SWM BASIN DESIGN CALCULI ATIONS The Chazen Companies S:\1\10400-10499\10454.00\ENG\Stormwater\Proposed Subcatchment Stats\_10454-01.xls 4/10/2006 3:39 PM

5.112 0.000

5.112 0.242

s20-2(IC) s20-2(OW)

10454.01 SILO RIDGE COUNTRY GOLF RESORT COMMUNITY STORMWATER - PROPOSED CONDITIONS SWM BASIN DESIGN CALCULATIONS

STORMWATER - PROPOSED CONDITIONS	I BASIN DESIGN CALCULATIONS
RMWATE	<b>I BASIN I</b>
5TO	WM

Basin Surface Area Provided	(sq.ft.)	5,112		3,323		3,941		7,318	
Su	(sq.ft.)	2,244		2,693		3,640		7,011	
7	(cu.ft.)	26,302		18,105		34,187		86,895	
WQv Provided Extended Detention	(cu.ft.)	11, <u>996</u>		13,258		25,203		70,968	
WQv Provided Permanent Pool	(cu.ft.)	14,306		4,847		8,984		15,927	
WQv Required	(cu.ft.)	6,934		4,937		15,648		29,035	
Impervious Cover	(%)	31.9		10.4		46.6		44.6	
Impervious Area	(acres)	1.643	0.000 1.643 0.000	0.643	0.000 0.643 0.000	3.890	0.000 3.890 0.000	7.185	0.000 7.185 0.000
Drainage Area	(acres)	5.153	3.392 1.643 0.117	6.182	5.463 0.643 0.076	8.355	4.375 3.890 0.090	16.094	8.741 7.185 0.168
Contributing Subcatchments		TOTAL	s21-4 s21-4(IC) s21-4(OW)	TOTAL	s21-6 s21-6(IC) s21-6(OW)	TOTAL	s21-7 s21-7(IC) s21-7(OW)	TOTAL	s23-2 s23-2(IC) s23-2(OW)
NYSDEC Practice		Micropool ED		Micropool ED		Micropool ED		Micropool ED	
Stormwater Management Facility		p21-4		p21-6		p21-7		p23-2	